

HISTORIC AND DESIGN REVIEW COMMISSION

March 17, 2021

HDRC CASE NO: 2021-107
ADDRESS: 6161 GIBBS-SPRAWL RD
LEGAL DESCRIPTION: NCB 17629 P-1 (17.015) P-8 (.376)
ZONING: R-6
CITY COUNCIL DIST.: 2
APPLICANT: Cullen Colrane/Vickrey & Associates
OWNER: Desiree Salmon/CITY OF SAN ANTONIO
TYPE OF WORK: Park improvements
APPLICATION RECEIVED: February 26, 2021
60-DAY REVIEW: Not Applicable Due to City Council Emergency Orders
CASE MANAGER: Rachel Rettaliata
REQUEST:

The applicant is requesting a Certificate of Appropriateness for approval to:

1. Replace gravel overflow parking with 10 paved parking spaces.
2. Construct a new dog park.
3. Install park amenities.
4. Install a Portland Loo restroom.
5. Install a shade structure.
6. Install a new concrete walkway.

APPLICABLE CITATIONS:

UDC Section 35-450. General Rules.

(a)Area of Jurisdiction. A certificate of appropriateness is required and shall be secured by a party prior to the issuance of a permit from the department of planning and development services before said party will be allowed to undertake activities affecting a designated historic landmark, property within a designated historic district, a state archaeological landmark, a recorded Texas historical landmark, property within a National Register Historic District, property listed on the National Register of Historic Places, a National Historic Landmark, property within the river improvement overlay district, public property, public rights-of-way, or public art.

UDC Sec. 35-641. - Design Considerations for Historic and Design Review Commission Recommendations.

In reviewing an application, the historic and design review commission shall be aware of the importance of attempting to find a way to meet the current needs of the City of San Antonio, lessee or licensee of public property. The historic and design review commission shall also recognize the importance of recommending approval of plans that will be reasonable to implement. The best urban design standards possible can and should be employed with public property including buildings and facilities, parks and open spaces, and the public right-of-way. Design and construction on public property should employ such standards because the use of public monies for design and construction is a public trust. Public commitment to quality design should encourage better design by the private sector. Finally, using such design standards for public property improves the identity and the quality of life of the surrounding neighborhoods.

UDC Sec 35-642. – New Construction of Buildings and Facilities:

In considering whether to recommend approval or disapproval of a certificate, the historic and design review commission shall be guided by the following design considerations. These are not intended to restrict imagination, innovation or variety, but rather to assist in focusing on design principles, which can result in creative solutions that will enhance the city and its neighborhoods. Good and original design solutions that meet the individual requirements of a specific site or neighborhood are encouraged and welcomed.

(a) Site and Setting.

(1) Building sites should be planned to take into consideration existing natural climatic and topographical features. The intrusive leveling of the site should be avoided. Climatic factors such as sun, wind, and temperature should become an integral part of the design to encourage design of site-specific facilities which reinforces the individual identity of a neighborhood and promotes energy efficient facilities.

- (2) Special consideration should be given to maintain existing urban design characteristics, such as setbacks, building heights, streetscapes, pedestrian movement, and traffic flow. Building placement should enhance or create focal points and views. Continuity of scale and orientation shall be emphasized.
- (3) Accessibility from streets should be designed to accommodate safe pedestrian movement as well as vehicular traffic. Where possible, parking areas should be screened from view from the public right-of-way by attractive fences, berms, plantings or other means.
- (4) Historically significant aspects of the site shall be identified and if possible incorporated into the site design. Historic relationships between buildings, such as plazas or open spaces, boulevards or axial relationships should be maintained.

FINDINGS:

- a. The property located at 6161 Gibbs Sprawl is commonly known as Lou Kardon Park and is located northeast of downtown. The park is bound by Gibbs Sprawl to the south, Castle Cross to the west, green space between the park and East Village to the north, and green space to the east. The proposed park improvements include installing new paved parking spaces, parking lot lighting, park amenities, a new dog park, a new Portland Loo restroom, concrete walkways, and shade structures.
- b. **PARKING LOT INSTALLATION** – The applicant has proposed to install 10 new paved parking spaces and parking lot lighting to the west of the existing parking lot that is accessed from Gibbs Sprawl. The parking lot addition will be connected to the park and proposed dog park by accessible walkways. Staff finds the proposal consistent with the UDC.
- c. **DOG PARK** – The applicant has proposed to construct a fully enclosed dog park with areas for small and large dogs. The proposed dog park will be located west of the existing and proposed parking lots. The dog park will include wire mesh panel fencing installed in concrete footings. The proposed fencing will be 6 feet in height for the large dog area and 4 feet in height for the small dog area. The dog park will feature an all-dog entry gate and a small dog entry gate, mulch area, concrete walks, trash & recycling receptacles, picnic benches with shade, dog waste dispensers, a drinking fountain, and a 2-bowl dog drinking fountain. Staff finds the proposal generally appropriate.
- d. **SIGNAGE** – The applicant has proposed to install new signage for the proposed dog park. The signage proposal includes a 4-foot-tall steel post sign in concrete footings with a dog park rules sign of approximately 9 square feet with a dark green background and white text. Additional standard Parks dog park signage is proposed as well as two signs with emergency information and pet waste disposal information. Staff finds the proposal consistent with the UDC.
- e. **SHADE STRUCTURE** – The applicant has proposed to install five cantilever umbrella shade structures. One shade structure will be 15'x15', two of these structures are proposed for the dog park and two more are proposed for existing picnic areas. The second shade structure will be 25'x22', and this shade structure is proposed to cover an existing exercise equipment area. The proposed shade canopy features a large shade canopy supported by a metal post and arm set in concrete. Staff finds the proposal appropriate.
- f. **RESTROOM FACILITY** – The applicant has proposed to install a Portland Loo restroom facility to the north of the existing parking lot. Staff finds the proposal appropriate.
- g. **WALKWAY INSTALLATION** – The applicant has proposed to install new concrete paving to connect the new and existing parking lots with the dog park and the existing walking trail. The proposed paving will range from 6'-6" in width to 8' in width. Staff finds the proposal consistent with the UDC.
- h. **ARCHAEOLOGY** – The project shall comply with all federal, state, and local laws, rules, and regulations regarding archaeology, as applicable.

RECOMMENDATION:

Staff recommends approval based on findings a through h with the following stipulation:

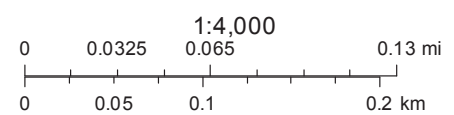
- i. **ARCHAEOLOGY** – The project shall comply with all federal, state, and local laws, rules, and regulations regarding archaeology, as applicable.

City of San Antonio One Stop



March 11, 2021

— User drawn lines





Date:	Revisions/Submissions

CITY OF SAN ANTONIO
PARKS AND RECREATION DEPARTMENT
PARK PROJECT SERVICES
DIVISION
SAN ANTONIO, TX 78204
78283-5966
FAX: (210) 207-2720

506 DOLOROSA ST.
P.O. BOX 859966
TEL: (210) 207-2679

LOU KARDON PARK
6651 Gibbs Sprawl Rd.
SAN ANTONIO, TEXAS
OVERALL SITE PLAN

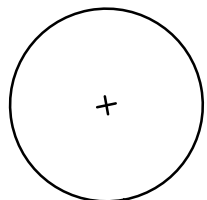
L2.0
26

Designed By:
Cullen Coltrane
Drawn By:
SDT
Date:
02/09/2021
Project No.:
2901-003
Filename:
REVISED DOG PARK
LAYOUT.DWG

1 SUMMARY OF SCOPE OF WORK AND NOTES

1. Alternate #: install sidewalk from subdivision to existing gravel trail in park.
2. Unless otherwise noted, areas requiring demolition includes removing topsoil and gravel to sub-grade. Rough grade in preparation of proposed improvements.
3. Install tree protection fencing within limits of work. See sheet L11 for Tree Preservation Plan.
4. Install 6" of double shredded native mulch in all unpaved areas within the dog park. See sheet L20 for Site Plan.
5. Fence post locations are shown graphically only and may not correspond to proper post spacing. Refer to detail 3/L2.2 for fence post spacing.
6. All dog park gates to swing inward.
7. Install concrete paving as indicated on drawings. 5% maximum longitudinal slope, 2% maximum cross slope. Dowel connections between existing and new paving.
8. Install benches, mutt-mitts, animal proof trash receptacle, dog park signage and drinking fountain. Locations to be field verified by landscape architect. Refer to civil sheets for location of water line for connection to drinking fountain.
9. Install site light standards. Refer to electrical plans for details and locations.
10. Install pre-fabricated restroom and connect to potable water, sanitary sewer and electricity. Refer to civil sheets for potable water and sanitary sewer details and location. Refer to electrical sheets for power details to restroom.
11. Install ten-vehicle parking lot. Refer to civil sheets for parking lot layout, grading and dimensions.
12. Clear invasive species and trim trees to 6' height within 10' of walking trail. contractor to walk site with landscape architect and parks department to field identify trees to be selectively cleared.

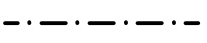
2 LEGEND



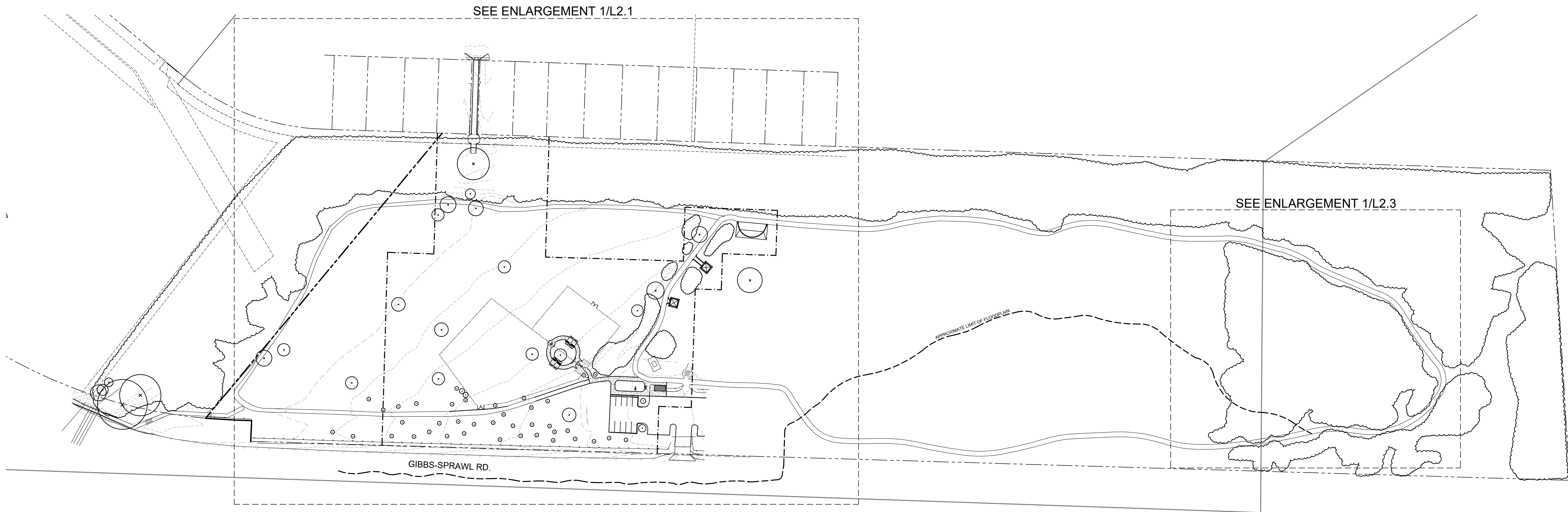
EXISTING TREE (TYP.)



EXISTING TREE CANOPY

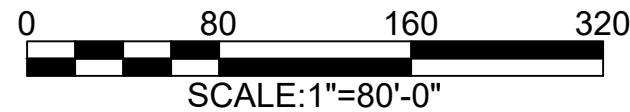


ARTIFICIAL LOT LINE/PROJECT LIMIT



3 OVERALL SITE PLAN

1"=80'-0"



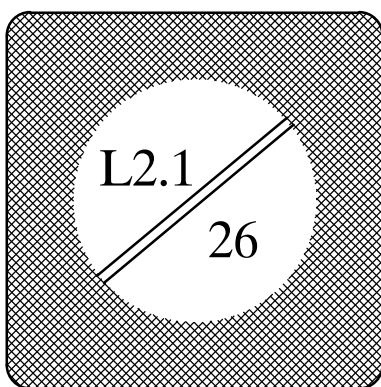
SCALE: 1"=80'-0"



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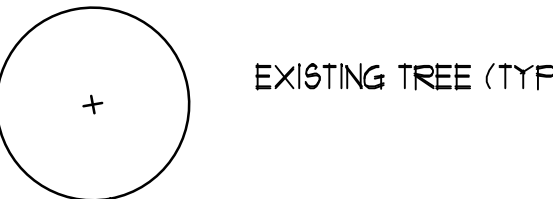
LOU KARDON PARK
6651 Gibbs Sprawl Rd.
SAN ANTONIO, TEXAS
SITE PLAN



Designed By:	Cullen Coltrane
Drawn By:	SDT
Date:	02/09/2021
Project No.:	2901-003
Filename:	REVISED DOG PARK LAYOUT.DWG

2 LEGEND

- 1 LARGE DOG PARK SIDE
- 2 SMALL DOG PARK SIDE
- 3 INSTALL DOUBLE SHREDDED NATIVE MULCH (6" DEPTH)
- 4 PARKING LOT: REF. CIVIL DRAWINGS AND SPECIFICATIONS FOR DETAILS.
- 5 RESTROOM: REF. SPECS
- 6 ADD. ALTERNATE #1. REFERENCE CIVIL DRAWINGS AND SPECIFICATIONS FOR DETAILS.
- 7 LIGHT STANDARD. REF. TO ELECTRICAL PLANS AND SPECIFICATIONS FOR DETAILS.
- 8 CLEAR INVASIVE SPECIES AND TRIM TREES TO 6' HEIGHT WITHIN 10' OF WALKING TRAIL. CONTRACTOR TO WALK SITE WITH LANDSCAPE ARCHITECT AND PARKS DEPARTMENT TO FIELD IDENTIFY TREES TO BE SELECTIVELY CLEARED. DO NOT REMOVE PROTECTED-SIZE TREES.
- 9 DRINKING FOUNTAIN, REFER TO SPECIFICATIONS
- 10 TRASH/RECYCLE RECEPTACLES, REFER TO SPECIFICATIONS.
- 11 2-BOWL DOG DRINKING FOUNTAIN, REFER TO SPECIFICATIONS



EXISTING TREE (TYP.)



EXISTING TREE CANOPY

--- ARTIFICIAL LOT LINE/PROJECT LIMIT



1 SITE PLAN
1"=40'-0"



0 40 80 120
SCALE: 1"=40'-0"

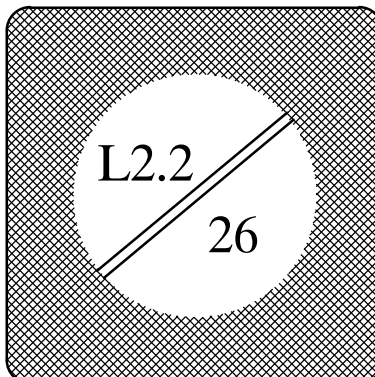


San Antonio
Parks and
Recreation
Department

Date:	Revisions/Submissions

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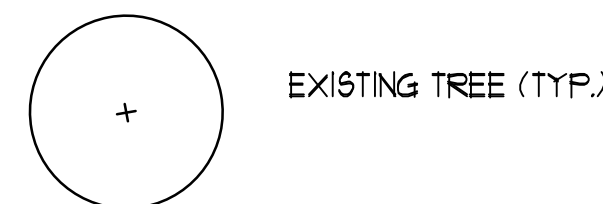
LOU KARDON PARK
6651 Gibbs Sprawl Rd.
SAN ANTONIO, TEXAS
ENLARGED SITE PLAN



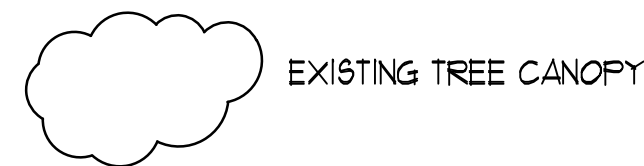
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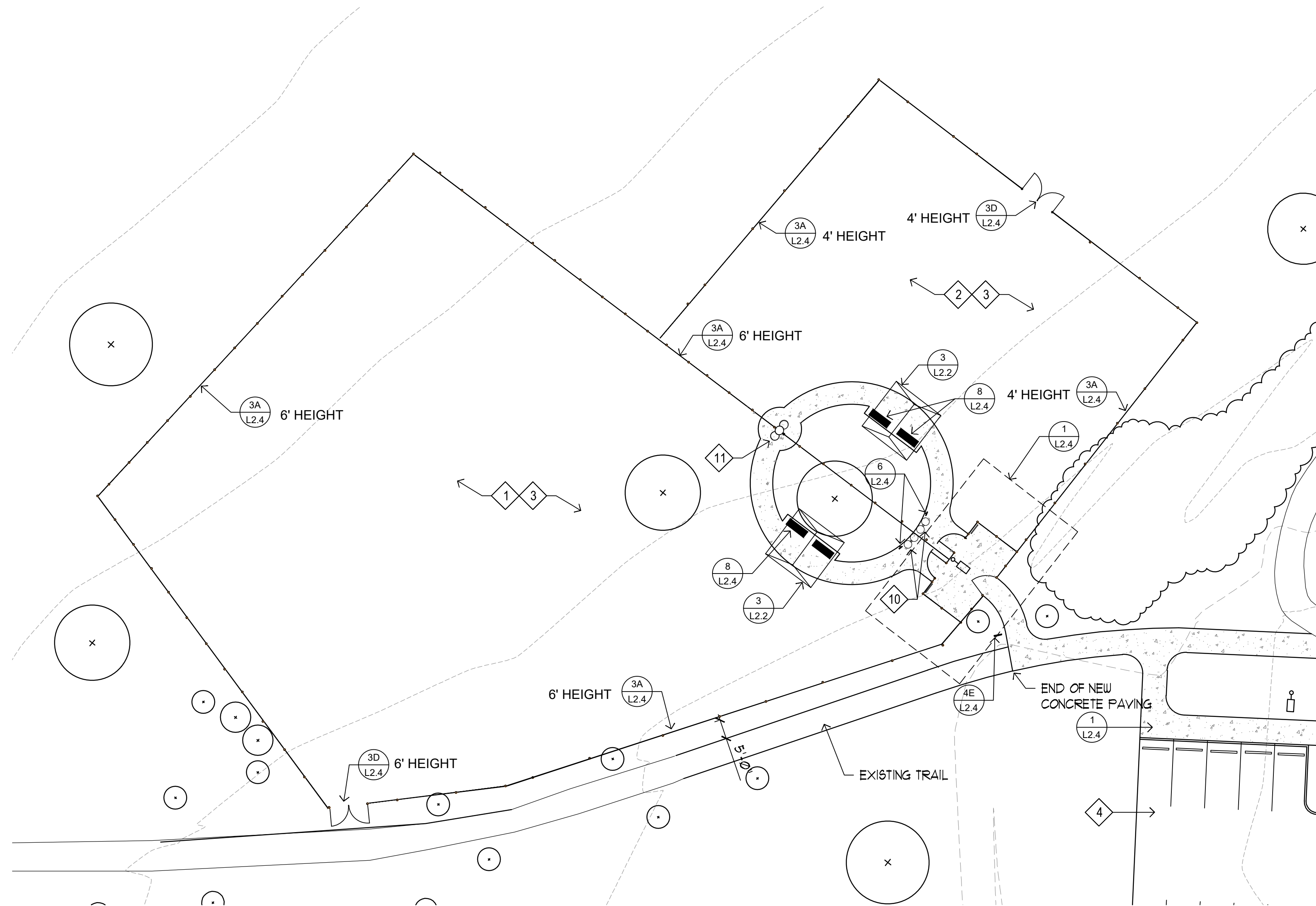


EXISTING TREE (TYP.)



EXISTING TREE CANOPY

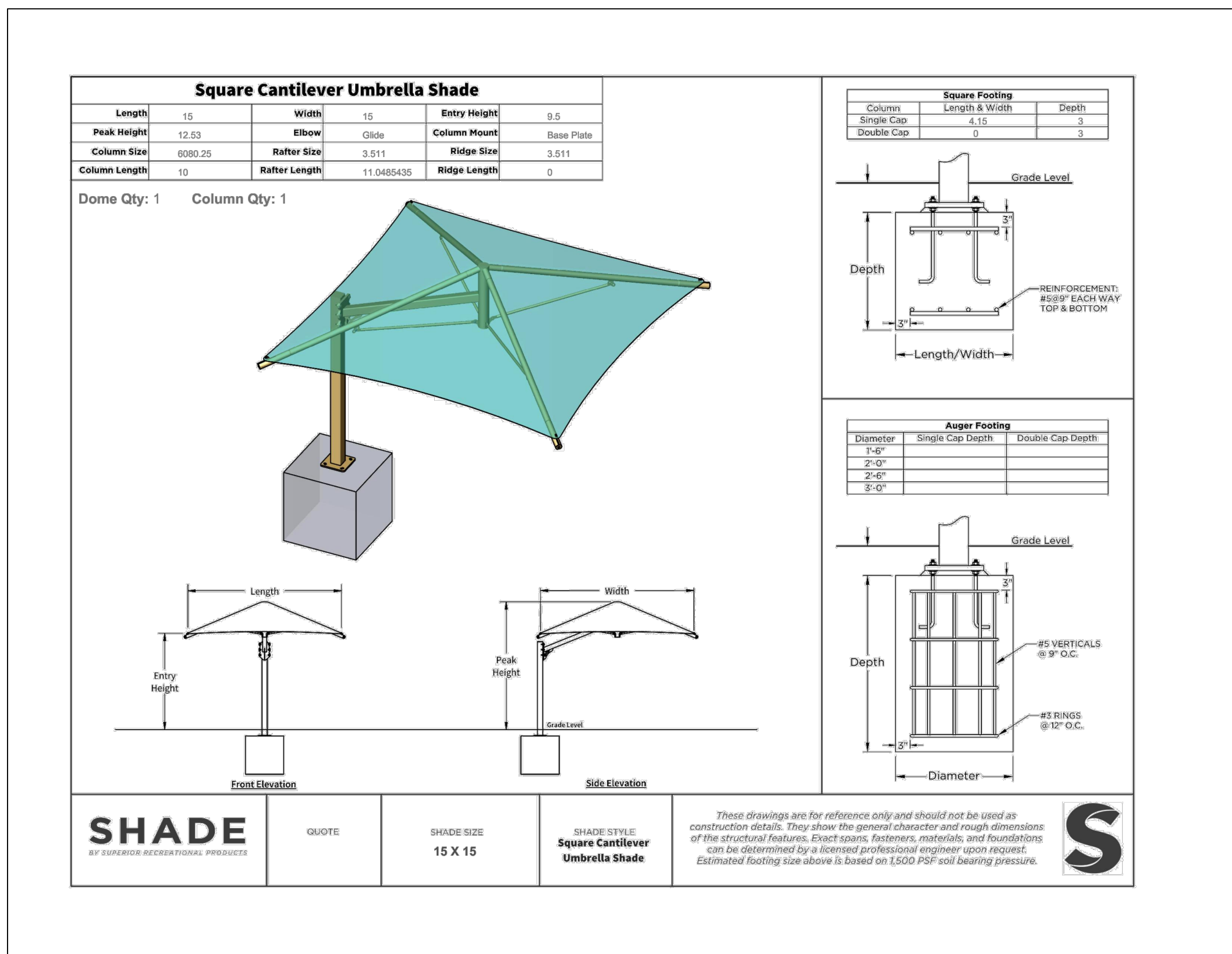
--- ARTIFICIAL LOT LINE/PROJECT LIMIT



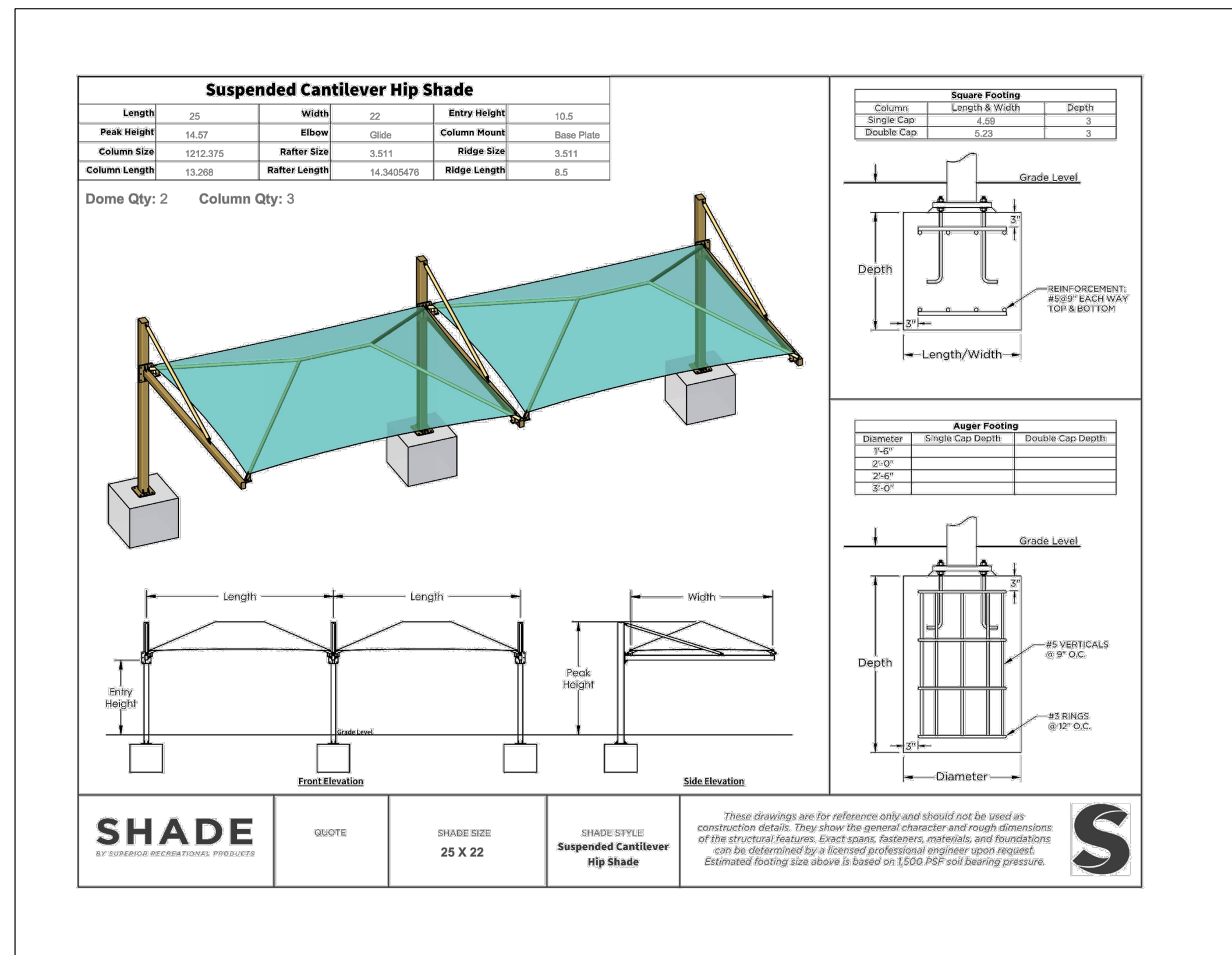
1 ENLARGED SITE PLAN: DOG PARK
1"=20'-0"



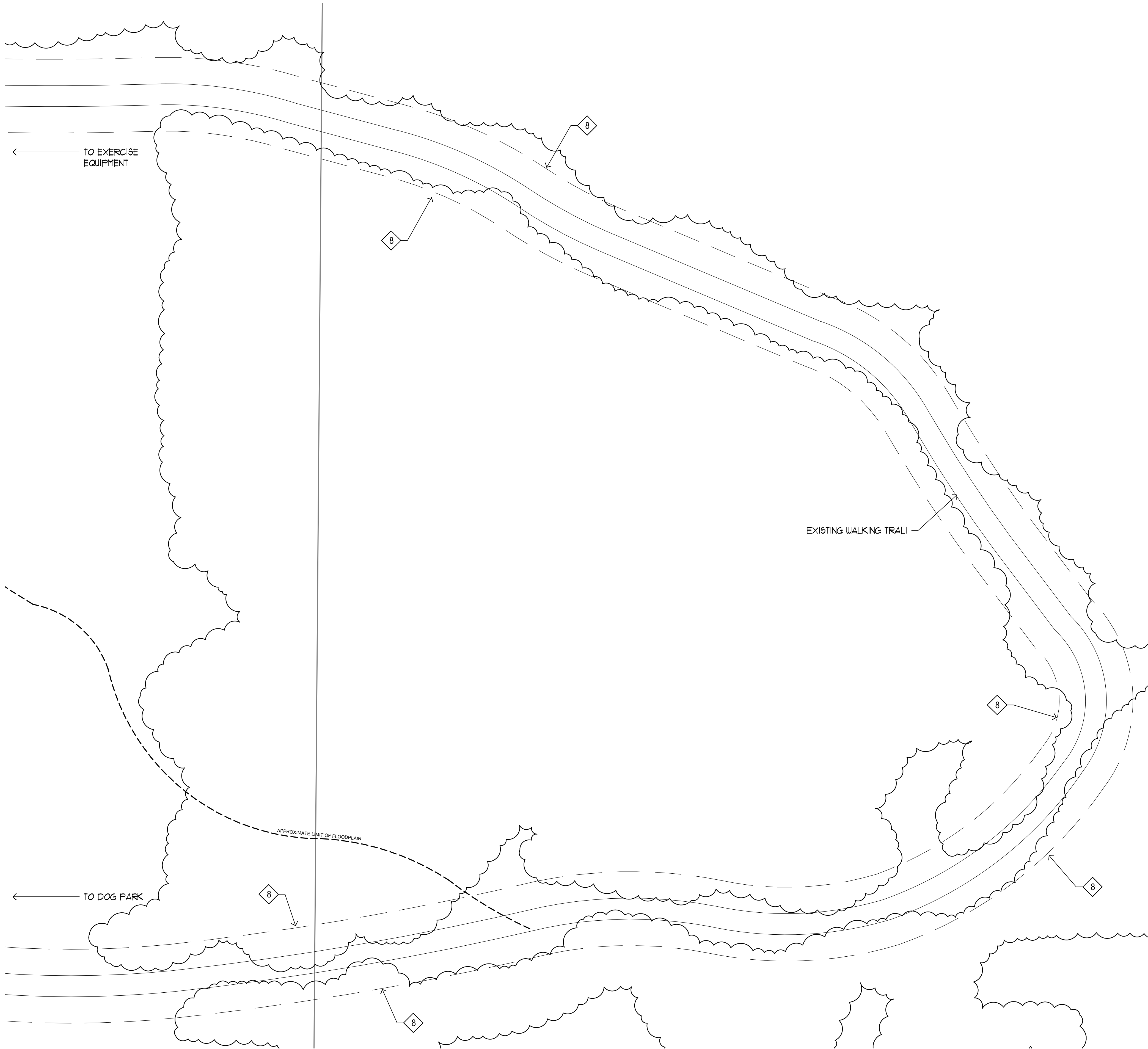
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SCALE: 1"=20'-0"



3 SMALL SHADE CANOPY
(ADD ALTERNATE OVER EXISTING PICNIC AREAS)



4 LARGE SHADE CANOPY (ADD ALTERNATE #2)



1 ENLARGED SITE PLAN: TRAIL CLEARING
1"=20'-0"

2 LEGEND

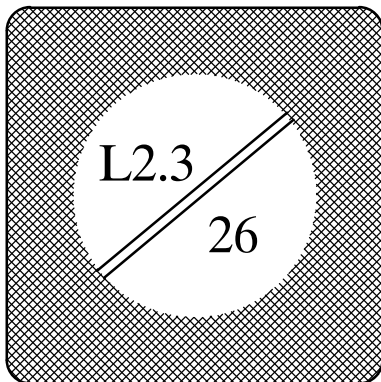
- 8 CLEAR INVASIVE SPECIES AND TRIM TREES TO 6' HEIGHT WITHIN 10' OF WALKING TRAIL. CONTRACTOR TO WALK SITE WITH LANDSCAPE ARCHITECT AND PARKS DEPARTMENT TO FIELD IDENTIFY TREES TO BE SELECTIVELY CLEARED. DO NOT REMOVE PROTECTED-SIZE TREES.
- + EXISTING TREE (TYP.)
- EXISTING TREE CANOPY
- - - - - ARTIFICIAL LOT LINE/PROJECT LIMIT



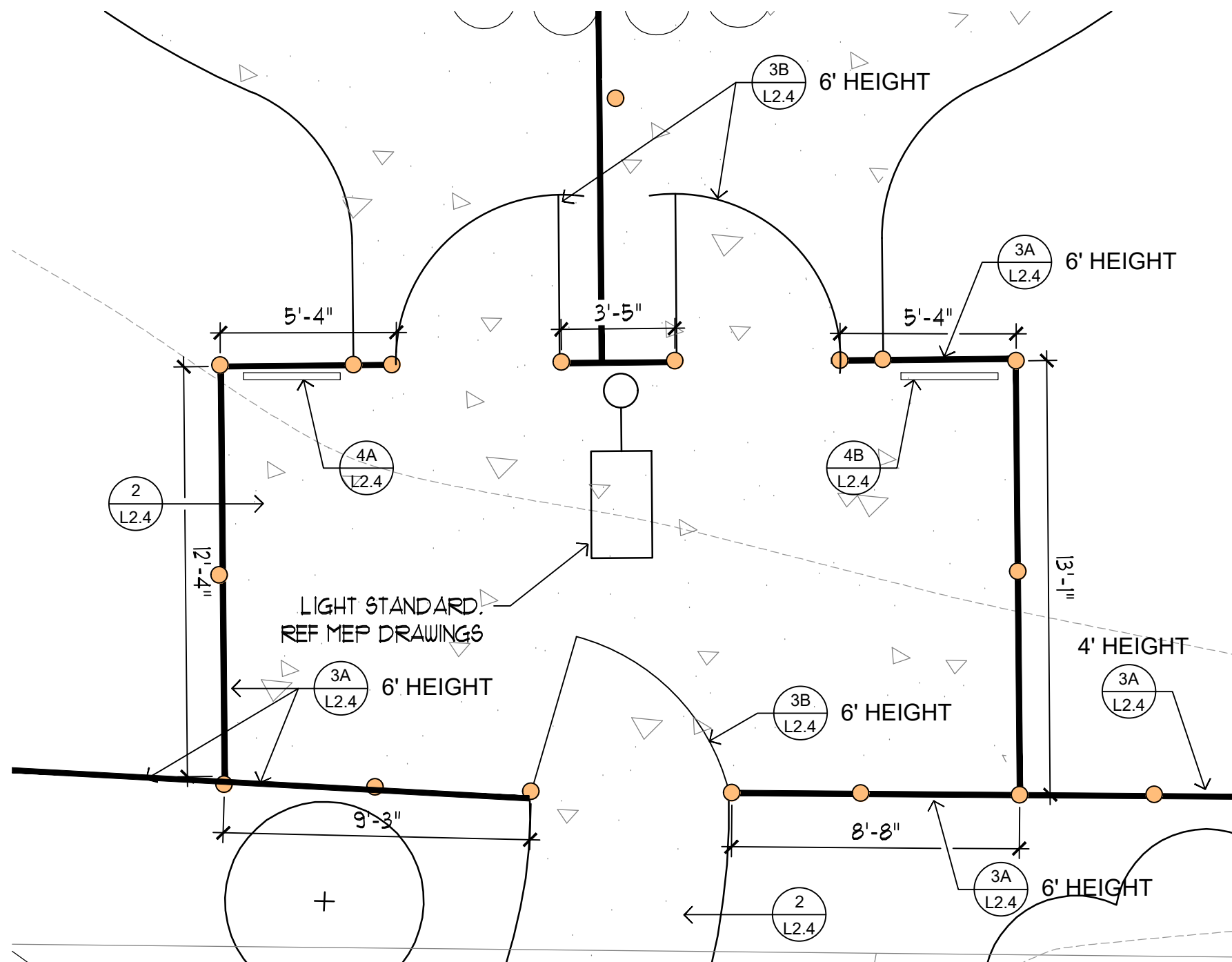
Date:	Revisions/Submissions

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PARK PROJECT SERVICES
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P.O. BOX 839966
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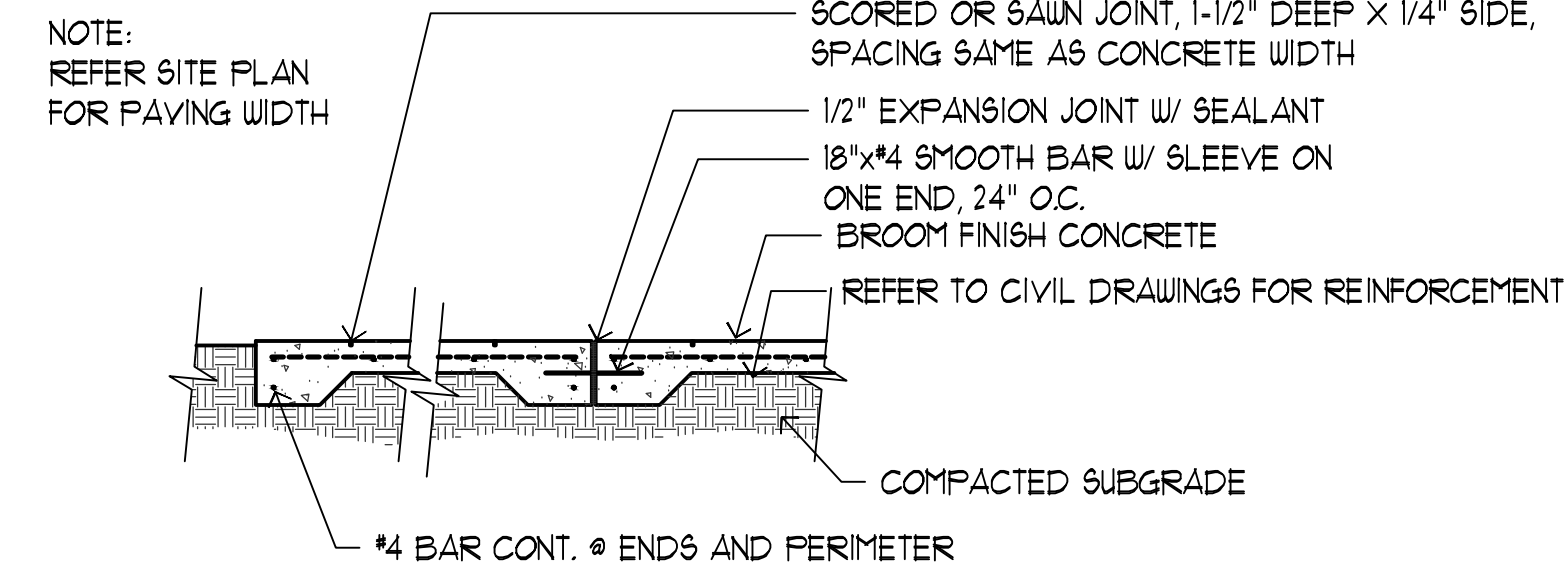
LOU KARDON PARK
6651 Gibbs Sprawl Rd.
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ENLARGED SITE PLAN



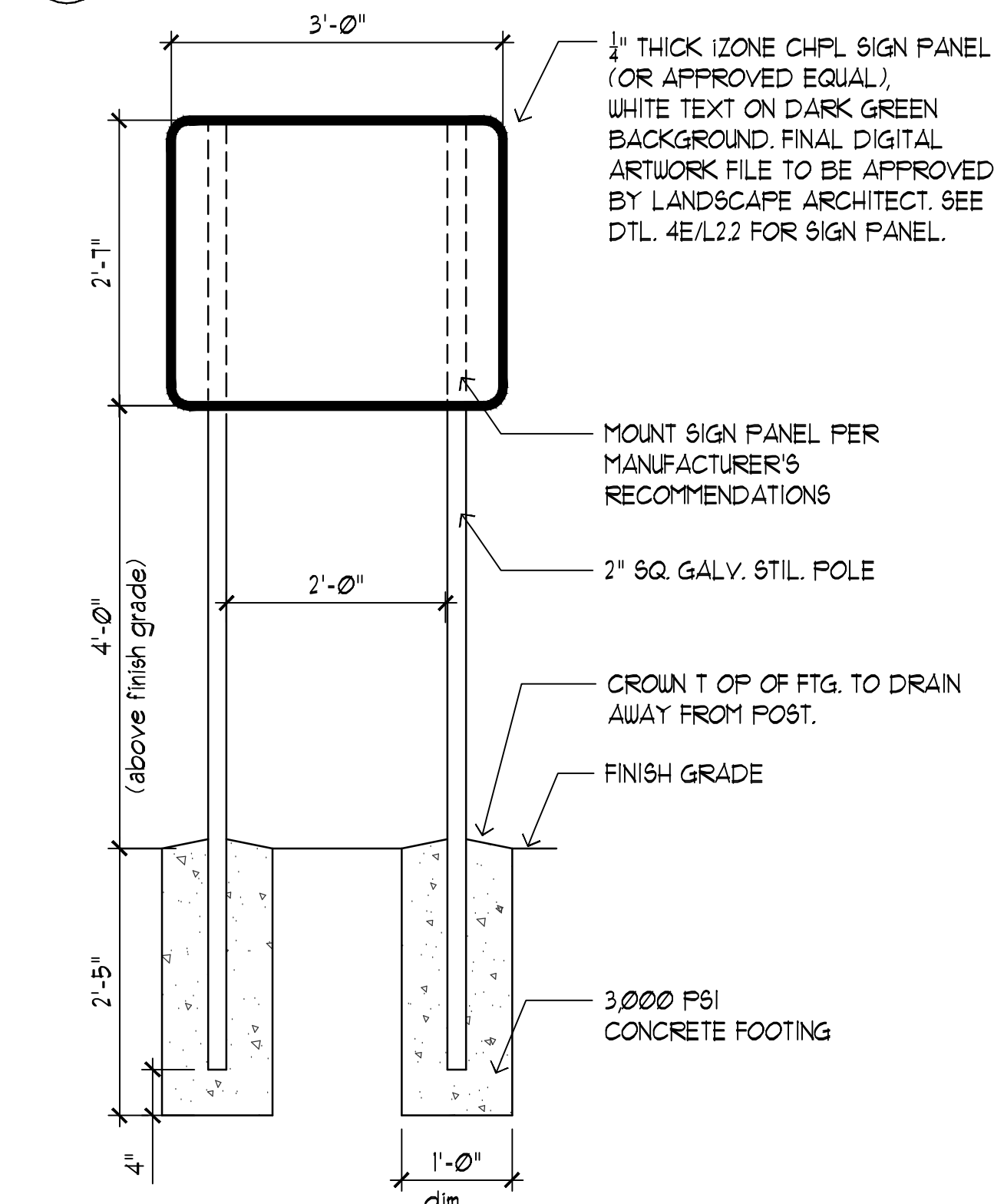
Designed By:	Cullen Coltrane
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1 DOG PARK VESTIBULE
1/4"=1'-0"

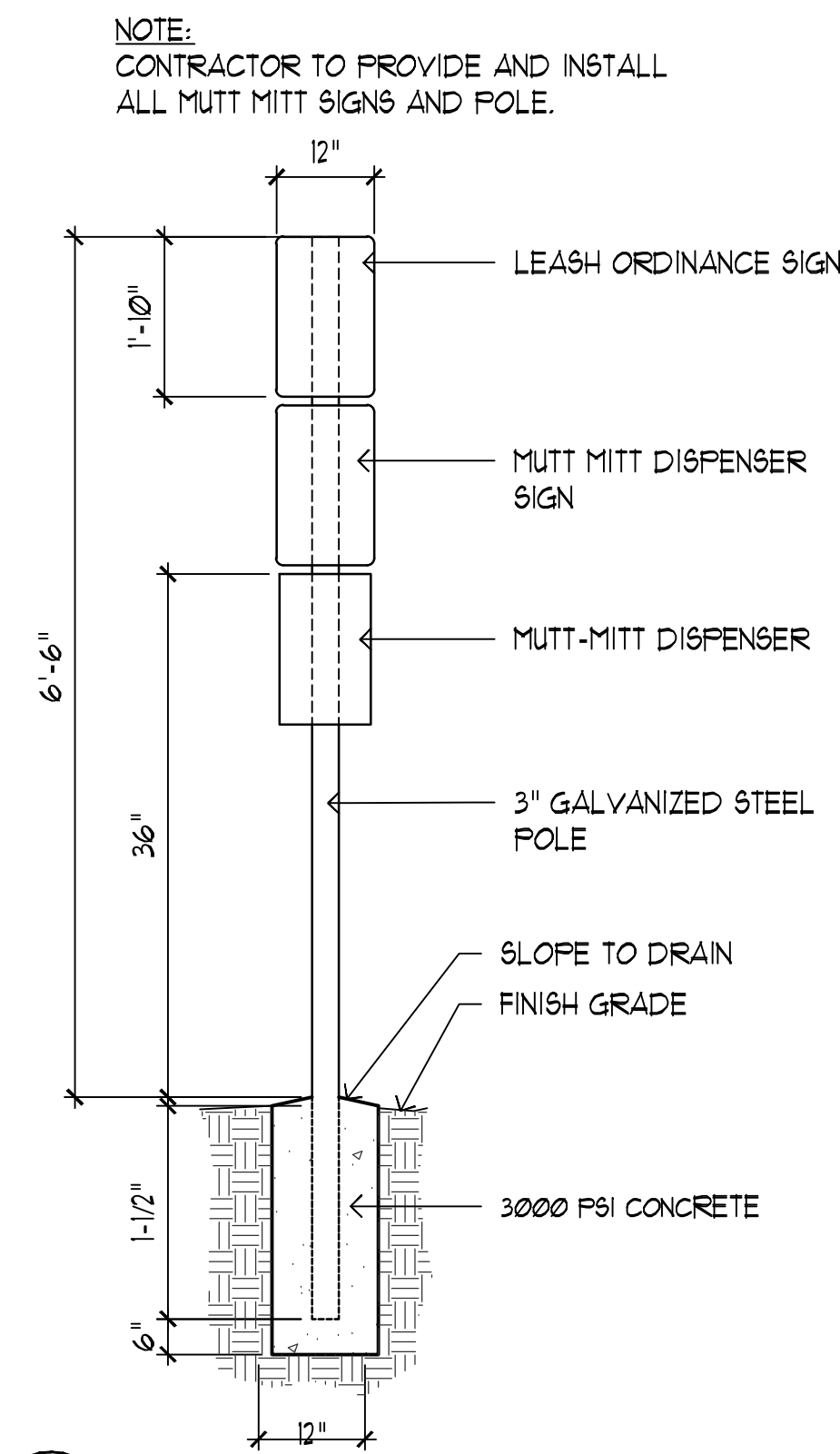


2 CONCRETE PAVING
1/2"=1'-0"



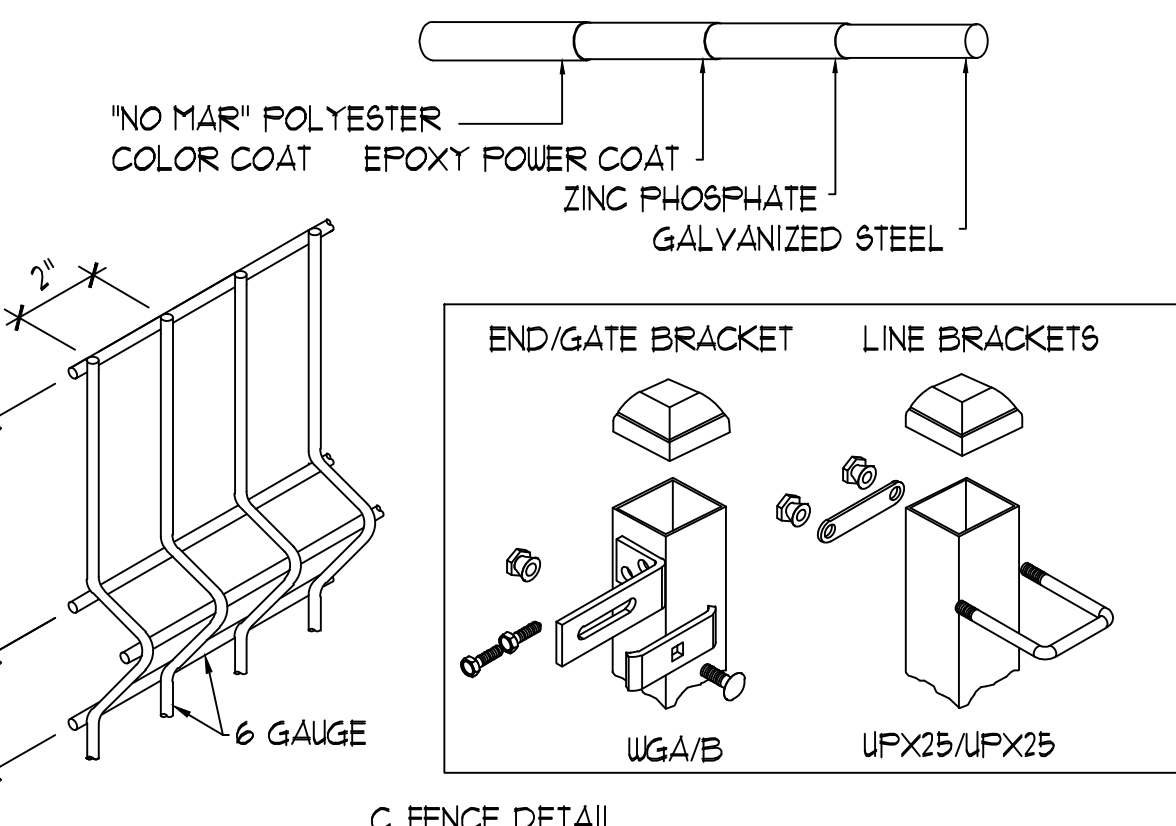
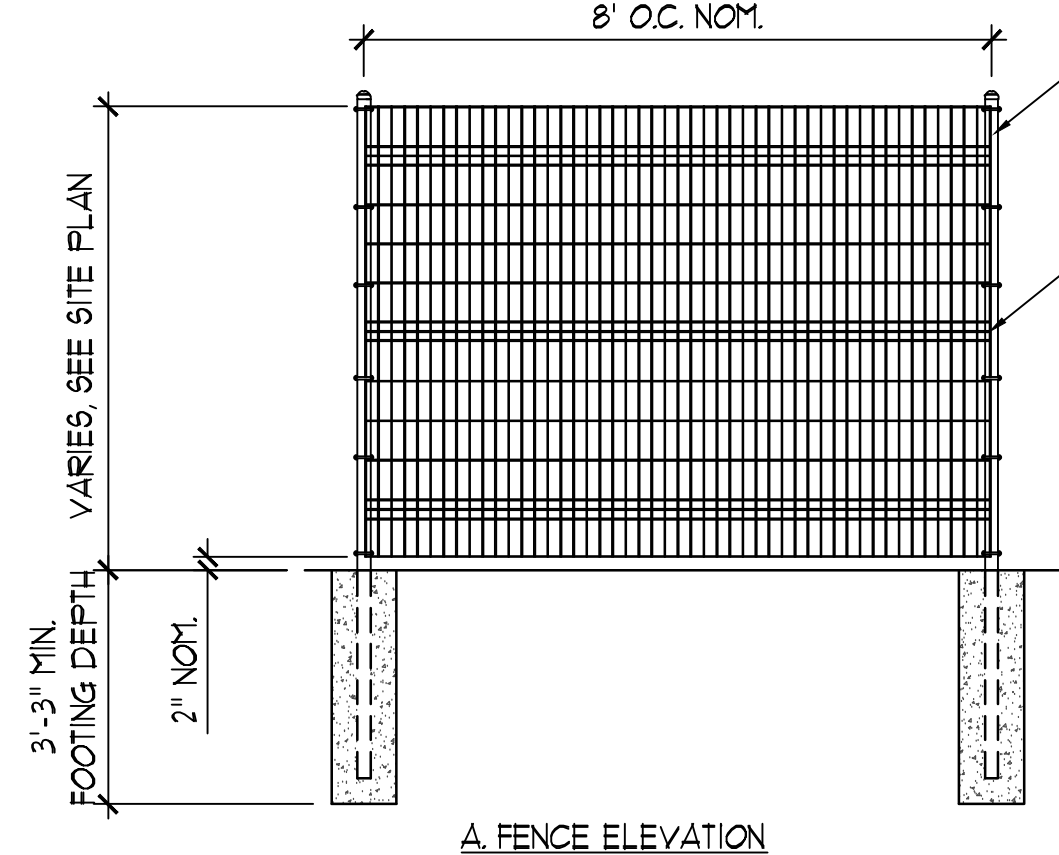
NOTE:
1. LOCATIONS SHOWN ON SITE PLAN
2. SECURELY FASTEN SIGN PANEL TO POST AT LOCATIONS TO NOT INTERFERE WITH LETTERING.

5 DOG PARK RULES SIGN
3/4"=1'-0"

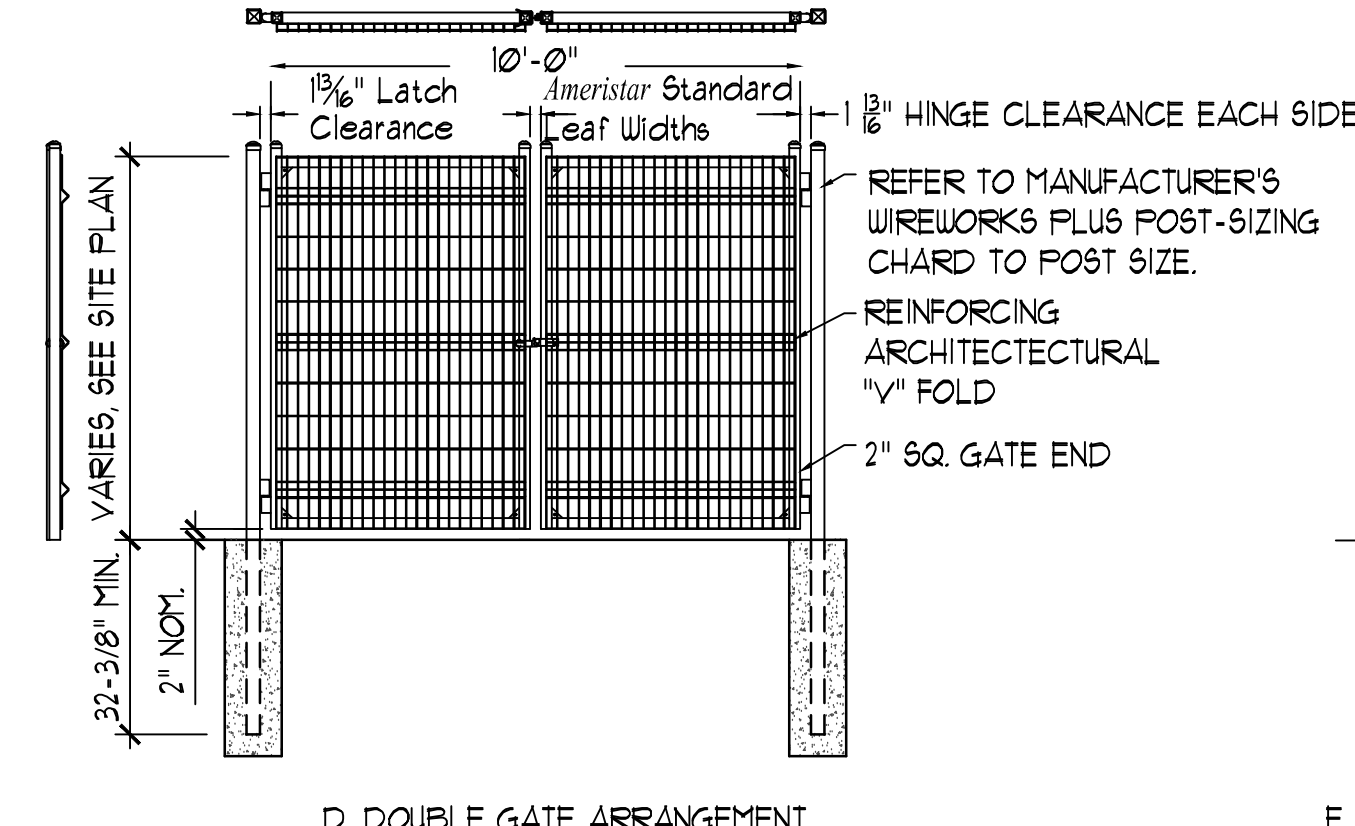
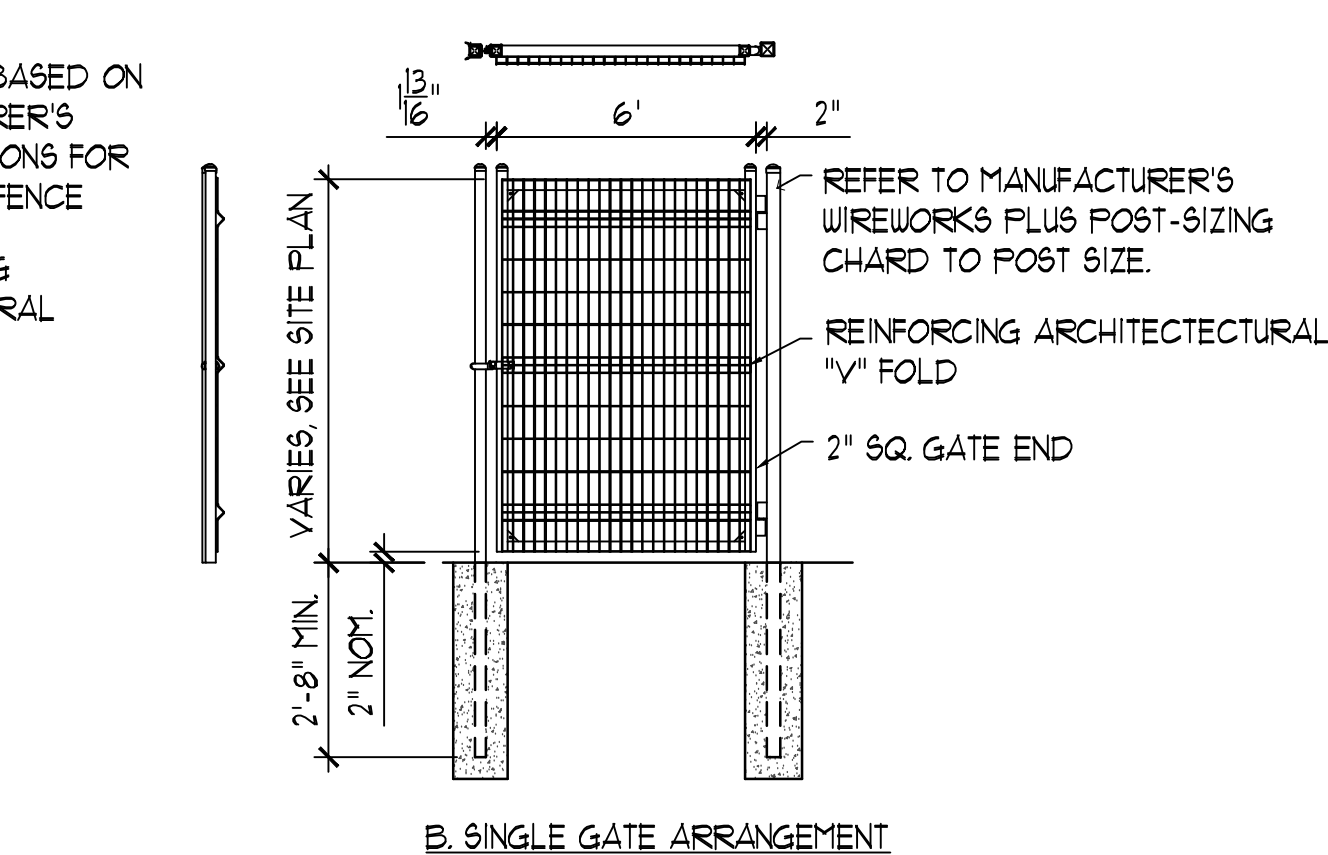


6 MUTT MITT DISPENSER
3/8"=1'-0"

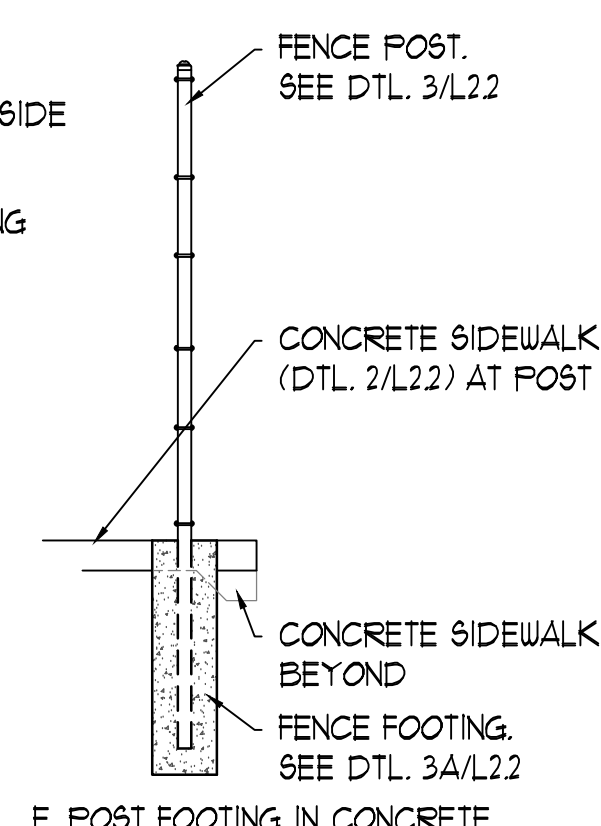
- NOTES:
1. REFER TO WIREWORKS PLUS SPECIFICATIONS FOR POST SIZE BASED ON 6'-0" AND 4'-0" HEIGHT, WEIGHT AND LOAD WIND LOAD REQUIREMENTS.
 2. FENCE AND GATES WITHIN LARGE DOG PARK AREA ARE TO BE 6'-0". FENCES AND GATES WITHIN SMALL DOG PARK AREA ARE TO BE 4'-0". REFER TO SITE PLAN.
 3. REFER TO MANUFACTURER'S GATE TABLE FOR STANDARD OUT TO OUTS.
 4. REFER TO MANUFACTURER'S SPECIFICATIONS FOR TOTAL "V" FOLD REQUIREMENTS BASED ON 6'-0" FENCE.
 5. VALUES SHOWN ARE NOMINAL AND NOT TO BE USED FOR INSTALLATION PURPOSES. SEE MANUFACTURER'S SPECIFICATIONS FOR INSTALLATION REQUIREMENTS.
 6. ALL GATES TO SWING INWARD.



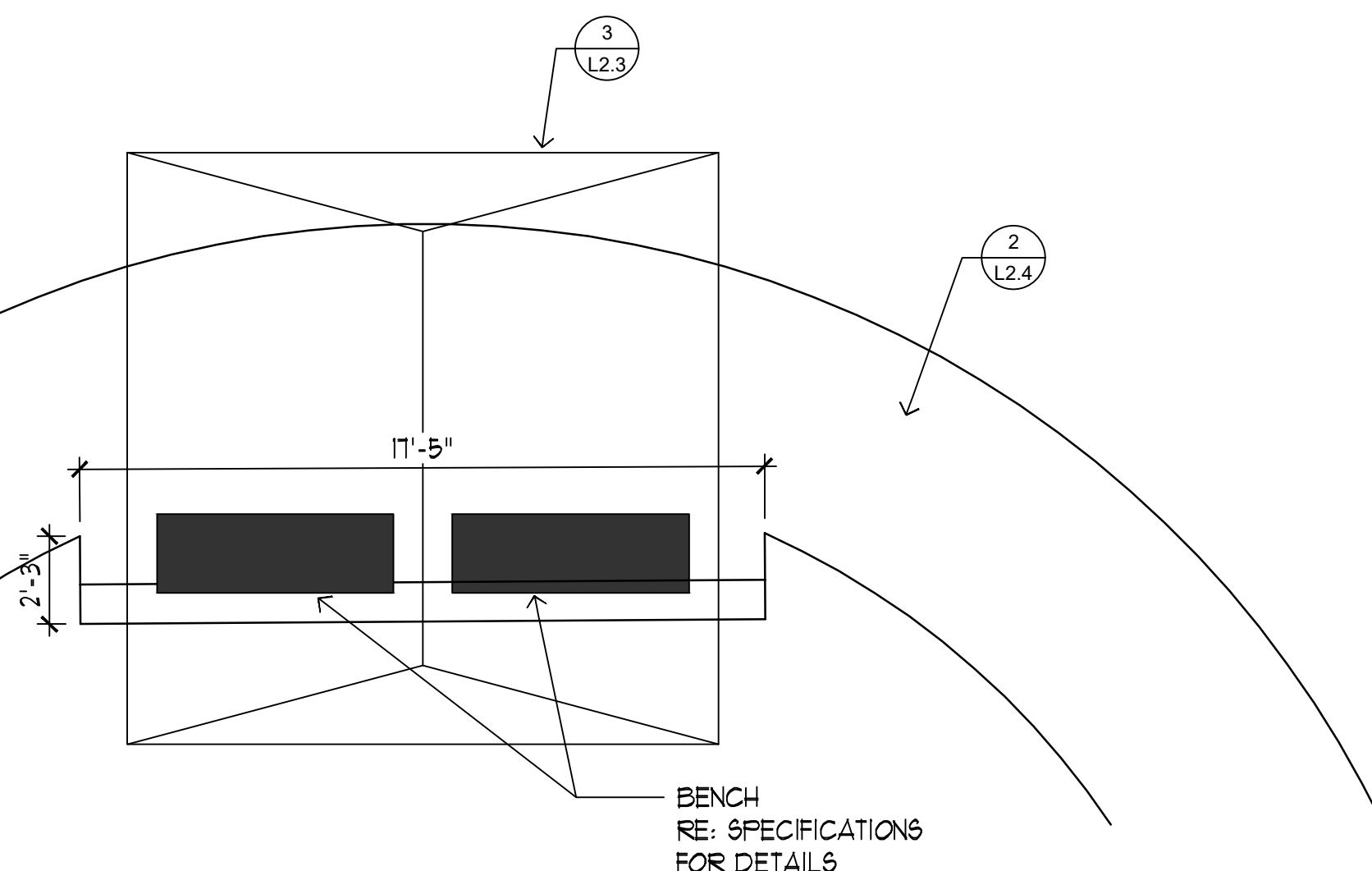
3 DOG PARK FENCE
3/8"=1'-0"



D. DOUBLE GATE ARRANGEMENT



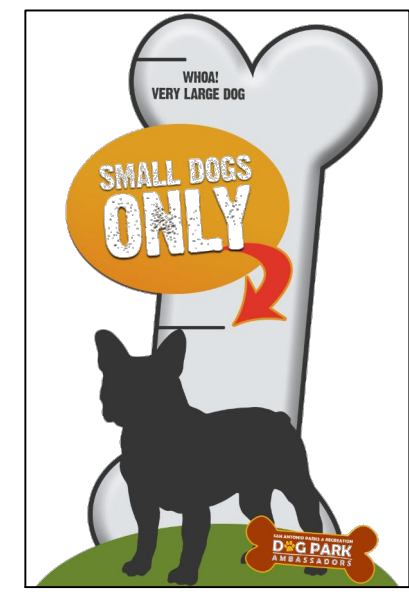
E. POST FOOTING IN CONCRETE



7 BENCH WITH SHADE CANOPY
1/4"=1'-0"



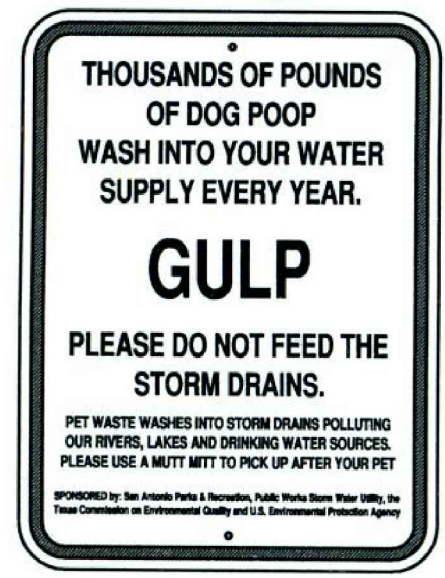
A.



B.



C.



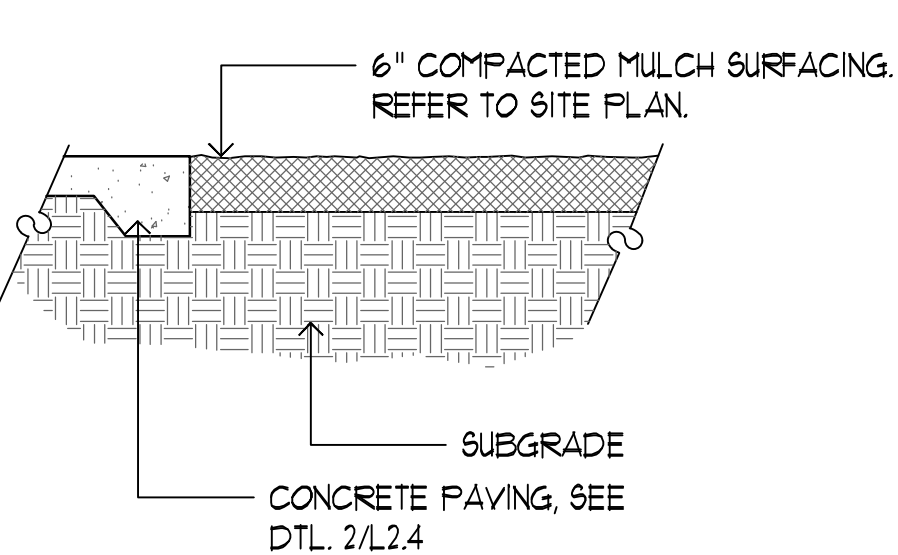
D.



E.

- SIGN PANEL NOTES:
1. ALL SIGN PANELS TO BE 1/4" THICK IZONE CHIPEL SIGN PANEL (OR APPROVED EQUAL). FINAL DIGITAL ARTWORK TO BE PROVIDED BY LANDSCAPE ARCHITECT.
 2. SIGN PANELS "A" AND "B" TO BE SECURED AT ALL FOUR CORNERS, DIRECTLY ON FENCING, PER MANUFACTURER'S RECOMMENDATION.
 3. SIGN PANELS "C", "D", AND "E" TO BE POST-MOUNTED (SEE DTL. 5/L2.2) SECURED AT TWO POINTS, PER MANUFACTURER'S RECOMMENDATION.

4 DOG PARK SIGNS
NTS

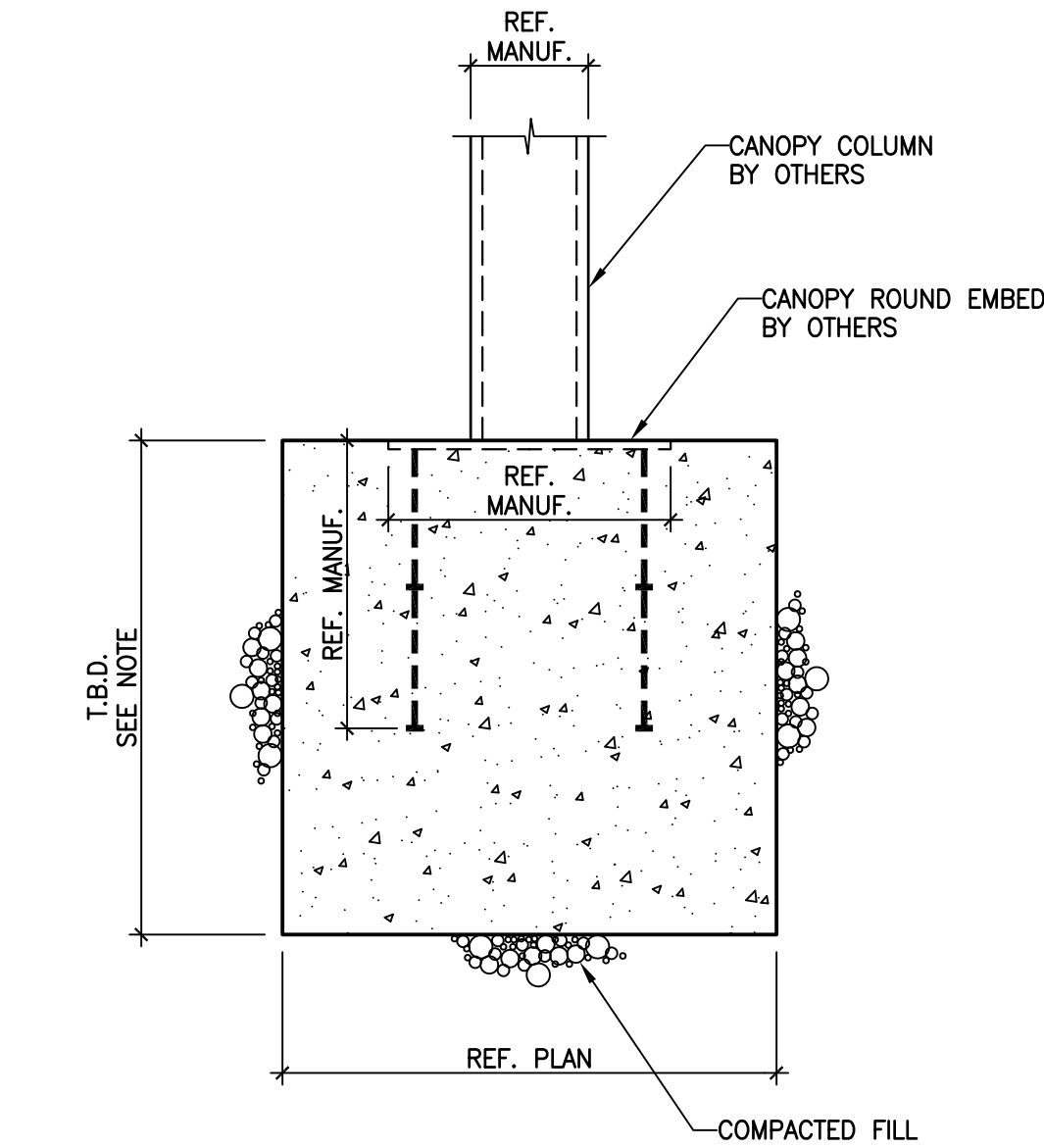


8 DOG PARK MULCH
1/2"=1'-0"

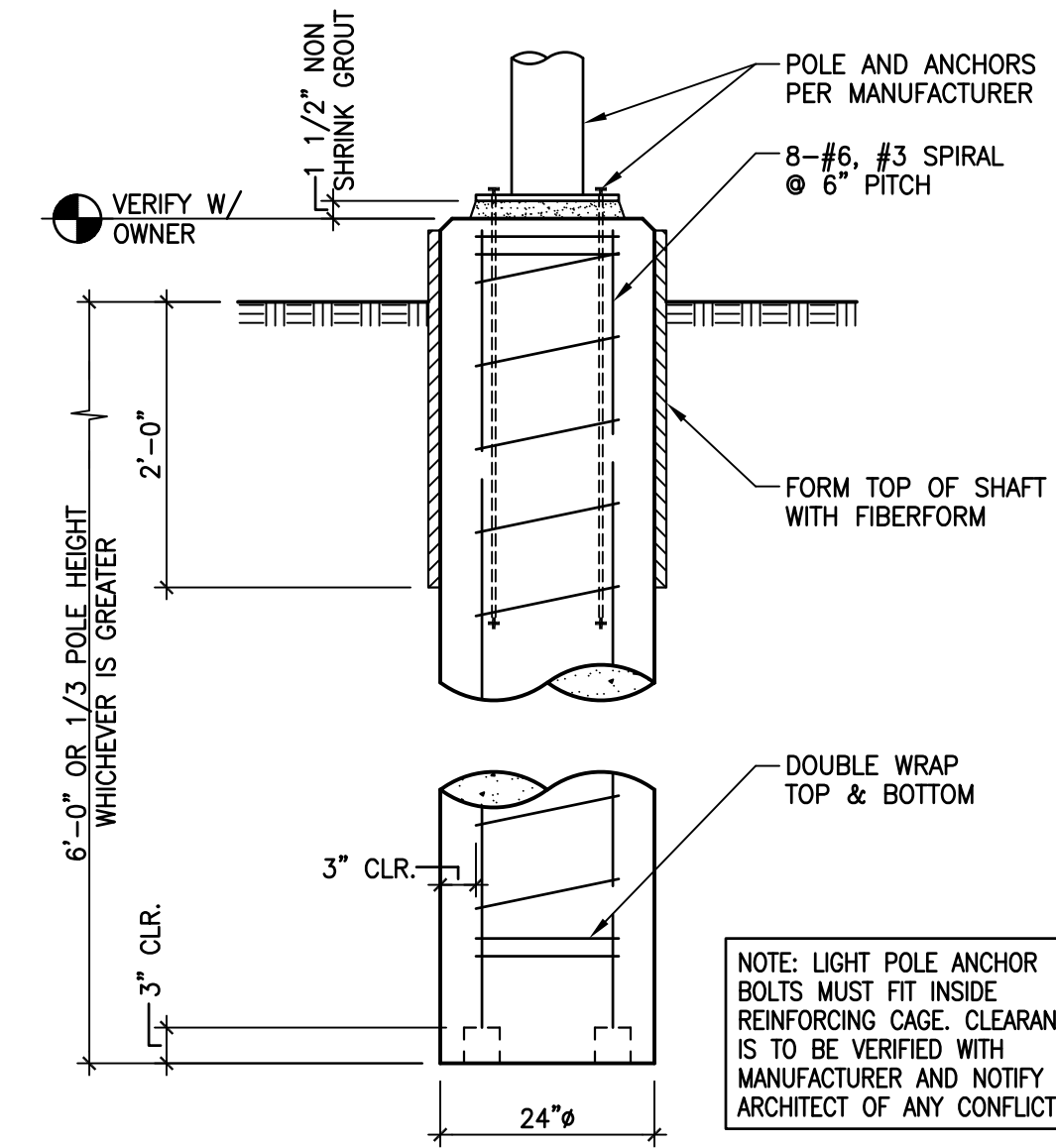
THIS DOCUMENT IS RELEASED FOR THE PURPOSE OF INTERIM REVIEW UNDER THE AUTHORITY OF CULLEN COLTRANE, NUMBER: 1784 ON FEB. 9, 2021. IT IS NOT TO BE USED FOR CONSTRUCTION, BIDDING, OR PERMIT PURPOSES.

Date:	Revisions/Submissions

NOTE: FOOTING DESIGNS ARE PRELIMINARY UNTIL FINAL REACTIONS ARE PROVIDED BY CANOPY MANUFACTURERS



3 DETAIL TYPICAL N.T.S.



4 DETAIL TYPICAL N.T.S.

REINFORCING BAR LAP SPlice TABLE (BEAMS AND COLUMNS)

BAR SIZE	POSITION	CONCRETE f'c (PSI) AND LAP CLASS					
		3000 B	4000 B	5000 B	6000 B		
#3 thru #6	ALL	74db	64db	58db	50db		
#7 thru #11	ALL	93db	80db	72db	60db		

REINFORCING BAR LAP SPlice TABLE (SLABS AND WALLS)

BAR SIZE	POSITION	CONCRETE f'c (PSI) AND LAP CLASS					
		3000 B	4000 B	5000 B			
#3 thru #6	0.75" COVER 2.0" COVER	75db 46db	64db 40db	58db 40db			
#7 thru #11	0.75" COVER 2.0" COVER	138db 74db	120db 65db	106db 56db			

REBAR LAP SPlice TABLE NOTES:

RL-1 "db" DENOTES BAR DIAMETER.

RL-2 ALL SPLICES SHALL BE CLASS B UNLESS OTHERWISE NOTED.

RL-3 VALUES APPLY TO ALL BARS WITH MINIMUM CONCRETE COVER 1.0db AND MINIMUM CENTER TO CENTER SPACING OF 2.0db.

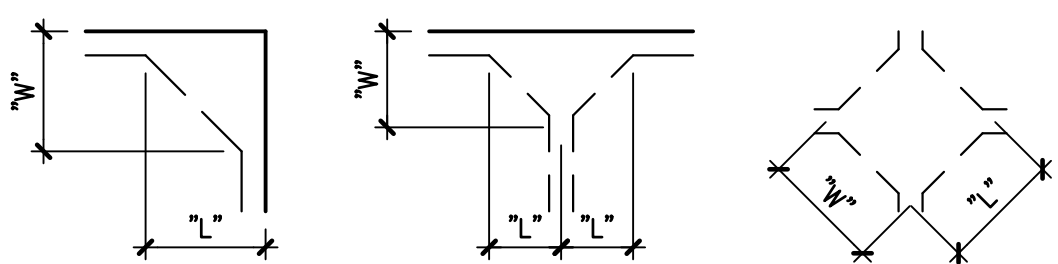
RL-4 FOR LIGHTWEIGHT CONCRETE, MULTIPLY BY 1.3.

RL-5 THE CHART ABOVE IS A SIMPLIFIED AND CONSERVATIVE METHOD FOR MEETING THE REQUIREMENTS OF ACI 12.2.2. THE CONTRACTOR MUST SUBMIT A DETAILED REBAR SPLICING PLAN IN ACCORDANCE WITH ACI 12.2.2 FOR APPROVAL.

FOOTING SCHEDULE

MARK	DIMENSION			REINFORCING
	LENGTH	WIDTH	DEPTH	
F1	5'-0"	5'-0"	SEE NOTE	#5 @ 10"o.c. EACH WAY TOP & BOTTOM LAYER
F2	4'-0"	4'-0"	SEE NOTE	#5 @ 10"o.c. EACH WAY TOP & BOTTOM LAYER

NOTE: INTEGRAL SPREAD FOOTING SAME DEPTH AS DEEPEST BEAM



EXPANSION ANCHORS NOTES:

EA-1 ALL EXPANSION ANCHORS SHALL BE 3/4" DIAMETER UNLESS NOTED OTHERWISE WITH A MINIMUM EMBEDMENT DEPTH OF 4 1/8", UNLESS SPECIFICALLY NOTED OTHERWISE ON THE DRAWINGS. OTHER DIAMETERS INDICATED IN SECTIONS OR DETAILS SHALL COMPLY WITH THE MINIMUM EMBEDMENT DEPTHS INDICATED IN THE SCHEDULE BELOW.

EA-2 EXPANSION ANCHORS SHALL BE A STUD BOLT TYPE WITH HEX HEAD NUT AND SHALL BE ZINC PLATED.

EA-3 ANCHORS SHALL BE STRONG-BOLT 2 WEDGE ANCHOR (ICC-ES ESR 3037) BY SIMPSON STRONG-TIE CO., INC. OR HILTI KWIK BOLT III (ICC-ES ESR 2302).

EA-4 ANCHORS SHALL BE INSTALLED AND TORQUED IN COMPLETE ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS.

EXPANSION ANCHOR

DIAMETER	MIN. EMBEDMENT	REMARKS
1/2"	2 3/4"	
5/8"	3 3/8"	
3/4"	4 1/8"	SEE NOTE EA-1

CONCRETE NOTES:

CN-1 CONCRETE SHALL BE LABORATORY DESIGNED TO DEVELOP MINIMUM 28-DAY COMPRESSIVE STRENGTHS AS GIVEN BELOW. REFER TO SPECIFICATIONS FOR AGGREGATES, CEMENT, ADMIXTURES, ETC.

GRADE BEAMS, SLABS-ON-GRADE 3,000 PSI

NOTE: FLY ASH WILL BE PERMITTED UP TO 20% PORTLAND CEMENT REPLACEMENT, REFER TO SPECIFICATIONS.

CN-2 REINFORCING STEEL SHALL BE FROM NEW BILLET AND SHALL CONFORM TO THE FOLLOWING ASTM SPECIFICATIONS:

A615-GR 60 FOOTING SPIRALS
A185 WELDED WIRE FABRIC
A615-GR 60 BEAM STIRRUPS, COLUMN TIES
A615-GR 60 ALL OTHER REINFORCING
ASTM A108-60T HEADED CONCRETE ANCHORS
ASTM A496 DEFORMED BAR ANCHORS

CN-3 DETAILING OF CONCRETE REINFORCEMENT BARS AND ACCESSORIES SHALL BE IN ACCORDANCE WITH LATEST ACI MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES (ACI 315). BAR SUPPORTS SHALL HAVE PLASTIC COATED LEGS OR BE HOT DIPPED GALVANIZED AFTER FABRICATION.

CN-4 PROVIDE BAR LAPS AND SPLICES PER REINFORCING BAR LAP SPlice TABLE BELOW. SEE "CORNER DETAILS" FOR CONTINUOUS BARS AT CORNERS. SPIRALS SHALL BE LAPPED 1-1/2 TURNS. WELDED WIRE MESH SHALL BE LAPPED 8" MINIMUM AT SPlice POINTS, OR 1-1/2 MESHES, WHICHEVER IS GREATEST.

CN-5 CONTRACTOR SHALL PROVIDE NECESSARY CONSTRUCTION JOINTS IN MONOLITHIC CONCRETE FORMING SO THAT NOT MORE THAN 400 CUBIC YARDS IS POURED IN ONE DAY. LOCATION OF CONSTRUCTION JOINTS MUST HAVE PRIOR APPROVAL OF STRUCTURAL ENGINEER OF RECORD AND SHALL GENERALLY BE LOCATED AT OR NEAR MID-POINTS OF SPANS OF SLAB, BEAMS AND WALLS. ALL CONTINUOUS REINFORCING SHALL BE CARRIED THROUGH THE JOINT. SEE DETAILS FOR CONTINUOUS KEY BETWEEN ADJACENT POURS.

CN-6 SEE ARCHITECTURAL, MECHANICAL, PLUMBING AND ELECTRICAL DRAWINGS FOR LOCATION AND SIZES OF ALL SLAB OPENINGS AND SLEEVES, INSERTS, ANCHORS AND BOLTS REQUIRED BY ABOVE.

CN-7 REFER TO ARCHITECTURAL DRAWINGS FOR ALL FLOOR FINISHES, DIMENSIONS AND LOCATIONS OF SLAB DROPS AND DEPRESSIONS.

CN-8 MECHANICAL AND ELECTRICAL CONDUITS IN SLABS SHALL RUN UNDER THE TOP LAYER OF SLAB REINFORCING OR WELDED WIRE FABRIC. PROVIDE A MINIMUM OF 1-1/2" CLEAR BETWEEN INDIVIDUAL CONDUITS, AND BETWEEN CONDUIT AND PARALLEL REINFORCING. DO NOT "BUNDLE" CONDUITS.

CN-9 "HEADED CONCRETE ANCHORS" (HCA) SHALL BE OF 50,000 PSI STEEL ROD WITH UPSET ENDS, AUTOMATICALLY ARC WELDED THROUGH CERAMIC FERRULES, "NELSON CONCRETE ANCHORS" OR EQUAL.

MECHANICAL TESTING OF HCA IN SHOP

MECHANICAL TESTS SHALL BE MADE BEFORE INITIATION OF PRODUCTION WELDING AND AFTER ANY EQUIPMENT MAINTENANCE TO ENSURE THAT THE WELDING SCHEDULE IS SATISFACTORY. THEY MAY ALSO BE MADE DURING THE PRODUCTION RUN OR AT THE BEGINNING OF A SHIFT TO ENSURE THAT WELDING CONDITIONS HAVE NOT CHANGED. ARC WELDED STUDS ARE TESTED BY BENDING THE STUD. BENDING MAY BE DONE BY STRIKING THE STUD WITH A HAMMER OR BY BENDING IT USING A TUBE OR PIPE, THE ANGLE THROUGH WHICH THE STUD WILL BEND WITHOUT WELD FAILURE WILL DEPEND ON THE STUD AND BASE METAL COMPOSITIONS, CONDITIONS (COLD WORKED, HEAT TREATED), AND STUD DESIGN. ACCEPTABLE BENDING SHOULD BE DETERMINED WHEN THE WELDING PROCEDURE SPECIFICATION IS ESTABLISHED OR FROM THE APPLICABLE WELDING CODE. BEND TESTING MAY DAMAGE THE STUD; THEREFORE, IT SHOULD BE DONE ON QUALIFICATION SAMPLES ONLY. THE METHOD USED TO APPLY TENSILE LOAD ON AN ARC WELDED STUD WILL DEPEND ON THE STUD DESIGN. SPECIAL TOOLING MAY BE REQUIRED TO GRIP THE STUD PROPERLY WITHOUT DAMAGE, AND A SPECIAL LOADING DEVICE MAY BE NEEDED.

MECHANICAL TESTING OF HCA IN FIELD

MECHANICAL TESTS SHALL BE MADE IN THE FIELD BEFORE PLATES ARE INSTALLED IN CONCRETE. THE CONTRACTOR SHALL SUPPLY AT A MINIMUM ONE ADDITIONAL PER 50 PLATES OF EACH TYPE OR ADDITIONAL STUDS SHALL BE PLACED ON SPECIAL CONFIGURATION PLATES AND MEMBERS, THESE STUDS SHALL BE TESTED IN THE FIELD. ARC WELDED STUDS ARE TESTED BY BENDING THE STUD. BENDING MAY BE DONE BY STRIKING THE STUD WITH A HAMMER OR BY BENDING IT USING A TUBE OR PIPE, THE ANGLE THROUGH WHICH THE STUD WILL BEND WITHOUT WELD FAILURE WILL DEPEND ON THE STUD AND BASE METAL COMPOSITIONS, CONDITIONS (COLD WORKED, HEAT TREATED), AND STUD DESIGN. BEND TESTING MAY DAMAGE, THUS THEY MAY NOT BE USED. THE STUD; THEREFORE, IT SHOULD BE DONE ON QUALIFICATION SAMPLES ONLY. THE METHOD USED TO APPLY TENSILE LOAD ON AN ARC WELDED STUD WILL DEPEND ON THE STUD DESIGN, PROPERLY WITHOUT DAMAGE, AND A SPECIAL LOADING DEVICE MAY BE NEEDED.

CN-10 REFER TO SPECIFICATIONS FOR TESTING REQUIREMENTS. ALL TESTING SHALL BE AT POINT OF DISCHARGE. IF PUMP IS USED, TESTING SHALL BE AT THE END OF THE HOSE.

GENERAL NOTES:

GN-1 THIS STRUCTURE IS DESIGNED IN ACCORDANCE WITH THE INTERNATIONAL BUILDING CODE (2018) AS AMENDED AND ADOPTED BY THE GOVERNING AUTHORITY, AND APPLICABLE INDUSTRY STANDARDS (AISC, ACI, ETC.).

GN-2 THE DESIGN LOADS ARE:

SUPERIMPOSED DEAD LOADS 5 PSF
MECHANICAL DUCTS/CONDUITS, CEILING, ETC. AS INDICATED ON PLANS
MECHANICAL EQUIPMENT AS INDICATED ON PLANS

FLOOR LIVE LOAD 100 PSF
CORRIDOR 50 PSF
OFFICES 20 PSF
MOVEABLE PARTITIONS 150 PSF
MECHANICAL ROOMS 125 PSF
(NON REDUCIBLE)
CATWALKS 40 PSF

ASSEMBLY AREAS:
FIXED SEATS 60 PSF
LOBBIES 100 PSF
MOVEABLE SEATS 100 PSF
STAGES & PLATFORMS 125 PSF
CATWALKS 40 PSF

ROOF LIVE LOAD 20 PSF
FLAT ROOF 20 PSF
PITCHED ROOF 20 PSF

ROOF SNOW LOAD 5 PSF
GROUND SNOW Pg 1.0
SNOW EXPOSURE FACTOR Ce 1.1
SNOW LOAD IMPORTANCE FACTOR Is 1.0
THERMAL FACTOR Ct 1.0

WIND LOAD 120
BASIC WIND SPEED (ULTIMATE DESIGN) III
BUILDING CATEGORY C
WIND EXPOSURE ±0.18
INTERNAL PRESSURE COEF. 25 PSF
COMPONENTS AND CLADDING WIND PRESSURE 25 PSF

EARTHQUAKE LOADS
SEISMIC IMPORTANCE FACTOR Ie 1.00
SPECTRAL RESPONSE ACCELERATION Ss 14%
SPECTRAL RESPONSE ACCELERATION S 3%
SPECTRAL RESPONSE COEF. Sds 14%
SPECTRAL RESPONSE COEF. SD 5%
SEISMIC DESIGN CATEGORY A
SEISMIC RESPONSE COEF Cs 01

RETAINING WALLS
GLOBAL STABILITY ANALYSIS FACTOR OF SAFETY 1.5
TYPE CANTILEVER
EQUIVALENT FLUID PRESSURE 50 PCF
BACKFILL DRAINED/ONSITE
FOOTING BEARING 1500 PSF
SURCHARGE 200 PSF

FLOOD LOAD
ELEVATION OF LOWEST FLOOR REF. ARCH. DWGS.

GN-3 ALLOWABLE STRESS DESIGN LOAD COMBINATIONS (FOR ALL DESIGNS EXCEPT CONCRETE)

D
D+L
D+(Lr, or S or R)
D+0.75L+0.75(Lr, or S or R)
D+(0.6W)
D+0.75L+0.75(0.6W)+0.75(Lr or S or R)
0.6D+0.6W
D+0.7E

STRENGTH DESIGN LOAD COMBINATIONS (FOR CONCRETE DESIGN)

1.4D
1.2D+1.6L+0.5(Lr, or S or R)
1.2D+1.6(Lr, or S or R)+0.5W
1.2D+1.0W+L+0.5(Lr, or S or R)
0.9+1.0W
1.2D+E+L+0.2S

GN-4 PRIOR TO START OF CONSTRUCTION, THE CONTRACTOR AND FABRICATOR SHALL VERIFY ALL QUANTITIES, DIMENSIONS AND CONDITIONS AND NOTIFY ARCHITECT/STRUCTURAL ENGINEER OF RECORD OF ANY DISCREPANCIES BEFORE PROCEEDING WITH THE WORK.

GN-5 UTILITIES PENETRATING BUILDING SHALL BE FLEXIBLE, USING SLEEVE JOINTS, BENDS, LOOPS, ETC. TO PERMIT MOVEMENTS DUE TO EXPANSIVE UNDERLYING SOILS.

GN-6 PROVIDE ADEQUATE AND APPROPRIATE STRUCTURAL STEEL FRAMING FOR THE SUPPORT AND MOUNTING OF MECHANICAL EQUIPMENT RESTING ON, OR SUSPENDED FROM, STEEL SUPERSTRUCTURE.

GN-7 THE STRUCTURAL DRAWINGS FOR THIS PROJECT ARE COPYRIGHTED AND SHALL NOT BE REPRODUCED FOR USE AS FABRICATOR'S ERECTION DRAWINGS. THE CONTRACTOR SHALL ALLOW ADEQUATE TIME AND EXPENSE FOR SUBCONTRACTORS TO PRODUCE THEIR OWN ORIGINAL ERECTION AND PLACEMENT DRAWINGS.

GN-8 THE STRUCTURE HAS BEEN DESIGNED TO RESIST DESIGN LOADS ONLY AS A COMPLETED STRUCTURE. ANY PROPOSED APPLICATION OF CONSTRUCTION LOADS OR OF ANY LOADS TO THE PARTIALLY COMPLETED STRUCTURE WHICH EXCEED THE DESIGN LOADS WILL REQUIRE REANALYSIS AND PROBABLE REDESIGN.

GN-9 PROVIDE 5.0 TONS OF EXTRA REINFORCING STEEL, DETAILING, LABOR FOR PLACING AND FABRICATION AS DIRECTED IN THE FIELD AND SHOP.

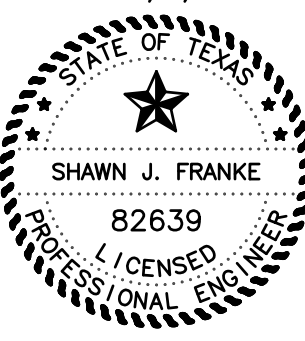
GN-10 PROVIDE 10.0 TONS OF EXTRA STRUCTURAL STEEL, DETAILING, LABOR FOR ERECTION AND FABRICATION AS DIRECTED IN THE FIELD AND SHOP.

CONTRACTOR NOTE

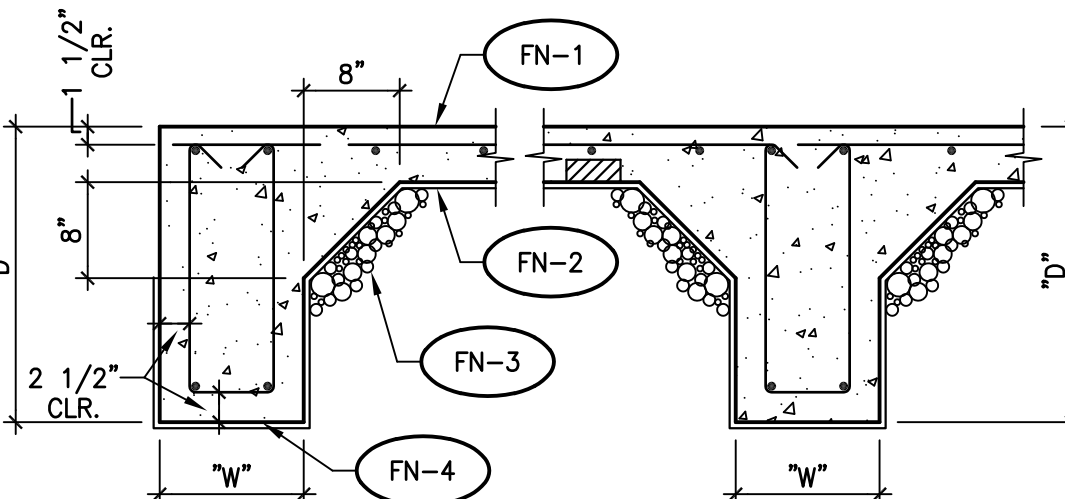
THE STRUCTURAL SYSTEM FOR THIS PROJECT SHALL NOT BE CONSTRUCTED BY USING THE STRUCTURAL DRAWINGS ALONE. THESE DRAWINGS WERE DEVELOPED FROM DATA DERIVED PRIMARILY FROM THE ARCHITECTURAL DRAWINGS AND SECONDARILY FROM MEP, CIVIL AND OTHER DISCIPLINES' DOCUMENTS. IT IS INTENDED THAT CONSTRUCTION PROCEED BY UTILIZING ALL OF THE INFORMATION CONTAINED IN THE ENTIRE SET OF CONSTRUCTION DOCUMENTS TAKEN AS A WHOLE. FAILURE TO DO SO WILL RESULT IN ERRORS WHICH SHALL BE CORRECTED AT THE CONTRACTOR'S EXPENSE.



DATE:02/05/2021



Shawn Franke



1 SECTION N.T.S. 2 SECTION N.T.S.

GRADE BEAM SCHEDULE

MARK	W x D	MAIN REINFORCING	TIES
GB1	12 x 24	2-#6 x CONT. TOP & BOTTOM	#3 @ 18"o.c.
GB2	24 x 24	4-#6 x CONT. TOP & BOTTOM	#3 @ 18"o.c.

FOUNDATION NOTES:

FN-1 5" CONCRETE SLAB REINFORCED W/ #4 @ 12"o.c. EACH WAY IN TOP. SUPPORT AT 4'-0"o.c. EACH WAY WITH CONCRETE BLOCKS OR BRICKS. SUPPORT BOTTOM BEAM REINFORCEMENT AT 4'-0" INTERVALS.

FN-2 15 MIL. POLYETHYLENE LINER OR EQUAL UNLESS NOTED OTHERWISE IN SPECIFICATIONS. FOR SOFFIT TO BEAR 12" MINIMUM BELOW FINISH GRADE.

FN-3 COMPACTED SELECT FILL (SEE UF-6 "UNDERFLOOR FILL NOTES").

FN-4 ALL BEAM SOFFITS SHALL BEAR 12" MINIMUM INTO NATURAL GRADE OR COMPACTED FILL. ON PERIMETER, INCREASE SCHEDULED BEAM DEPTH AS REQUIRED FOR SOFFIT TO BEAR 12" MINIMUM BELOW FINISH GRADE.

FN-5 GRADE BEAMS AND SLAB TURNDOWNS SHALL BE FORMED BY WALLS AND SOFFIT OF CAREFULLY SHAPED TRENCH. USE A SMOOTH-MOUTHED BUCKET. IF A TOOTHED BUCKET IS USED, EXCAVATION SHALL BE STOPPED 6" ABOVE FINAL GRADE AND THE REMAINING EXCAVATION ACCOMPLISHED WITH A SMOOTH MOUTHED BUCKET OR BY HAND LABOR TO REMOVE ALL LOOSE SOILS DISTURBED BY THE BUCKET TEETH. WOODFORM EXPOSED FACES TO A DEPTH OF 8" BELOW FINISHED GRADE.

FN-6 AT ALL BEAM CORNERS & T-INTERSECTIONS, PROVIDE 4-#6 X 6'-0" CORNER BARS (2-TOP AND 2-BOTTOM).

FN-7 TRENCHES SHALL BE VERIFIED FOR SIZE TO MAINTAIN CLEARANCES AROUND REINFORCEMENT PRIOR TO PLACING REINFORCEMENT.

FN-8 WHERE BEAM DEPTH EXCEEDS 36", ADD #4 @ 12"o.c. IN EACH FACE OF BEAM.

UNDERFLOOR FILL NOTES:

UF-1 BEFORE ANY CONSTRUCTION IS BEGUN, PERFORM ROUGH GRADING AND CUT SWALES SO THAT GROUNDS WILL DRAIN AWAY FROM THE BUILDING. MAINTAIN DRAINAGE DURING ALL PHASES OF CONSTRUCTION SO THAT STORM WATER WILL BE CONDUCTED AWAY FROM THE BUILDING. KEEP EXCAVATIONS PUMPED FREE OF STORM WATER AT ALL TIMES.

UF-2 PRECAUTIONS SHALL BE TAKEN TO PROTECT OPEN EXCAVATIONS FROM EXCESSIVE LOSS OR GAIN IN NATURAL MOISTURE LEVEL PRIOR TO PLACEMENT OF BASE MATERIAL. KEEP MOIST DURING DRY WEATHER AND KEEP STORM WATER PUMPED OUT, INCLUDING NIGHTS AND WEEKENDS, DURING RAINS.

UF-3 IN THE AREA OCCUPIED BY THE FOUNDATION AND ALL ADJACENT SIDEWALKS, PLUS 3'-0". REMOVE A MINIMUM OF 5'-0" OF TOPSOIL INCLUDING ALL ORGANIC MATERIALS, ROOTS, ETC. FROM THE SITE. DO NOT USE FOR UNDERFLOOR FILL. REMOVE ADDITIONAL MATERIAL AS NECESSARY TO PROVIDE A MINIMUM OF 7'-0" OF SELECT FILL AS PER UF-6.

UF-4 THE RESULTING SURFACE SHALL BE PROOF ROLLED WITH A SUFFICIENTLY HEAVY ROLLER (15 TONS) TO LOCATE AND DENSITY WEAK AND COMPRESSIBLE ZONES. A MINIMUM OF 6 PASSES OF THE ROLLER IS REQUIRED. ANY SOFT SPOTS SHALL BE REMOVED AND REPLACED WITH COMPACTED SELECT FILL.

UF-5 THE ROLLED SUBGRADE SHALL BE SCARIFIED JUST PRIOR TO FILL PLACEMENT TO A MINIMUM DEPTH OF 6" AND RECOMPACTED TO MINIMUM OF 95% OF THE MAXIMUM DENSITY DETERMINED BY ASTM D698 COMPACTION TEST, MAINTAINING MOISTURE CONTENT BETWEEN -1 AND +3 PERCENTAGE POINTS UNTIL COVERED.

UF-6 FOR A DISTANCE OF 3'-0" OUTSIDE OF THE BUILDING LINE AND ALL ADJACENT SIDEWALKS, AND BEGINNING AT THE LOW END, BUILD UP TO THE ELEVATION OF THE BOTTOM OF THE SLAB WITH SELECT CRUSHED STONE FILL CONFORMING TO TxDOT SPECIFICATIONS, ITEM 247, TYPE "A" GRADE 2. A MINIMUM THICKNESS OF 5'-0" IS REQUIRED. NO DIRT FILL SHALL BE USED UNDER THE BUILDING FOUNDATION. SUBMIT WRITTEN CERTIFICATION OF COMPLIANCE WITH TxDOT, ITEM 247 SPECIFICATIONS BY TEST PERFORMED ON FIELD EXAMPLES.

UF-7 ALL FILL SHALL BE PLACED IN 8" LOOSE HORIZONTAL LIFTS AND COMPACTED TO A MINIMUM OF 95% OF THE MAXIMUM DENSITY AS DETERMINED BY ASTM D698 COMPACTION TEST, MAINTAINING MOISTURE CONTENT BETWEEN -1 AND +3 PERCENTAGE POINTS UNTIL COVERED. EXCESS FILL AT BUILDING PERIMETER SHALL BE CUT AND GRADED TO COMPLY WITH FINISHED GRADE REQUIREMENTS.

UF-8 PERFORM ALL EARTH WORK DESCRIBED ABOVE BEFORE TRENCHING FOR GRADE BEAMS OR MECHANICAL LINES.

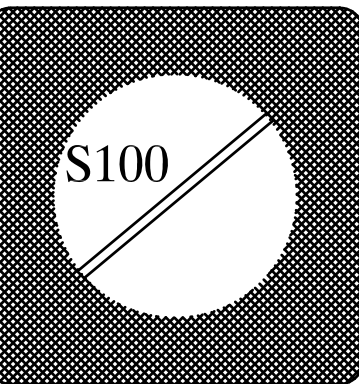


Date: Revisions/Submissions

CITY OF SAN ANTONIO
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NOTES, SECTIONS & DETAILS



Designed By:

J.H.

Drawn By:

S.J.F.

Date:

02/05/2021

Project No.:

00-000-00

Filename:

3. CONCRETE CONSTRUCTION CONT.:				
L. REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS.	PERIODIC	VERIFY IN-SITU CONCRETE STRENGTH PRIOR TO REMOVAL.	ACI 318--CH. 5.11, 5.13	*QUALIFICATIONS BASED ON ASTM E329
M. POST INSTALLED REINFORCING & ANCHORS (EXPANSION ANCHORS, SCREW ANCHORS, ADHESIVE ANCHORS, ECT.).	CONTINUOUS	THE SPECIAL INSPECTOR SHALL BE ON THE JOB SITE CONTINUOUSLY DURING ANCHOR INSTALLATION TO VERIFY ANCHOR TYPE, ANCHOR DIMENSIONS, CONCRETE TYPE AND COMPRESSION STRENGTH, PRE-DRILLED HOLE DIMENSIONS, ANCHOR SPACING, EDGE DISTANCES, CONCRETE THICKNESS AND ANCHOR EMBEDMENT.	ACI 318 APPENDIX D--CH. D.9.1	*QUALIFICATIONS BASED ON ASTM E329 & ASTM C1077 OR CERTIFIED MANUFACTURER REPRESENTATIVE
4. STEEL CONSTRUCTION			IBC 1705.2	
A. MATERIAL VERIFICATION OF HIGH-STRENGTH BOLTS, NUTS AND WASHERS:	PERIODIC	1. IDENTIFICATION MARKINGS TO CONFORM TO ASTM STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS.	STRUCTURAL STEEL GENERAL NOTES	CWI/ASSOCIATE/TECHNICAL RADIATE, AWS OR CRSI
	PERIODIC	2. MANUFACTURER'S CERTIFICATE OF COMPLIANCE REQUIRED.	APPLICABLE ASTM SPECIFICATIONS; AISC 335, SECTION A3.4; AISC LRFD, SECTION A3.3	
B. HIGH STRENGTH BOLTING:	PERIODIC	1. BEARING--TYPE CONNECTIONS.	IBC 1704.3.3; STRUCTURAL STEEL GENERAL NOTES	CWI/ASSOCIATE/TECHNICAL RADIATE, AWS OR CRSI
	CONTINUOUS OR PERIODIC	2. SLIP--CRITICAL CONNECTIONS.	AISC LRFD SECTION M2.5	
C. MATERIAL VERIFICATION OF STRUCTURAL STEEL:	PERIODIC	1. IDENTIFICATION MARKINGS TO CONFORM TO ASTM STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS.	IBC 1705.2; STRUCTURAL STEEL GENERAL NOTES	CWI/ASSOCIATE/TECHNICAL RADIATE, AWS OR CRSI
	PERIODIC	2. MANUFACTURERS' CERTIFIED MILL TEST REPORTS.	ASTM A 6 OR ASTM A 568	
D. MATERIAL VERIFICATION OF WELD FILLER MATERIALS:	PERIODIC	1. IDENTIFICATION MARKINGS TO CONFORM TO AWS SPECIFICATION IN THE APPROVED CONSTRUCTION DOCUMENTS.	STRUCTURAL STEEL GENERAL NOTES	CWI/ASSOCIATE/TECHNICAL RADIATE, AWS OR CRSI
	PERIODIC	2. MANUFACTURERS' CERTIFIED OF COMPLIANCE REQUIRED.	AISC, ASD, SECTION A3.6; AISC LRFD, SECTION A3.5	
E. WELDING: OF STRUCTURAL STEEL:	N/A	1. COMPLETE & PARTIAL PENETRATION GROOVE WELDS.	IBC 1705.2.2.1; STRUCTURAL STEEL GENERAL NOTES	CWI AND ASNT
	N/A	2. MULTIPASS FILLET WELDS.	AWS D1.1	CWI AND ASNT OR LICENSED ENGINEER
	N/A	3. SINGLE-PASS FILLET WELDS > 5/16"		
	N/A	4. SINGLE-PASS FILLET WELDS ≤ 5/16"		
	N/A	5. FLOOR AND DECK WELDS.	AWS D1.3	
F. WELDING OF REINFORCING STEEL:	N/A	1. VERIFICATION OF WELD ABILITY OF REINFORCING STEEL OTHER THAN A706.	IBC 1705.2.2.1.2	CWI/ASSOCIATE/TECHNICIAN TRAINED IN FIELD OF WORK AND HAS AT LEAST ONE YEAR OF EXPERIENCE.
	N/A	2. REINFORCING STEEL--RESISTING FLEXURAL AND AXIAL FORCES IN INTERMEDIATE AND SPECIAL MOMENT FRAMES, AND BOUNDARY ELEMENTS OF SPECIAL REINFORCED CONCRETE SHEAR WALLS AND SHEAR REINFORCEMENT.		
	N/A	3. SHEAR REINFORCEMENT.		
	N/A	4. OTHER REINFORCING STEEL.		

DEFERRED SUBMITTALS			
BUILDING CONSTRUCTION	YES	NO	DESCRIPTION
STEEL	X		CANOPY STRUCTURE
CONCRETE		X	--
WOOD		X	--

2B. PIER FOUNDATIONS				
A. THE GEOTECHNICAL ENGINEER OR A QUALIFIED E.I.T. INVOLVED IN THE ORIGINAL GEOTECHNICAL INVESTIGATION AND UNDER THE DIRECT SUPERVISION OF THE GEOTECHNICAL ENGINEER SHALL BE PRESENT DURING THE EXCAVATION OF THE FIRST PIER SHAFT.	N/A	1. VERIFY THE BEARING STRATUM IS ENCOUNTERED AT THE ANTICIPATED DEPTH. 2. ADDRESS UNFORESEEN SUBSURFACE CONDITIONS, IF ANY. 3. VERIFY CONFORMANCE WITH THE FOUNDATION RECOMMENDATIONS PROVIDE IN THE PROJECT "GEOTECHNICAL ENGINEERING STUDY" AND THE STRUCTURAL DRAWINGS ISSUED FOR THE PROJECT.	IBC 1705.8 GEOTECHNICAL REPORT;	GRADUATE ENGINEER *QUALIFICATIONS BASED ON ASTM E329 & ASTM C1077
B. ALL FOOTINGS SHALL BE OBSERVED AND MONITORED BY A REPRESENTATIVE OF THE GEOTECHNICAL ENGINEER. THE CONTRACTOR SHALL PROVIDE THE GEOTECHNICAL ENGINEER WITH A COMPLETE SET OF STRUCTURAL DRAWINGS THAT ARE TO REMAIN WITH THE GEOTECHNICAL ENGINEER OR HIS REPRESENTATIVE.	N/A	1. PROVIDE RECORD OF EACH PIER INSTALLED. 2. RECORD LOAD TESTS, CUTOFF AND TIP OF EACH PIER.	IBC 1705.8 GEOTECHNICAL REPORT;	*QUALIFICATIONS BASED ON ASTM E329 & ASTM C1077
3. CONCRETE CONSTRUCTION				
A. REINFORCING STEEL	PERIODIC	PROVIDE PERIODIC INSPECTION OF REINFORCING SIZES, SPACING, GRADE OF REBAR; AND PLACEMENT AT THE FOLLOWING FREQUENCY: COLUMNS: 10% BEAMS: 30% JOIST: 10% OTHER MEMBERS: RANDOMLY @ 20%	IBC 1704.4 ACI 318: CH. 3.5, 7.1--7.7; CONCRETE AND REINFORCING GENERAL NOTES.	*QUALIFICATIONS BASED ON ASTM E329
B. REINFORCING STEEL WELDING	N/A	NO FIELD WELDING PERMITTED.	AWS D1.4 ACI 318: 3.5.2	CWI OR ASSOCIATE CWI
C. BOLTS TO BE INSTALLED IN CONCRETE PRIOR TO & DURING PLACEMENT OF CONCRETE WHERE ALLOWABLE LOADS HAVE BEEN INCREASED.	CONTINUOUS	VERIFY LOCATION, SIZE AND SPACING OF ANCHORS.	IBC 1705.3	**TECHNICIAN TRAINED IN FIELD OF WORK AND HAS AT LEAST ONE YEAR EXPERIENCE.
D. ANCHORS TO BE INSTALLED IN EXISTING CONCRETE	CONTINUOUS	VERIFY LOCATION, SIZE AND SPACING OF ANCHORS.	IBC 1705.3	**TECHNICIAN TRAINED IN FIELD OF WORK AND HAS AT LEAST ONE YEAR EXPERIENCE.
E. VERIFY USE OF CONCRETE MIX DESIGN	PERIODIC	EACH CONCRETE POUR.	ACI 318--CH. 4, 5.2--5.4	*QUALIFICATIONS BASED ON ASTM C1077
F. SAMPLING OF FRESH CONCRETE.	CONTINUOUS EACH CONCRETE POUR;	1. ALL CONCRETE TESTING IS TO BE MADE AFTER WATER, IF ANY, IS ADDED AT SITE. 2. TAKE SAMPLES & PERFORM SLUMP, AIR & COMPRESSION TESTS IN ACCORDANCE WITH ASTM C-39 ON CONCRETE PLACED EACH DAY AT THE RATE OF ONE SET OF FOUR CYLINDERS FOR EACH 80 cu. yds. OR FRACTION THEREOF. WHEN MORE THAN 80 cu. yds. IS BEING CONTINUOUSLY PLACED, THE INTERVAL BETWEEN TEST SAMPLES SHALL BE AT LEAST 50 cu. yds. SO AS TO BE REPRESENTATIVE OF THE WHOLE DAYS POUR. SAMPLES SHALL BE TAKEN AT THE THE POINT OF DEPOSIT IN THE FIELD & ALL CYLINDERS SHALL BE ACCURATELY MARKED & REFERENCED TO SHOW DATE, TIME & EXACT LOCATION IN THE STRUCTURE FROM WHICH THEY CAME. MAKE 7--DAY TEST ON TWO CYLINDERS & 28--DAY TEST ON TWO CYLINDERS. REPORT OF TESTS SHALL BE PROMPTLY SENT AS FOLLOWS: TWO TO THE PDPIRC (ARCHITECT), ONE TO THE ENGINEER AND ONE TO THE CONTRACTOR.	ACI 318--CH. 5.6, 5.8	*QUALIFICATIONS BASED ON ASTM C1077
G. PLACEMENT OF CONCRETE & SHOTCRETE.	CONTINUOUS		ACI 318--CH. 5.9, 5.10	*QUALIFICATIONS BASED ON ASTM C1077
H. MAINTENANCE OF SPECIFIED CURING TEMPERATURE & TECHNIQUES.	PERIODIC	EACH CONCRETE POUR	ACI 318--CH. 5.11, 5.13	*QUALIFICATIONS BASED ON ASTM C1077
I. PRE-STRESSED CONCRETE	N/A	1. APPLICATION OF PRESTRESSING FORCE. 2. GROUTING OF BOUNDED PRESTRESSING TENDONS IN SEISMIC--FORCE RESISTING SYSTEMS.		*QUALIFICATIONS BASED ON ASTM C1077
J. ERECTION OF PRECAST CONCRETE MEMBERS.	N/A			TECHNICIAN TRAINED IN FIELD OF WORK AND HAS AT LEAST ONE YEAR OF EXPERIENCE.
K. POST--TENSIONED CONCRETE:	N/A	1. VERIFY IN-SITU CONCRETE STRENGTH PRIOR TO STRESSING OF TENDONS.		*QUALIFICATIONS BASED ON ASTM E329
	N/A	2. THE POST--TENSIONING ENGINEER, OR A MEMBER OF HIS STAFF, SHALL INSPECT THE TENDON PLACEMENT AND CHAIRING TO INSURE COMPLIANCE WITH THE INTENT OF THE DESIGN.		
	N/A	3. CONTINUOUS INSPECTION IS REQUIRED DURING ALL STRESSING ACTIVITIES.		
	N/A	4. RECORDS OF ALL JACKING FORCES AND ELUNGATIONS SHALL BE MADE IN ACCORDANCE WITH THE PTI FIELD MANUAL AND RECORDS SHALL BE PROMPTLY SUBMITTED TO THE ARCHITECT AND ENGINEER.		

NOTES:

1 THESE INSPECTIONS DO NOT RELIEVE ENGINEER FROM STRUCTURAL OBSERVATIONS AS MAY REQUIRED BY IBC 2018, SECTION 1709, AND/OR CONTRACTUAL REQUIREMENTS OF ARCHITECT/CLIENT, (I.E. C141).

2 DEFINITIONS/TERM: PERIODIC VS. CONTINUOUS INSPECTIONS -- REF. IBC SECTION 1702
ADSC -- THE INTERNATIONAL ASSOCIATION OF FOUNDATION DRILLING
ASNT -- AMERICAN SOCIETY FOR NONDESTRUCTIVE TESTING
ASTM -- AMERICAN SOCIETY FOR TESTING MATERIALS
AWS -- AMERICAN WELDING SOCIETY
CWI -- CERTIFIED WELDING INSPECTOR
CRSI -- CONCRETE REINFORCING STEEL INSTITUTE
PCI -- PRECAST/PRESTRESSED CONCRETE INSTITUTE
PTI -- POST--TENSIONING INSTITUTE
N/A -- NOT APPLICABLE

*TESTING AND INSPECTION DIRECTED BY ASTM E329 GUIDELINES.

Pursuant to IBC Chapter 17 (1704.2.1) provide the following Special Inspector Qualifications to the RDPIRC prior to start of inspections;

- Testing Laboratory Qualifications meeting ASTM0329 and accreditation by AASHTO and/or A2LA, and CCRL of the National Bureau of Standards.
- Special Inspector's name and proof of meeting the qualification requirements set forth in
 - ASTM C1077 for concrete,
 - ASTM D3740 for soils,
 - ASTM C1093 for masonry.
 - ASTM D-2922 and D-3017 for Density control of compaction

IBC 1704.2.1 "written documentation demonstrating the competence and relevant experience or training of special inspectors who will perform special inspections and tests during construction. Experience or training shall be considered relevant where the documented experience or training is related in complexity to the same type of *special inspection* or testing activities for projects of similar complexity and material qualities." These qualifications are in addition to qualifications specified in other sections of the IBC.

TESTING & INSPECTION REQUIREMENTS
(INCLUDING SPECIAL INSPECTIONS)

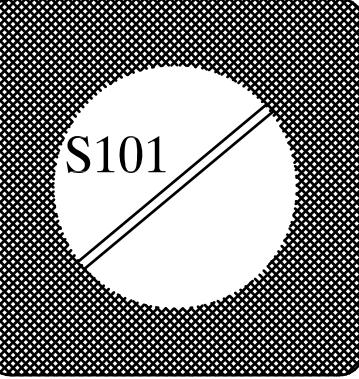
REQUIRED INSPECTION VERIFICATION, OR TEST	VERIFICATION MONITORING FREQUENCY	TYPE AND/OR FREQUENCY OF TESTING	IBC SECTION & REFERENCE CRITERIA	INSPECTOR QUALIFICATIONS
1. SOILS (SLAB ON GRADE)		SITE PREPARATION	IBC 1705.6	
A. SUB--GRADE 1. VISUAL OBSERVATION	PERIODIC	AT THE CONTRACTORS EXPENSE, INSTRUMENT READINGS SHALL BE TAKEN BY A LICENSED SURVEYOR TO VERIFY FINAL SUBGRADE ELEVATIONS AND SLOPES.	GEOTECHNICAL REPORT, BUILDING PAD GENERAL NOTES	*QUALIFICATIONS BASED ON ASTM D3740 LICENSED SURVEYOR
2. PROOFROLLING OBSERVATIONS	CONTINUOUS	PROOFROLLING SHALL BE MONITORED BY A GEOTECHNICAL ENGINEER. THE GEOTECHNICAL ENGINEER SHALL BE APPROVE THE TYPE OF PROOFROLLING EQUIPMENT AND PROCEDURES.	GEOTECHNICAL REPORT, BUILDING PAD GENERAL NOTES	*QUALIFICATIONS BASED ON ASTM D3740
3. MOISTURE CONDITIONING & RECOMPACTION	CONTINUOUS OR PERIODIC	PROVIDE (1) ON DENSITY TEST FOR EACH 3000 SQ. FT. REFER TO UNDERFLOOR FILL NOTES FOR TESTING SPECIFICATIONS.	GEOTECHNICAL REPORT, BUILDING PAD GENERAL NOTES	*QUALIFICATIONS BASED ON ASTM D3740
B. CHEMICAL INJECTION	N/A	QUALITY CONTROLLED TESTING AND EVALUATION PRIOR AND SUBSEQUENT TO INJECTION SHALL BE PERFORMED BY THE GEOTECHNICAL ENGINEER TO DETERMINE THE EFFECTIVENESS OF THE CHEMICAL INJECTION PROCESS. THE GEOTECHNICAL ENGINEER OR HIS REPRESENTATIVE SHALL MONITOR THE INJECTION PROCESS TO VERIFY AREA COVERAGE, INJECTION DEPTH AND TO REVIEW AND MONITOR THE SWELL TEST RESULTS.	GEOTECHNICAL REPORT, BUILDING PAD GENERAL NOTES	*QUALIFICATIONS BASED ON ASTM D3740
C. DURING FILL PLACEMENT	CONTINUOUS OR PERIODIC	VISUAL OBSERVATIONS: DURING PLACEMENT AND COMPACTION OF FILL, SPECIAL INSPECTOR SHALL DETERMINE THE MATERIAL BEING USED AND THE MAXIMUM LIFT THICKNESS COMPLY WITH ADDITIONAL SAMPLES TESTED EACH DAY, OR MORE OFTEN IF MATERIAL APPEARS TO VARY.	IBC 1705.6 GEOTECHNICAL REPORT, BUILDING PAD GENERAL NOTES	*QUALIFICATIONS BASED ON ASTM D3740
D. EVALUATION OF IN--PLACE DENSITY OF FILL	CONTINUOUS OR PERIODIC	PROVIDE (1) ON DENSITY TEST FOR EACH 3000 SQ. FT. REFER TO UNDERFLOOR FILL NOTES FOR TESTING SPECIFICATIONS.	IBC 1705.6 GEOTECHNICAL REPORT, BUILDING PAD GENERAL NOTES	*QUALIFICATIONS BASED ON ASTM D3740
E. TRENCH BACKFILLING:	CONTINUOUS OR PERIODIC	TRENCH BACKFILLING: TRENCH BACKFILLING WITH CLAY CAP AND PLACING OF CLAY PLUG SHALL BE MONITORED BY GEOTECHNICAL ENGINEER.		
2A. PILE FOUNDATIONS				
A. THE GEOTECHNICAL ENGINEER OR A QUALIFIED E.I.T. INVOLVED IN THE ORIGINAL GEOTECHNICAL INVESTIGATION AND UNDER THE DIRECT SUPERVISION OF THE GEOTECHNICAL ENGINEER SHALL BE PRESENT DURING THE EXCAVATION OF THE FIRST PILE.	N/A	1. VERIFY THE BEARING STRATUM IS ENCOUNTERED AT THE ANTICIPATED DEPTH. 2. ADDRESS UNFORESEEN SUBSURFACE CONDITIONS, IF ANY. 3. VERIFY CONFORMANCE WITH THE FOUNDATION RECOMMENDATIONS PROVIDE IN THE PROJECT "GEOTECHNICAL ENGINEERING STUDY" AND THE STRUCTURAL DRAWINGS ISSUED FOR THE PROJECT.	IBC 1705.7 GEOTECHNICAL REPORT;	GRADUATE ENGINEER *QUALIFICATIONS BASED ON ASTM E329 & ASTM C1077
B. ALL FOOTINGS SHALL BE OBSERVED AND MONITORED BY A REPRESENTATIVE OF THE GEOTECHNICAL ENGINEER. THE CONTRACTOR SHALL PROVIDE THE GEOTECHNICAL ENGINEER WITH A COMPLETE SET OF STRUCTURAL DRAWINGS THAT ARE TO REMAIN WITH THE GEOTECHNICAL ENGINEER OR HIS REPRESENTATIVE.	N/A	1. PROVIDE RECORD OF EACH PILE INSTALLED. 2. RECORD LOAD TESTS, CUTOFF AND TIP OF EACH PILE.	IBC 1705.7 GEOTECHNICAL REPORT;	*QUALIFICATIONS BASED ON ASTM E329 & ASTM C1077



Date:	Revisions/Submissions

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LOU KARDON PARK
6651 Gibbs Sprawl Rd.
SAN ANTONIO, TEXAS
SPECIAL INSPECTIONS



Designed By:	J.H.
Drawn By:	S.J.F.
Date:	02/05/2021
Project No.:	00-000-00
Filename:	

7. WOOD CONSTRUCTION		IBC 1704.6	
A. PREFABRICATED STRUCTURAL ELEMENTS & ASSEMBLIES	N/A	INSPECT STRUCTURAL LOAD BEARING MEMBERS AND ASSEMBLIES. VERIFY THAT THE FABRICATOR MAINTAINS DETAILED FABRICATION AND QUALITY CONTROL PROCEDURES THAT PROVIDE A BASIS FOR INSPECTION CONTROL OF THE WORKMANSHIP AND THE FABRICATOR'S ABILITY TO CONFORM TO APPROVED CONSTRUCTION DOCUMENTS AND REFERENCED STANDARDS. THE SPECIAL INSPECTOR SHALL REVIEW THE PROCEDURES FOR CODE REQUIREMENTS FOR THE FABRICATOR'S SCOPE OF WORK. EXCEPTION: SPECIAL INSPECTIONS SHALL NOT BE REQUIRED WHERE THE FABRICATOR IS ENROLLED IN A NATIONALLY ACCEPTED INSPECTIONS PROGRAM ACCEPTABLE TO THE REGISTERED DESIGN PROFESSIONAL IS RESPONSIBLE CHARGE.	IBC 1705.5 TECHNICAL REPRESENTATIVE UNDER DIRECTION OF LICENSED ENGINEER
B. SITE BUILT ASSEMBLIES	N/A	SITE BUILT ASSEMBLIES SHALL BE INSPECTED IN ACCORDANCE WITH IBC SECTION 1704.1	IBC 1705.5 LICENSED ENGINEER OR HIS/HER REPRESENTATIVE.
C. DIAPHRAGMS	N/A	HIGH LOAD DIAPHRAGMS SHALL BE INSPECTED IN ACCORDANCE WITH IBC SECTION 1704.1, AND SHEATHING CHECKED FOR PROPER GRADE, THICKNESS, SIZE OF FRAMING MEMBERS AT ADJOINING PANEL EDGES, NAIL/STAPLE DIAMETER AND LENGTH, AND FASTENER PATTERN.	IBC 1705.5.1
D. TRUSS BRACING	N/A	CHECK ALL REQUIRED PERMANENT AND LATERAL BRACING HAS BEEN INSTALLED ACCORDING TO STRUCTURAL DRAWINGS AND FABRICATOR DESIGN/SHOP DRAWINGS.	
8. LIGHT GAGE FRAME CONSTRUCTION		IBC 1704.13	
A. PREFABRICATED STRUCTURAL ELEMENTS & ASSEMBLIES	N/A	INSPECT STRUCTURAL LOAD BEARING MEMBERS AND ASSEMBLIES. VERIFY THAT THE FABRICATOR MAINTAINS DETAILED FABRICATION AND QUALITY CONTROL PROCEDURES THAT PROVIDE A BASIS FOR INSPECTION CONTROL OF THE WORKMANSHIP AND THE FABRICATOR'S ABILITY TO CONFORM TO APPROVED CONSTRUCTION DOCUMENTS AND REFERENCED STANDARDS. THE SPECIAL INSPECTOR SHALL REVIEW THE PROCEDURES FOR CODE REQUIREMENTS FOR THE FABRICATOR'S SCOPE OF WORK. EXCEPTION: SPECIAL INSPECTIONS SHALL NOT BE REQUIRED WHERE THE FABRICATOR IS ENROLLED IN A NATIONALLY ACCEPTED INSPECTIONS PROGRAM ACCEPTABLE TO THE REGISTERED DESIGN PROFESSIONAL IS RESPONSIBLE CHARGE.	IBC 1705.5.1 TECHNICAL REPRESENTATIVE UNDER DIRECTION OF LICENSED ENGINEER
B. SITE BUILT ASSEMBLIES	N/A	SITE BUILT ASSEMBLIES SHALL BE INSPECTED IN ACCORDANCE WITH IBC SECTION 1704.1	IBC 1705.5.1 LICENSED ENGINEER OR HIS/HER REPRESENTATIVE.
C. DIAPHRAGMS	N/A	HIGH LOAD DIAPHRAGMS SHALL BE INSPECTED IN ACCORDANCE WITH IBC SECTION 1704.1, AND SHEATHING CHECKED FOR PROPER GRADE, THICKNESS, SIZE OF FRAMING MEMBERS AT ADJOINING PANEL EDGES, NAIL/STAPLE DIAMETER AND LENGTH, AND FASTENER PATTERN.	IBC 1705.10.3
D. TRUSS BRACING	N/A	CHECK ALL REQUIRED PERMANENT AND LATERAL BRACING HAS BEEN INSTALLED ACCORDING TO STRUCTURAL DRAWINGS AND FABRICATOR DESIGN/SHOP DRAWINGS.	

LEVEL 1 INSPECTION CONT.:					
C. PRIOR TO GROUTING, THE FOLLOWING SHALL BE VERIFIED TO ENSURE COMPLIANCE:	N/A	1. GROUT SPACE IS CLEAN.			
	N/A	2. PLACEMENT OF REINFORCEMENT AND CONNECTORS AND PRESTRESSING TENDONS AND ANCHORAGES.			
	N/A	3. PROPORTIONS OF SITE-PREPARED GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS.			
	N/A	4. CONSTRUCTION OF MORTAR JOINTS.			
D. GROUT PLACEMENT	N/A	1. VERIFY COMPLIANCE WITH CODE AND CONSTRUCTION DOCUMENTS PROVISIONS.			
	N/A	2. GROUTING OF PRESTRESSING BONDED TENDONS.			
E. PREPARATION OF ANY REQUIRED GROUT SPECIMENS, MORTAR SPECIMENS AND/OR PRISMS SHALL BE OBSERVED.	N/A	1. VERIFY COMPLIANCE WITH CODE AND CONSTRUCTION DOCUMENTS PROVISIONS.			QUALIFICATIONS BASED ON C1093
F. COMPLIANCE WITH REQUIRED INSPECTION PROVISION OF THE CONSTRUCTION DOCUMENTS AND THE APPROVED SUBMITTALS SHALL BE VERIFIED.	N/A	1. VERIFY COMPLIANCE WITH CODE AND CONSTRUCTION DOCUMENTS PROVISIONS.			
G. TESTING OF GROUT SPECIMENS, MORTAR SPECIMENS AND/OR PRISMS.	N/A	1. TEST ONE SET OF MORTAR CUBES PER 2000 sf OR PORTION THEREOF. 2. TEST ONE SET OF GROUT CYLINDERS PER 2000 sf OR PORTION THEREOF. 3. TEST ONE PRISM PER 6000 sf OR PORTION THEREOF. (SUBMITTED PRISM WILL BE ACCEPTABLE FOR FIRST PRISM TEST).			QUALIFICATIONS BASED ON C1093
H. POST INSTALLED REINFORCING & ANCHORS (EXPANSION ANCHORS, SCREW ANCHORS ADHESIVE ANCHORS, ECT.).	N/A	THE SPECIAL INSPECTOR SHALL BE ON THE JOB SITE CONTINUOUSLY DURING ANCHOR INSTALLATION TO VERIFY ANCHOR TYPE, ANCHOR DIMENSIONS, MASONRY TYPE AND COMPRESSION STRENGTH, PRE DRILLED HOLE DIMENSIONS, ANCHOR SPACING, EDGE DISTANCES, MASONRY THICKNESS AND ANCHOR EMBEDMENT.	ACI 318 APPENDIX D-CH. D.9.1		*QUALIFICATIONS BASED ON ASTM E329 & ASTM C1077 OR CERTIFIED MANUFACTURER REPRESENTATIVE
LEVEL 2 INSPECTION:					
A. FROM THE BEGINNING OF MASONRY CONSTRUCTION, THE FOLLOWING SHALL BE VERIFIED TO ENSURE COMPLIANCE:		ENGINEERED MASONRY IN ESSENTIAL FACILITIES.	IBC 1704.5.3		QUALIFICATIONS BASED ON C1093
	N/A	1. PROPORTIONS OF SITE-PREPARED MORTAR, GROUT, AND PRESTRESSING GROUT FOR BONDED TENDONS.			
	N/A	2. PLACEMENT OF MASONRY UNITS AND CONSTRUCTION OF MORTAR JOINTS.			
	N/A	3. PLACEMENT OF REINFORCEMENT, CONNECTORS, AND PRESTRESSING TENDONS AND ANCHORAGES.			
	N/A	4. GROUT SPACE PRIOR TO GROUTING.			
	N/A	5. PLACEMENT OF GROUT.			
B. THE INSPECTION PROGRAM SHALL VERIFY:	N/A	1. SIZE AND LOCATION OF STRUCTURAL ELEMENTS.			
	N/A	2. TYPE, SIZE AND LOCATION OF ANCHORS, INCLUDING OTHER DETAILS OF ANCHORAGE OF MASONRY TO STRUCTURAL MEMBERS, FRAMES, OR OTHER CONSTRUCTION.			
	N/A	3. SPECIFIED SIZE, GRADE AND TYPE OF REINFORCEMENT.			
	N/A	4. WELDING OF REINFORCEMENT.			
	N/A	PROTECTION OF MASONRY DURING COLD WEATHER (TEMPERATURE BELOW 40 DEGREES F) OR HOT WEATHER (TEMPERATURE ABOVE 90 DEGREES F).			
	N/A	6. APPLICATION AND MEASUREMENT OF PRESTRESSING FORCE.			
C. PREPARATION OF ANY REQUIRED GROUT SPECIMENS, MORTAR SPECIMENS AND/OR PRISMS SHALL BE OBSERVED.	N/A	1. VERIFY COMPLIANCE WITH CODE AND CONSTRUCTION DOCUMENTS PROVISIONS.			QUALIFICATIONS BASED ON C1093
D. COMPLIANCE WITH REQUIRED INSPECTION PROVISIONS OF THE CONSTRUCTION DOCUMENTS AND THE APPROVED SUBMITTALS SHALL BE VERIFIED.	N/A				
E. TESTING OF GROUT SPECIMENS, MORTAR SPECIMENS AND/OR PRISMS.	N/A	1. TEST ONE SET OF MORTAR CUBES PER 2000 sf OR PORTION THEREOF. 2. TEST ONE SET OF GROUT CYLINDERS PER 2000 sf OR PORTION THEREOF. 3. TEST ONE PRISM PER 6000 sf OR PORTION THEREOF. (SUBMITTED PRISM WILL BE ACCEPTABLE FOR FIRST PRISM TEST).			QUALIFICATIONS BASED ON C1093

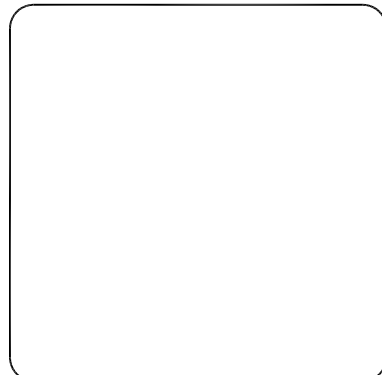
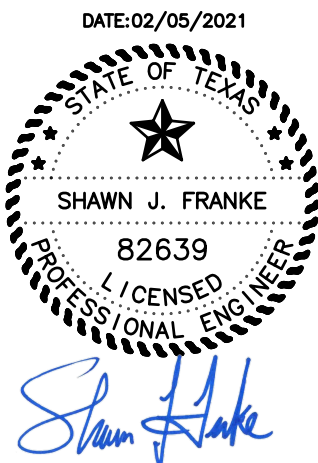
3. STEEL CONSTRUCTION CONT.:				
G. STEEL FRAME JOINT DETAILS; COMPLIANCE WITH APPROVED CONSTRUCTION DOCUMENTS:	PERIODIC	1. DETAILS SUCH AS BRACING & STIFFENING.	IBC 1705.2.1; STRUCTURAL DRAWINGS	PROJECT OF COMPLEX DETAILS: - ASSOCIATE CWI - PROJECTS OF RELATIVELY SIMPLE DETAILS: - TECHNICIAN TRAINED IN FIELD OF WORK AND HAS AT LEAST ONE YEAR OF EXPERIENCE.
	PERIODIC	2. MEMBER LOCATIONS.		
	PERIODIC	3. APPLICATION OF JOINT DETAILS AT EACH CONNECTION.		
H. POST INSTALLED REINFORCING & ANCHORS (EXPANSION ANCHORS, SCREW ANCHORS ADHESIVE ANCHORS, ECT.).	CONTINUOUS	THE SPECIAL INSPECTOR SHALL BE ON THE JOB SITE CONTINUOUSLY DURING ANCHOR INSTALLATION TO VERIFY ANCHOR TYPE, ANCHOR DIMENSIONS, CONCRETE OR MASONRY TYPE AND COMPRESSION STRENGTH, PRE DRILLED HOLE DIMENSIONS, ANCHOR SPACING, EDGE DISTANCES, CONCRETE OR MASONRY THICKNESS AND ANCHOR EMBEDMENT.	ACI 318 APPENDIX D-CH. D.9.1	*QUALIFICATIONS BASED ON ASTM E329 & ASTM C1077 OR CERTIFIED MANUFACTURER REPRESENTATIVE
5. INSPECTION OF FABRICATORS FOR STRUCTURAL STEEL				
FABRICATION & IMPLEMENTATION PROCEDURES	PERIODIC	FABRICATION AND IMPLEMENTATION PROCEDURES. THE SPECIAL INSPECTOR SHALL VERIFY THAT THE FABRICATOR MAINTAINS DETAILED FABRICATION AND QUALITY CONTROL OF THE WORKMANSHIP AND THE FABRICATOR'S ABILITY TO CONFORM TO APPROVED CONSTRUCTION DOCUMENTS AND REFERENCED STANDARDS. THE SPECIAL INSPECTOR SHALL REVIEW THE PROCEDURES FOR COMPLETENESS AND ADEQUACY RELATIVE TO THE CODE REQUIREMENTS FOR THE FABRICATOR'S SCOPE OF WORK. EXCEPTION: SPECIAL INSPECTIONS SHALL NOT BE REQUIRED WHERE THE WORK IS DONE ON THE PREMISES OF A FABRICATOR THAT IS ENROLLED IN A NATIONALLY ACCEPTED INSPECTIONS PROGRAM ACCEPTABLE TO THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE. AT COMPLETION OF FABRICATION, THE APPROVED FABRICATOR SHALL SUBMIT A CERTIFICATE OF COMPLIANCE TO BUILDING OFFICIAL UPON REQUEST AND TO THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE STATING THAT THE WORK WAS PERFORMED IN ACCORDANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS.	IBC 1705.2.1	CWI, ASNT, LICENSED ENGINEER
6. MASONRY CONSTRUCTION				
EMPIRICALLY DESIGNED MASONRY, GLASS UNIT MASONRY, AND MASONRY VENEER IN NON-ESSENTIAL FACILITIES.	SPECIAL INSPECTIONS NOT REQUIRED PER 1704.5.1		IBC 1705.4	
LEVEL 1 INSPECTION:		ENGINEERED MASONRY IN NON-ESSENTIAL FACILITIES AND EMPIRICALLY DESIGNED MASONRY IN ESSENTIAL FACILITIES.	IBC 1705.4	QUALIFICATIONS BASED ON ASTM C1093
A. AS MASONRY CONSTRUCTION BEGINS, THE FOLLOWING SHALL BE VERIFIED TO ENSURE COMPLIANCE:	N/A	1. PROPORTIONS OF SITE-PREPARED MORTAR.		
	N/A	2. CONSTRUCTION OF MORTAR JOINTS.		
	N/A	3. LOCATION OF REINFORCEMENT AND CONNECTORS.		
	N/A	4. PRESTRESSING TECHNIQUE		
	N/A	5. GRADE AND SIZE OF PRESTRESSING TENDONS AND ANCHORAGES.		
B. THE INSPECTION PROGRAM SHALL VERIFY:	N/A	1. SIZE AND LOCATION OF STRUCTURAL ELEMENTS.		
	N/A	2. TYPE, SIZE AND LOCATION OF ANCHORS, INCLUDING OTHER DETAILS OF ANCHORAGE OF MASONRY TO STRUCTURAL MEMBERS, FRAMES, OR OTHER CONSTRUCTION.		
	N/A	3. SPECIFIED SIZE, GRADE AND TYPE OF REINFORCEMENT.		
	N/A	4. WELDING OF REINFORCING BARS.		
	N/A	5. PROTECTION OF MASONRY DURING COLD WEATHER (TEMPERATURE BELOW 40 DEGREES F) OR HOT WEATHER (TEMPERATURE ABOVE 90 DEGREES F).		
	N/A	6. APPLICATION AND MEASUREMENT OF PRESTRESSING FORCE.		

NOTES:

1 THESE INSPECTIONS DO NOT RELIEVE ENGINEER FROM STRUCTURAL OBSERVATIONS AS MAY REQUIRED BY IBC 2018, SECTION 1709, AND/OR CONTRACTUAL REQUIREMENTS OF ARCHITECT/CLIENT, (I.E. C141).

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PCI – PRECAST/PRESTRESSED CONCRETE INSTITUTE
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N/A – NOT APPLICABLE

*TESTING AND INSPECTION DIRECTED BY ASTM E329 GUIDELINES.

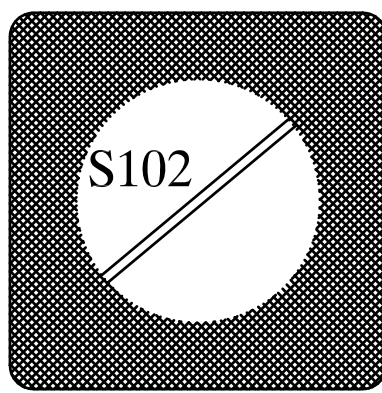


Date:	Revisions/Submissions

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SAN ANTONIO, TEXAS
SPECIAL INSPECTIONS



Designed By:	J.H.
Drawn By:	S.J.F.
Date:	02/05/2021
Project No.:	00-000-00
Filename:	

NOTE: FOOTING DESIGNS ARE PRELIMINARY UNTIL FINAL REACTIONS ARE PROVIDED BY CANOPY MANUFACTURERS

LUNDY & FRANKE
ENGINEERING
549 HEIMER ROAD SUITE 360 PH. (210) 979-7900
SAN ANTONIO, TEXAS 78232 FX. (210) 979-7800
TX FIRM REG. *3388

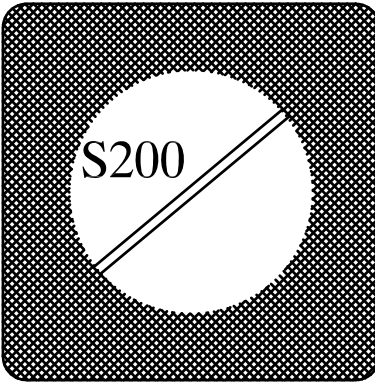
DATE: 02/05/2021
SHAWN J. FRANKE
82639
LICENSED PROFESSIONAL ENGINEER
Shawn Franke



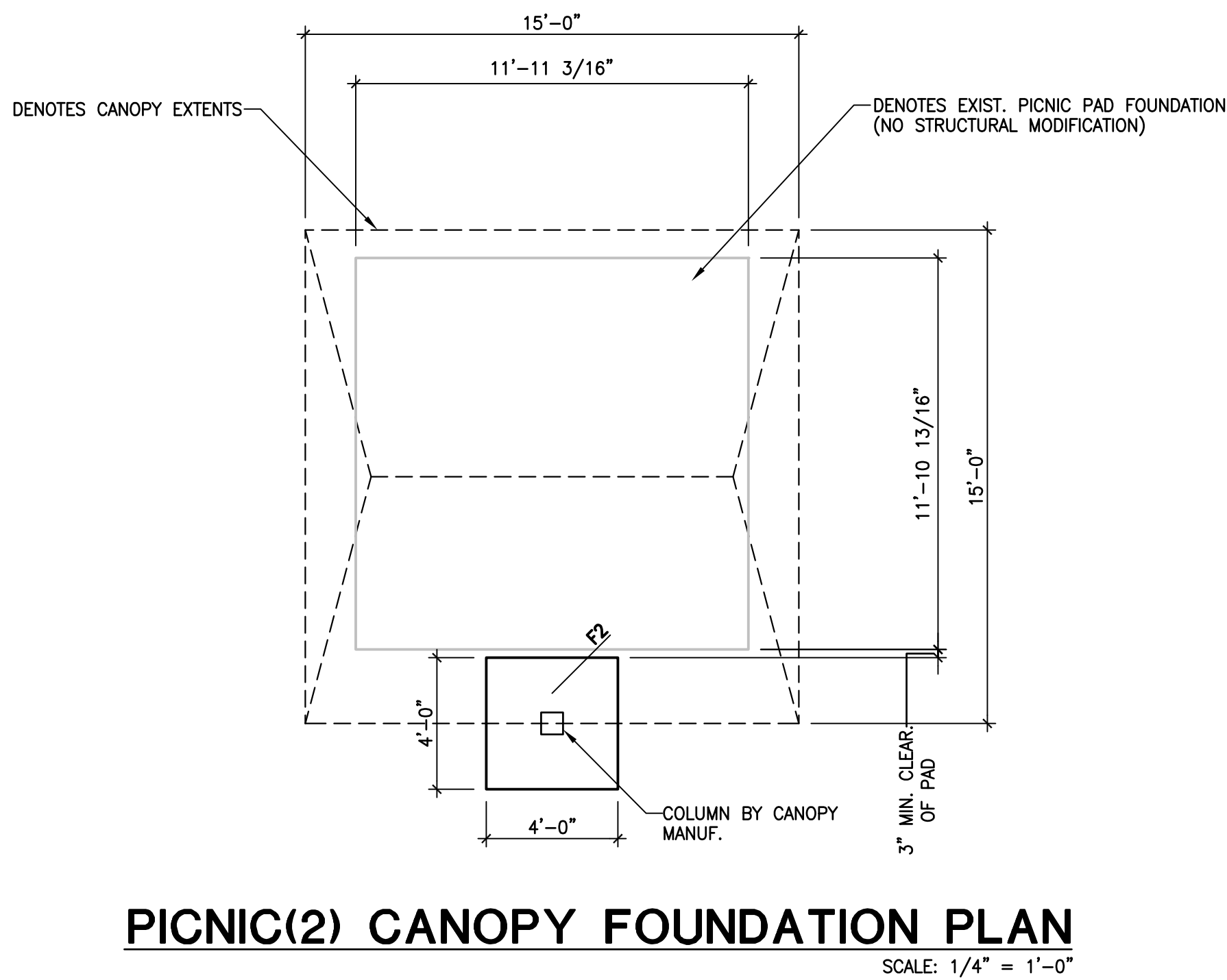
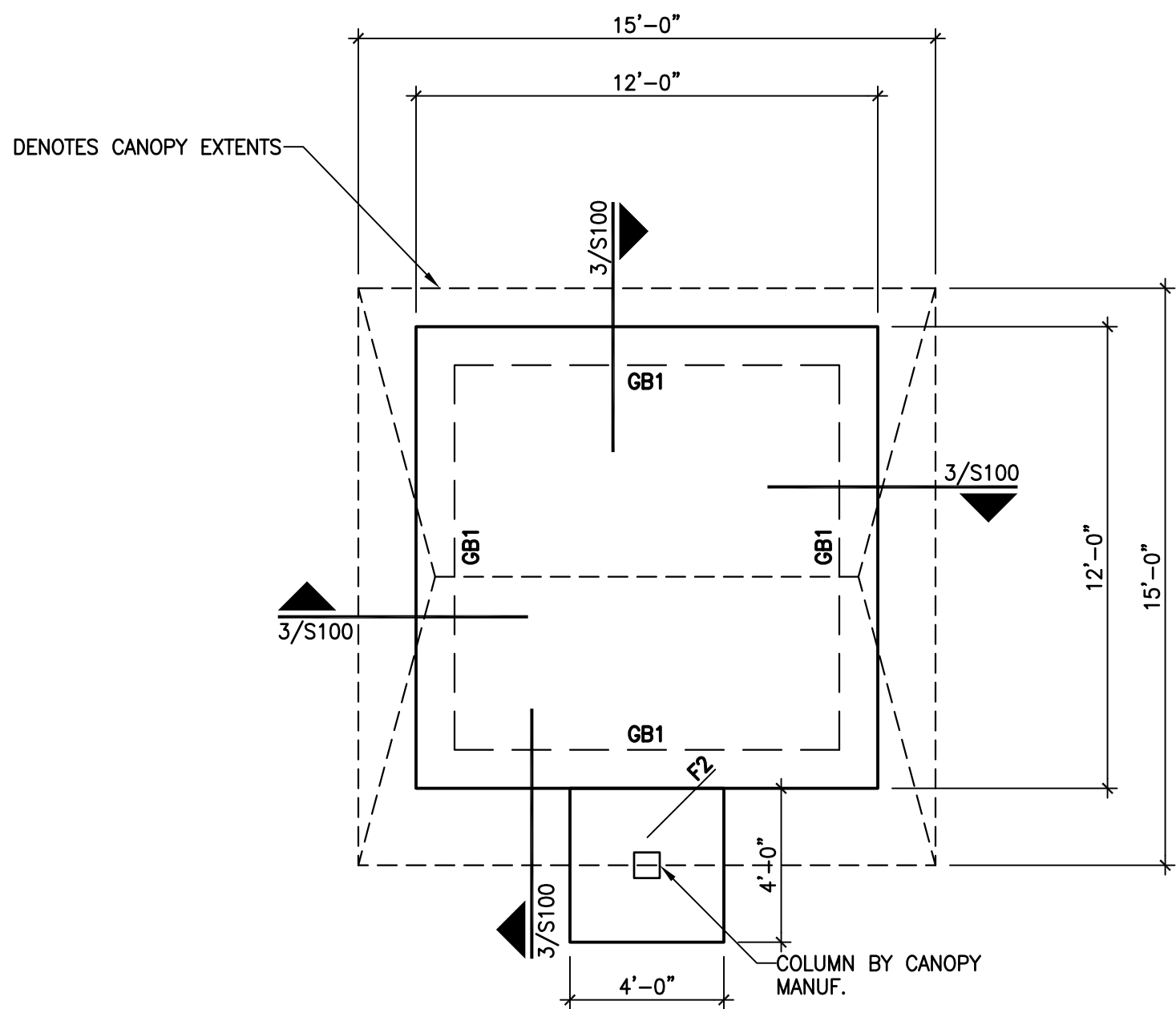
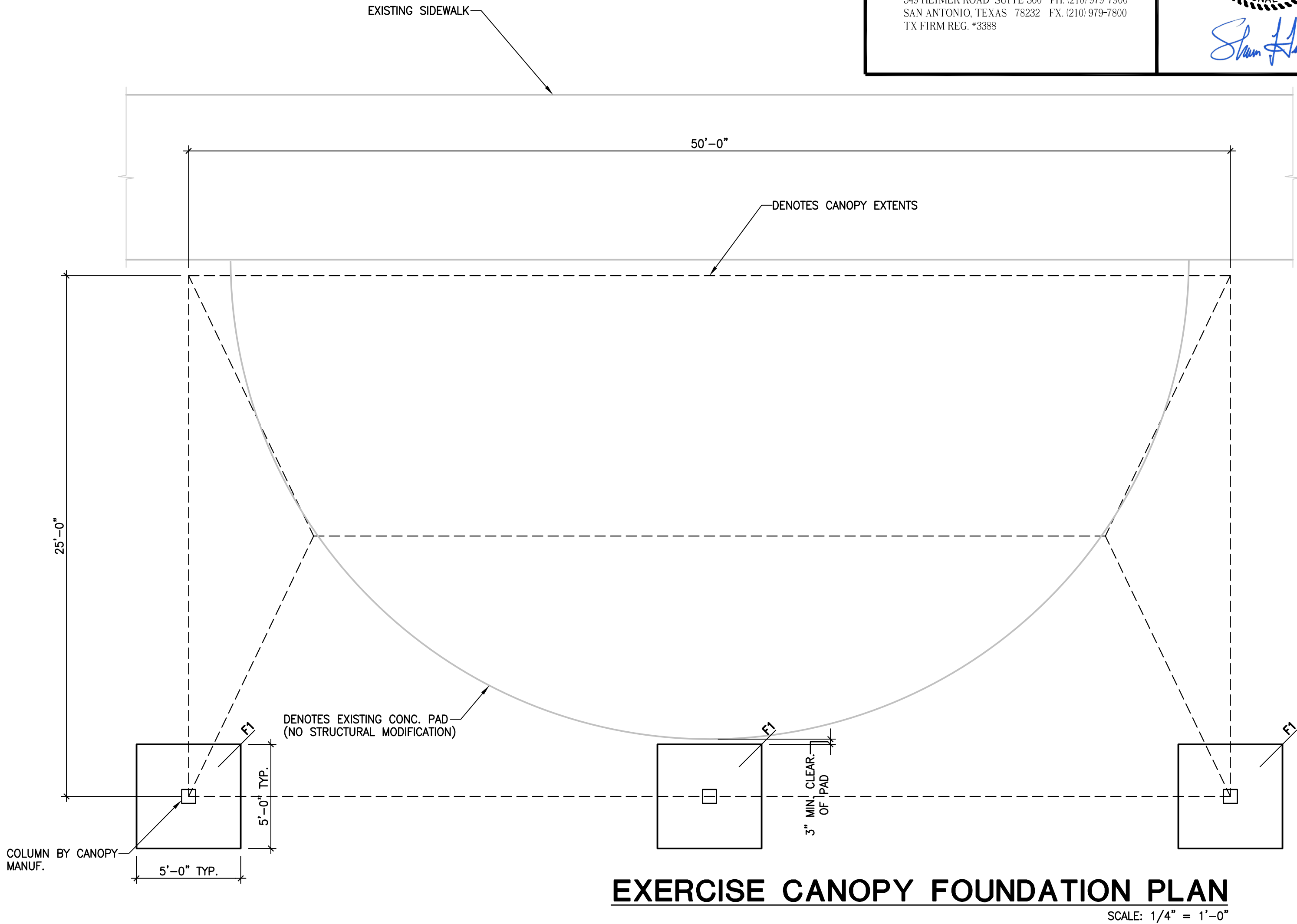
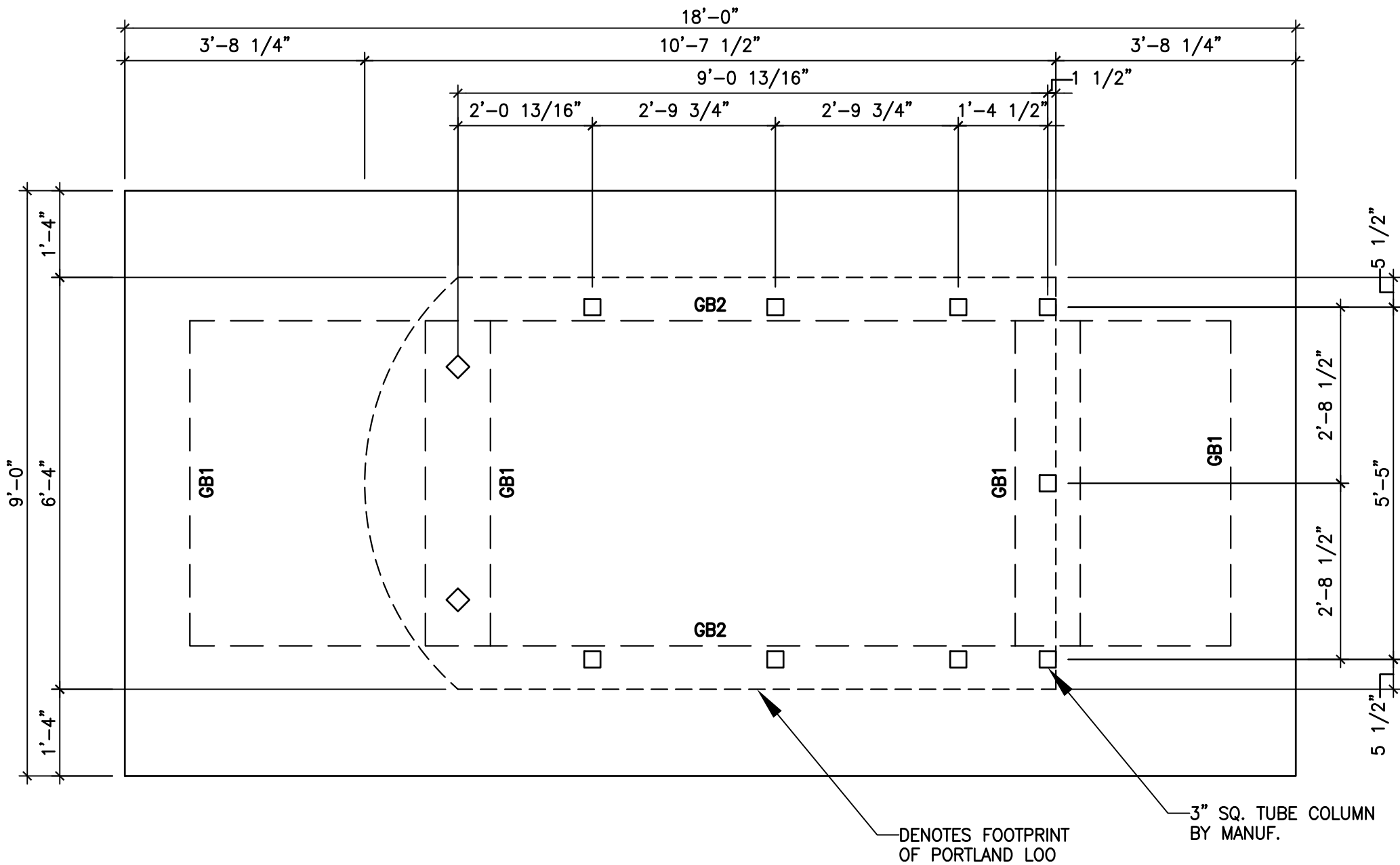
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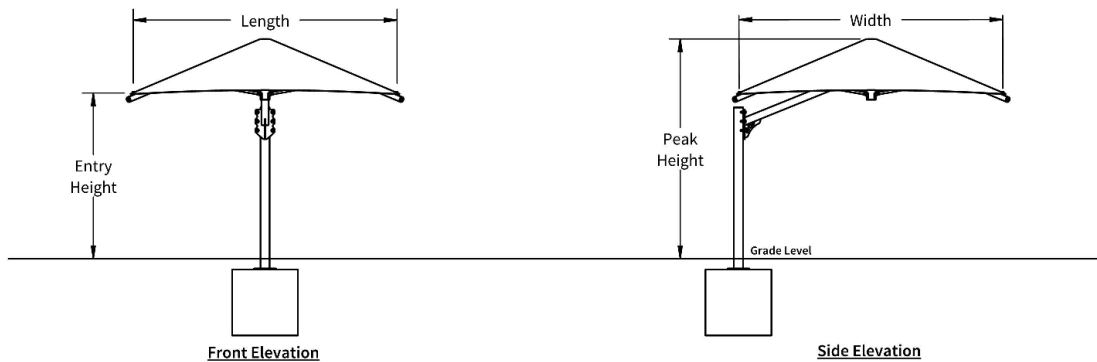
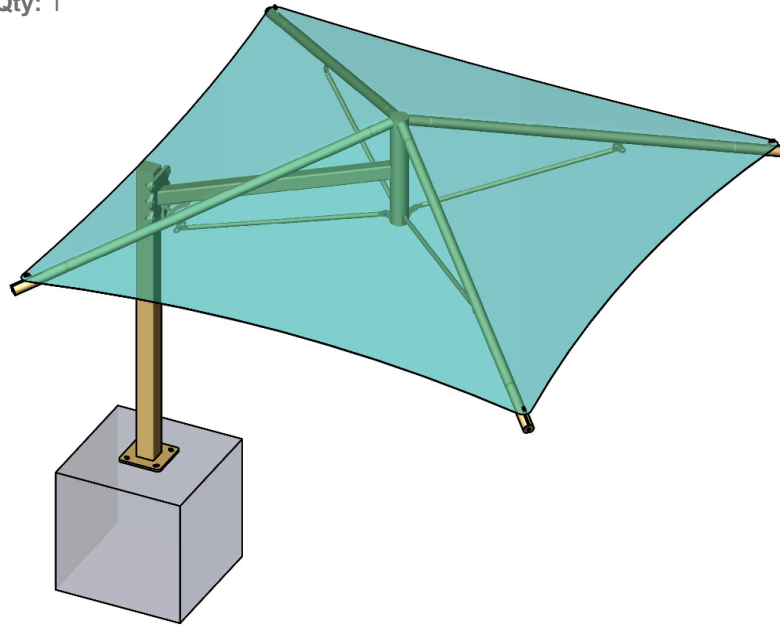
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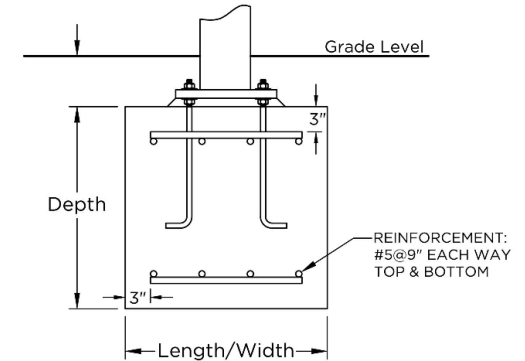
Square Cantilever Umbrella Shade

Length	15	Width	15	Entry Height	9.5
Peak Height	12.53	Elbow	Glide	Column Mount	Base Plate
Column Size	6080.25	Rafter Size	3.511	Ridge Size	3.511
Column Length	10	Rafter Length	11.0485435	Ridge Length	0

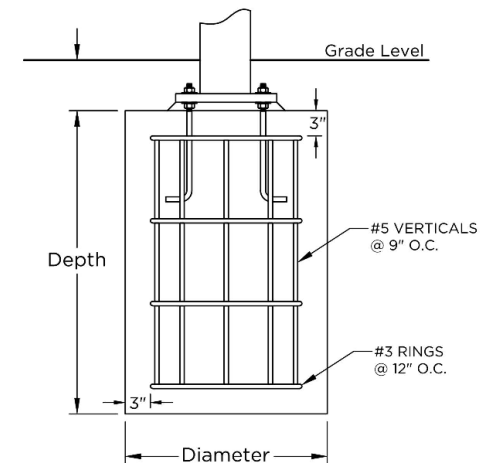
Dome Qty: 1 Column Qty: 1



Square Footing		
Column	Length & Width	Depth
Single Cap	4.15	3
Double Cap	0	3



Auger Footing		
Diameter	Single Cap Depth	Double Cap Depth
1'-6"		
2'-0"		
2'-6"		
3'-0"		



SHADE
BY SUPERIOR RECREATIONAL PRODUCTS

QUOTE

SHADE SIZE
15 X 15

SHADE STYLE
**Square Cantilever
Umbrella Shade**

These drawings are for reference only and should not be used as construction details. They show the general character and rough dimensions of the structural features. Exact spans, fasteners, materials, and foundations can be determined by a licensed professional engineer upon request. Estimated footing size above is based on 1,500 PSF soil bearing pressure.





PUBLIC REST




The Portland Loo
A Designer Sustainable Universal Restroom











