### HISTORIC AND DESIGN REVIEW COMMISSION

July 21, 2021

**HDRC CASE NO: 2021-324** 

**ADDRESS:** 909 N HACKBERRY ST

**LEGAL DESCRIPTION:** NCB 529 BLK 2 LOT N 92.5 FT OF 13 & 14

**ZONING:** IDZ-1, H

CITY COUNCIL DIST.: 2

**DISTRICT:** Dignowity Hill Historic District

**APPLICANT:** Michael Garansuay/GARANSUAY MICHAEL S & TERESA P **OWNER:** Michael Garansuay/GARANSUAY MICHAEL S & TERESA P

**TYPE OF WORK:** New construction of two, 2-story residential structures and two, 1-story

residential structures

**APPLICATION RECEIVED:** June 30, 2021

**60-DAY REVIEW:** Not applicable due to City Council Emergency Orders

**CASE MANAGER:** Edward Hall

**REQUEST:** 

The applicant is requesting a Certificate of Appropriateness for approval to construct two, 2-story residential structures and two, 1-story residential structures on the vacant lot at 909 N Hackberry, located within the Dignowity Hill Historic District.

### **APPLICABLE CITATIONS:**

Historic Design Guidelines, Chapter 4, Guidelines for New Construction

1. Building and Entrance Orientation

### A. FAÇADE ORIENTATION

i. Setbacks—Align front facades of new buildings with front facades of adjacent buildings where a consistent setback has been established along the street frontage. Use the median setback of buildings along the street frontage where a variety of setbacks exist. Refer to UDC Article 3, Division 2. Base Zoning Districts for applicable setback requirements. ii. Orientation—Orient the front façade of new buildings to be consistent with the predominant orientation of historic buildings along the street frontage.

### **B. ENTRANCES**

*i. Orientation*—Orient primary building entrances, porches, and landings to be consistent with those historically found along the street frontage. Typically, historic building entrances are oriented towards the primary street.

### 2. Building Massing and Form

### A. SCALE AND MASS

- i. Similar height and scale—Design new construction so that its height and overall scale are consistent with nearby historic buildings. In residential districts, the height and scale of new construction should not exceed that of the majority of historic buildings by more than one-story. In commercial districts, building height shall conform to the established pattern. If there is no more than a 50% variation in the scale of buildings on the adjacent block faces, then the height of the new building shall not exceed the tallest building on the adjacent block face by more than 10%.
- *ii. Transitions*—Utilize step-downs in building height, wall-plane offsets, and other variations in building massing to provide a visual transition when the height of new construction exceeds that of adjacent historic buildings by more than one-half story.
- *iii. Foundation and floor heights*—Align foundation and floor-to-floor heights (including porches and balconies) within one foot of floor-to-floor heights on adjacent historic structures.

### B. ROOF FORM

*i. Similar roof forms*—Incorporate roof forms—pitch, overhangs, and orientation—that are consistent with those predominantly found on the block. Roof forms on residential building types are typically sloped, while roof forms on nonresidential

building types are more typically flat and screened by an ornamental parapet wall.

*ii. Façade configuration*—The primary façade of new commercial buildings should be in keeping with established patterns. Maintaining horizontal elements within adjacent cap, middle, and base precedents will establish a consistent street wall through the alignment of horizontal parts. Avoid blank walls, particularly on elevations visible from the street. No new façade should exceed 40 linear feet without being penetrated by windows, entryways, or other defined bays.

### D. LOT COVERAGE

*i. Building to lot ratio*—New construction should be consistent with adjacent historic buildings in terms of the building to lot ratio. Limit the building footprint for new construction to no more than 50 percent of the total lot area, unless adjacent historic buildings establish a precedent with a greater building to lot ratio.

### 3. Materials and Textures

### A. NEW MATERIALS

- *i. Complementary materials*—Use materials that complement the type, color, and texture of materials traditionally found in the district. Materials should not be so dissimilar as to distract from the historic interpretation of the district. For example, corrugated metal siding would not be appropriate for a new structure in a district comprised of homes with wood siding.
- *ii. Alternative use of traditional materials*—Consider using traditional materials, such as wood siding, in a new way to provide visual interest in new construction while still ensuring compatibility.
- iii. Roof materials—Select roof materials that are similar in terms of form, color, and texture to traditionally used in the district.
- *iv. Metal roofs*—Construct new metal roofs in a similar fashion as historic metal roofs. Refer to the Guidelines for Alterations and Maintenance section for additional specifications regarding metal roofs.
- v. Imitation or synthetic materials—Do not use vinyl siding, plastic, or corrugated metal sheeting. Contemporary materials not traditionally used in the district, such as brick or simulated stone veneer and Hardie Board or other fiberboard siding, may be appropriate for new construction in some locations as long as new materials are visually similar to the traditional material in dimension, finish, and texture. EIFS is not recommended as a substitute for actual stucco.

### 4. Architectural Details

### A. GENERAL

- *i. Historic context*—Design new buildings to reflect their time while respecting the historic context. While new construction should not attempt to mirror or replicate historic features, new structures should not be so dissimilar as to distract from or diminish the historic interpretation of the district.
- ii. Architectural details—Incorporate architectural details that are in keeping with the predominant architectural style along the block face or within the district when one exists. Details should be simple in design and should complement, but not visually compete with, the character of the adjacent historic structures or other historic structures within the district. Architectural details that are more ornate or elaborate than those found within the district are inappropriate. iii. Contemporary interpretations—Consider integrating contemporary interpretations of traditional designs and details for new construction. Use of contemporary window moldings and door surroundings, for example, can provide visual interest while helping to convey the fact that the structure is new. Modern materials should be implemented in a way that does not distract from the historic structure.

### 5. Garages and Outbuildings

### A. DESIGN AND CHARACTER

- v. Garage doors—Incorporate garage doors with similar proportions and materials as those traditionally found in the district.
- 6. Mechanical Equipment and Roof Appurtenances

### A. LOCATION AND SITING

i. Visibility—Do not locate utility boxes, air conditioners, rooftop mechanical equipment, skylights, satellite dishes, and

other roof appurtenances on primary facades, front-facing roof slopes, in front yards, or in other locations that are clearly visible from the public right-of-way.

ii. Service Areas—Locate service areas towards the rear of the site to minimize visibility from the public right-of-way.

### **B. SCREENING**

- *i. Building-mounted equipment*—Paint devices mounted on secondary facades and other exposed hardware, frames, and piping to match the color scheme of the primary structure or screen them with landscaping.
- *ii. Freestanding equipment*—Screen service areas, air conditioning units, and other mechanical equipment from public view using a fence, hedge, or other enclosure.
- *iii. Roof-mounted equipment*—Screen and set back devices mounted on the roof to avoid view from public right-of-way. Historic Design Guidelines, Chapter 5, Guidelines for Site Elements

### B. NEW FENCES AND WALLS

- *i. Design*—New fences and walls should appear similar to those used historically within the district in terms of their scale, transparency, and character. Design of fence should respond to the design and materials of the house or main structure.
- *ii.* Location—Avoid installing a fence or wall in a location where one did not historically exist, particularly within the front yard. The appropriateness of a front yard fence or wall is dependent on conditions within a specific historic district. New front yard fences or wall should not be introduced within historic districts that have not historically had them.
- *iii.* Height—Limit the height of new fences and walls within the front yard to a maximum of four feet. The appropriateness of a front yard fence is dependent on conditions within a specific historic district. New front yard fences should not be introduced within historic districts that have not historically had them. If a taller fence or wall existed historically, additional height may be considered. The height of a new retaining wall should not exceed the height of the slope it retains.
- iv. Prohibited materials—Do not use exposed concrete masonry units (CMU), Keystone or similar interlocking retaining wall systems, concrete block, vinyl fencing, or chain link fencing.
- v. Appropriate materials—Construct new fences or walls of materials similar to fence materials historically used in the district. Select materials that are similar in scale, texture, color, and form as those historically used in the district, and that are compatible with the main structure. Screening incompatible uses—Review alternative fence heights and materials for appropriateness where residential properties are adjacent to commercial or other potentially incompatible uses.

### 3. Landscape Design

### A. PLANTINGS

- i. Historic Gardens— Maintain front yard gardens when appropriate within a specific historic district.
- ii. Historic Lawns—Do not fully remove and replace traditional lawn areas with impervious hardscape. Limit the removal of lawn areas to mulched planting beds or pervious hardscapes in locations where they would historically be found, such as along fences, walkways, or drives. Low-growing plantings should be used in historic lawn areas; invasive or large-scale species should be avoided. Historic lawn areas should never be reduced by more than 50%.
- iii. Native xeric plant materials—Select native and/or xeric plants that thrive in local conditions and reduce watering usage. See UDC Appendix E: San Antonio Recommended Plant List—All Suited to Xeriscape Planting Methods, for a list of appropriate materials and planting methods. Select plant materials with a similar character, growth habit, and light requirements as those being replaced.
- iv. Plant palettes—If a varied plant palette is used, incorporate species of taller heights, such informal elements should be restrained to small areas of the front yard or to the rear or side yard so as not to obstruct views of or otherwise distract from the historic structure.
- v. Maintenance—Maintain existing landscape features. Do not introduce landscape elements that will obscure the historic structure or are located as to retain moisture on walls or foundations (e.g., dense foundation plantings or vines) or as to cause damage.

### B. ROCKS OR HARDSCAPE

*i. Impervious surfaces* —Do not introduce large pavers, asphalt, or other impervious surfaces where they were not historically located.

- *ii. Pervious and semi-pervious surfaces*—New pervious hardscapes should be limited to areas that are not highly visible, and should not be used as wholesale replacement for plantings. If used, small plantings should be incorporated into the design.
- *iii.* Rock mulch and gravel Do not use rock mulch or gravel as a wholesale replacement for lawn area. If used, plantings should be incorporated into the design.

### D. TREES

- *i. Preservation*—Preserve and protect from damage existing mature trees and heritage trees. See UDC Section 35-523 (Tree Preservation) for specific requirements.
- *ii.* New Trees Select new trees based on site conditions. Avoid planting new trees in locations that could potentially cause damage to a historic structure or other historic elements. Species selection and planting procedure should be done in accordance with guidance from the City Arborist.
- 5. Sidewalks, Walkways, Driveways, and Curbing

### A. SIDEWALKS AND WALKWAYS

- *i. Maintenance*—Repair minor cracking, settling, or jamming along sidewalks to prevent uneven surfaces. Retain and repair historic sidewalk and walkway paving materials—often brick or concrete—in place.
- *ii. Replacement materials*—Replace those portions of sidewalks or walkways that are deteriorated beyond repair. Every effort should be made to match existing sidewalk color and material.
- *iii. Width and alignment*—Follow the historic alignment, configuration, and width of sidewalks and walkways. Alter the historic width or alignment only where absolutely necessary to accommodate the preservation of a significant tree.
- *iv. Stamped concrete*—Preserve stamped street names, business insignias, or other historic elements of sidewalks and walkways when replacement is necessary.
- v. ADA compliance—Limit removal of historic sidewalk materials to the immediate intersection when ramps are added to address ADA requirements.

### **B. DRIVEWAYS**

- i. Driveway configuration—Retain and repair in place historic driveway configurations, such as ribbon drives. Incorporate a similar driveway configuration—materials, width, and design—to that historically found on the site. Historic driveways are typically no wider than 10 feet. Pervious paving surfaces may be considered where replacement is necessary to increase stormwater infiltration.
- *ii. Curb cuts and ramps*—Maintain the width and configuration of original curb cuts when replacing historic driveways. Avoid introducing new curb cuts where not historically found.

### 7. Off-Street Parking

### A. LOCATION

- *i. Preferred location*—Place parking areas for non-residential and mixed-use structures at the rear of the site, behind primary structures to hide them from the public right-of-way. On corner lots, place parking areas behind the primary structure and set them back as far as possible from the side streets. Parking areas to the side of the primary structure are acceptable when location behind the structure is not feasible. See UDC Section 35-310 for district-specific standards.
- *ii. Front*—Do not add off-street parking areas within the front yard setback as to not disrupt the continuity of the streetscape.
- *iii.* Access—Design off-street parking areas to be accessed from alleys or secondary streets rather than from principal streets whenever possible.

### **B. DESIGN**

- *i. Screening*—Screen off-street parking areas with a landscape buffer, wall, or ornamental fence two to four feet high—or a combination of these methods. Landscape buffers are preferred due to their ability to absorb carbon dioxide. See UDC Section 35-510 for buffer requirements.
- *ii. Materials*—Use permeable parking surfaces when possible to reduce run-off and flooding. See UDC Section 35-526(j) for specific standards.
- iii. Parking structures—Design new parking structures to be similar in scale, materials, and rhythm of the surrounding

historic district when new parking structures are necessary.

Standard Specifications for Windows in Additions and New Construction

Consistent with the Historic Design Guidelines, the following recommendations are made for windows to be used in new construction:

- GENERAL: Windows used in new construction should be similar in appearance to those commonly found within the district in terms of size, profile, and configuration. While no material is expressly prohibited by the Historic Design Guidelines, a high quality wood or aluminum-clad wood window product often meets the Guidelines with the stipulations listed below.
- SIZE: Windows should feature traditional dimensions and proportions as found within the district.
- SASH: Meeting rails must be no taller than 1.25". Stiles must be no wider than 2.25". Top and bottom sashes must be equal in size unless otherwise approved.
- DEPTH: There should be a minimum of 2" in depth between the front face of the window trim and the front face of the top window sash. This must be accomplished by recessing the window sufficiently within the opening or with the installation of additional window trim to add thickness. All windows should be supplied in a block frame and exclude nailing fins which limit the ability to sufficiently recess the windows.
- TRIM: Window trim must feature traditional dimensions and architecturally appropriate casing and sloped sill detail.
- GLAZING: Windows should feature clear glass. Low-e or reflective coatings are not recommended for replacements. The glazing should not feature faux divided lights with an interior grille. If approved to match a historic window configuration, the window should feature true, exterior muntins.
- COLOR: Wood windows should feature a painted finish. If a clad or non-wood product is approved, white or metallic manufacturer's color is not allowed and color selection must be presented to staff.

### **FINDINGS:**

- a. The applicant is requesting a Certificate of Appropriateness for approval to construct two, 2-story residential structures and two, 1-story residential structures on the vacant lot at 909 N Hackberry, located within the Dignowity Hill Historic District.
- b. CONCEPTUAL APPROVAL The applicant received conceptual approval from the Historic and Design Review Commission hearing on November 18, 2020, to construct two, 2-story residential structures and two, 1-story residential structures on the vacant lot at 909 N Hackberry, with the following stipulations:
  - i. That the setbacks of both primary structures on N Hackberry be increased to be greater than the setback on N Hackberry of the structure at 527 Hays. **This stipulation has not been met.**
  - ii. That the applicant incorporate a foundation height that is consistent with the Guidelines. Per the submitted construction documents, the applicant has proposed foundation heights that appear consistent with the Guidelines; however, staff finds that foundation heights should be annotated on construction documents.
  - iii. That the applicant confirm consistency with the Guidelines in regards to lot coverage. **The applicant has not noted lot coverage calculations.**
  - iv. That standing seam metal roofs feature panels that are 18 to 21 inches wide, seams that are 1 to 2 inches in height, a standard galvalume finish and a crimped ridge seam or a low profile ridge cap. If a ridge cap is used, it must be reviewed and approved prior to installation. Board and batten siding should feature a smooth finish, boards that are 12 inches wide and battens that are approximately 2 inches in width. Columns should be six inches square and feature capital and base trim as well as chamfered corners.
  - v. That both driveways not exceed ten (10) feet in width, per the Guidelines, and that curb cuts be consistent with those found historically within the district. Staff recommends that the curb cut and driveway on Fayn Way be separated through the use of landscaping elements to maintain a ten (10) foot width. The applicant has not noted curb cut and driveway widths.
  - vi. That the proposed windows adhere to staff's standards for windows in new construction as noted in findings, and as noted in the applicable citations. The applicant has submitted a window product that generally meets staff's standards for windows in new construction.
- c. CONTEXT & DEVELOPMENT PATTERN This block on N Hackberry features a commercial structure constructed circa 1960 and one story historic structures. On the west side of N Hackberry, there are currently no residential structures that address N Hackberry.

- d. CURRENT LOT The current lot is void of any structures, and is bounded to the east by N Hackberry and to the north by Fayn Way, which is used as an alley.
- e. SETBACKS & ORITENTATION According to the Guidelines for New Construction, the front facades of new buildings are to align with front facades of adjacent buildings where a consistent setback has been established along the street frontage. Additionally, the orientation of new construction should be consistent with the historic examples found on the block. Staff finds that the proposed new construction should feature a setback that is greater than the side setback of the structure to the immediate south, which addresses Hays (527 Hays). Staff finds the proposed orientation to be appropriate and consistent with the Guidelines.
- f. ENTRANCES According to the Guidelines for New Construction 1.B.i., primary building entrances should be oriented towards the primary street. Per the submitted documents, the entrance of each primary structure will face N Hackberry. This is consistent with the Guidelines.
- g. SCALE, MASS & HEIGHT Per the Guidelines for New Construction 2.A.i., a height and massing similar to historic structures in the vicinity of the proposed new construction should be used. In residential districts, the height and scale of new construction should not exceed that of the majority of historic buildings by more than one-story. As noted in finding c, this block features one story structures; however, there are 2-story historic structure in the vicinity, specifically one block to the north. The applicant has noted an overall height of approximately twenty-eight (28) feet in height. Additionally, the applicant has proposed widths that are consistent with those found historically within the district. Generally, staff finds the proposed massing to be appropriate.
- h. FOUNDATION & FLOOR HEIGHTS Per the Guidelines for New Construction 2.A.iii., applicants should align foundation and floor-to-floor heights within one foot of floor-to-floor heights on adjacent historic structures. Per the submitted construction documents, the applicant has proposed foundation heights that appear consistent with the Guidelines; however, staff finds that foundation heights should be annotated on construction documents.
- i. ROOF FORMS The applicant has proposed for the new construction to feature front facing gabled roofs. Generally, staff finds the proposed primary roof forms to be consistent with the Guidelines.
- j. WINDOW & DOOR OPENINGS Per the Guidelines for New Construction 2.C.i., window and door openings with similar proportions of wall to window space as typical with nearby historic facades should be incorporated into new construction. Generally, staff finds the proposed window openings to be atypical in size with those found historically within the district. Staff finds that window groupings, heights, and widths should be modified to be consistent with those found historically within the district. As proposed, windows appear to feature widths and overall heights that are smaller in size as those found historically within the district. Additionally, staff finds that façade locations that are void of fenestration should be modified to feature window openings, specifically the south elevation of building A.
- k. PORCHES The applicant has proposed for both primary structures to feature both front and rear porches that are integral to the massing of both structures. Generally, staff finds the proposed porch depth and massing to be appropriate, as well as porch roof forms.
- l. LOT COVERAGE Per the submitted site plan, it appears that lot coverage is less than fifty (50) square feet; however, staff finds that the applicant should confirm consistency with the Guidelines.
- m. BUILDING SPACING The applicant has proposed approximately sixteen (16) feet between the primary structures and approximately thirty-three (33) feet between the primary and rear structures. Generally, staff finds the proposed building spacing to be appropriate; however, an increase in spacing between both primary structures would be appropriate.
- n. MATERIALS The applicant has proposed materials that include shingle roofs, board and batten siding, and wood columns. Board and batten siding should feature a smooth finish, boards that are 12 inches wide and battens that are approximately 2 inches in width. Columns should be six inches square and feature capital and base trim as well as chamfered corners.
- o. WINDOW MATERIALS The applicant has proposed Jeldwen aluminum clad wood windows. Generally, staff finds the proposed windows to be appropriate and consistent with staff's standards for windows in new construction.
- p. ARCHITECTURAL DETAILS Staff finds the proposed architectural details in regards to overall massing and porch forms to be appropriate; however, as noted in finding j, staff finds that window and door openings should be modified to be consistent with those found historically within the district. Additionally, staff finds that additional fenestration should be added to facades that are void of fenestration and that window groupings should be centered under the front facing gables.

- q. PARKING The applicant has proposed for parking to be located at the rear of each primary structure, with entrances on both N Hackberry and Fayn Way. Generally, staff finds the proposed parking to be appropriate.
- r. DRIVEWAY The applicant has proposed two driveways, one on N Hackberry and one on Fayn Way. Staff finds that both driveways should not exceed ten (10) feet in width, per the Guidelines, and that curb cuts should be consistent with those found historically within the district. Staff finds that the curb cut and driveway on Fayn Way should be separated through the use of landscaping elements to maintain a ten (10) foot width.
- s. LANDSCAPING The applicant has submitted a detailed landscaping plan that notes the installation of site pavers and decomposed granite throughout the site. Generally, staff finds the use of decomposed granite and site pavers to be appropriate; however, staff finds that front yards should feature fifty (50) landscaping ground cover.
- t. FENCING The applicant has proposed site fencing to include front, side and rear yard fencing. Staff finds the installation of fencing to be appropriate; however, front yard fencing should not exceed four (4) feet in height. Privacy fencing should not exceed six (6) feet in height. In the side yard, adjacent to Fayn Way, staff finds that fencing should not exceed six (6) feet in front of the primary façade of the two story structures. Driveway gates, if proposed, should be set behind the front façade of the two story structures.
- u. MECHANICAL EQUIPMENT The applicant has noted the location of mechanical equipment. The applicant is responsible from screening all mechanical equipment from view from the public right of way.
- v. REAR ACCESSORY STRUCTURES The applicant has proposed to construct two, 1-story residential structures on the west side of the lot at the rear of the two, 2-story primary structures. The proposed structures are to feature approximately 625 square feet each. The Guidelines for New Construction 5.A. notes that accessory structures should be designed to be visually subordinate to the principal structures in terms of their height, massing and form; should be no larger in plan than forty (40) percent of the primary structure's footprint; should feature complementary materials and simplified architectural details; and should feature similar window and door openings. Generally, staff finds the proposed rear structures to be consistent with the Guidelines.
- w. ACCESSORY STRUCTURES The Guidelines for New Construction 5.B. notes that new accessory structures should match the predominant orientation of accessory structures found along the block, and should follow historic setback patterns of similar structures along the streetscape or within the district. The applicant has proposed to locate the accessory structures at the rear of the lot, a location that is generally appropriate for the Dignowity Hill Historic District.
- x. MATERIALS (Accessory Structures) The applicant has proposed materials that include shingle roofs and board and batten siding. Board and batten siding should feature a smooth finish, boards that are 12 inches wide and battens that are approximately 2 inches in width.
- y. WINDOW MATERIALS (Accessory Structures) The applicant has noted the installation of aluminum clad wood windows. Staff finds this to be appropriate; however, the proposed windows should adhere to staff standards for windows in new construction, as noted in the applicable citations.
- z. ARCHITECTURAL DETAILS (Accessory Structures) Staff finds the proposed architectural details in regards to overall massing and porch forms to be appropriate; however, staff finds that window and door openings should be modified to be consistent with those found historically within the district. Additionally, staff finds that additional fenestration should be added to facades that are void of fenestration.

### **RECOMMENDATION:**

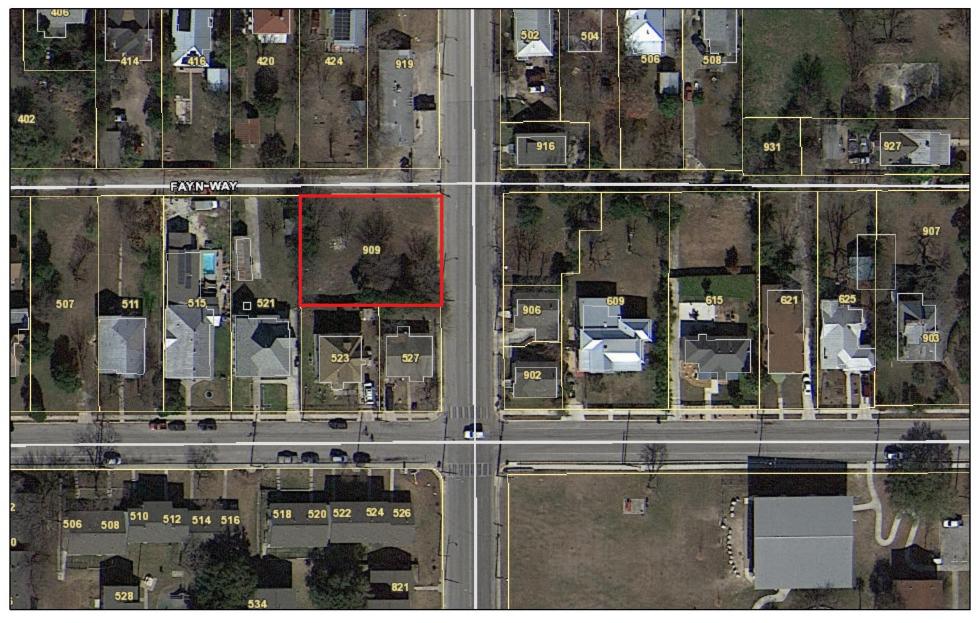
Staff recommends approval based on findings a through z with the following stipulations:

- i. That the setbacks of both primary structures on N Hackberry be increased to be greater than the setback on N Hackberry of the structure at 527 Hays, as noted in finding e.
- ii. That the applicant confirm that foundation heights are consistent with the Guidelines, as noted in finding h.
- iii. That the applicant confirm consistency with the Guidelines in regards to lot coverage, as noted in finding l.
- iv. That board and batten siding feature a smooth finish, boards that are 12 inches wide and battens that are approximately 2 inches in width, and that the proposed aluminum clad wood windows adhere to staff's standard specifications for windows in new construction, as noted in the applicable citations.

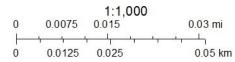
- v. That both driveways not exceed ten (10) feet in width, per the Guidelines, and that curb cuts be consistent with those found historically within the district. Staff recommends that the curb cut and driveway on Fayn Way be separated through the use of landscaping elements to maintain a ten (10) foot width.
- vi. That the front yard feature at least fifty (50) percent ground cover, that front yard fencing not exceed four (4) feet in height, that privacy fencing not extend past the front facades of the two story structures, and that driveway gates be setback behind the front façade of the two story structures.

A foundation inspection is to be scheduled with OHP staff to ensure that foundation setbacks and heights are consistent with the approved design. The inspection is to occur after the installation of form work and prior to the installation of foundation materials.

# City of San Antonio One Stop



July 12, 2021





### **PROJECT DATA**

PROJECT ADDRESS: 909 N HACKBERRY, SAN ANTONIO, TX 78202 OWNER: MICHAEL GARANSUAY

AREA & IMPERVIOUS COVER CALCULATIONS

LOT AREA: 10,934 SF GROSS BUILDING FOOTPRINTS (IMPERVIOUS): 3,630.15 GSF TOTAL BUILDING A: 1.199 GSF BUILDING B: 1.197.75 GSF ADU 1: 616.7 GSF ADU2: 616.7 GSF

GROSS HEATED SF: 5,148.75 GSF TOTAL

BUILDING A: 2,044.25 1ST FLOOR: 986 GSF 2ND FLOOR: 1,058.25 GSF BUILDING B: 1,987.5 GSF 1ST FLOOR: 985 GSF 2ND FLOOR: 1,002.5 GSF ADU 1: 558.5 GSF ADU2: 558.5 GSF

TOTAL IMPERVIOUS COVER: 5096.78 SF IMPERVIOUS COVERAGE AS PERCENTAGE OF LOT AREA: 46.6% ADU/PRIMARY BUILDING FOOTPRINT RATIO: 51.4%

**BUILDING STORIES: 2 STORY MAX** CONSTRUCTION TYPE: VB FIRE SPRINKLERS: NO OCCUPANCY: RESIDENTIAL SINGLE-FAMILY

### PROJECT SCOPE

THE PROJECT INCLUDES TWO NEW 3 BED/2 BATH SINGLE FAMILY RESIDENCES, AND TWO NEW 1 BED/1BATH ACCESORY DWELLING UNITS FOR AN UNIMPROVED LOT IN SAN ANTONIO'S DIGNOWITY HILL NEIGHBORHOOD.

# **VICINITY MAP**

# **LOCATION MAP**

DO NOT SCALE DRAWINGS, USE DIMENSIONS SHOWN.





## DRAWING SHEET INDEX

GENERAL STRUCTURAL SPB ENGINEERING, LLC COVER SHEET PROJECT STANDARDS & LEGENDS 14439 N.W. MILITARY HWY.. STE. 108-417 SAN ANTONIO, TEXAS 78231 STEVE P. BOURASSA, P.E. SENIOR ENGINEER/PARTNER CIVIL 210-273-5293, 210-355-0559 LIQUE DESIGN STUDIO STRUCTURAL NOTES 816 CAMARON STREET, SUITE 123 STRUCTURAL NOTES SAN ANTONIO, TX 78212 S-103 FASTENING SCHEDULE DAMAIN ESQUIVEL OVERALL FOUNDATION PLAN 210-549-4207 DAMIAN@LIQUE.US

S-504

S-505

FOUNDATION FRAMING PLAN - BUILDING A AND ADU 1 FOUNDATION FRAMING PLAN - BUILDING B AND ADU 2 SECOND FLOOR FRAMING PLANS -BUILDING A AND BUILDING B OVERALL ROOF PLAN **ROOF FRAMING PLANS - BUILDING A AND** ROOF FRAMING PLANS - BUILDING B AND

SECTIONS AND DETAILS

**SECTIONS AND DETAILS** 

**SECTIONS AND DETAILS** 

SECTIONS AND DETAILS

TRUSS ELEVATIONS

LANDSCAPE

C-101 GRADING PLAN

C-102 UTILITY PLAN

14607 SAN PEDRO AVE., SUITE 200 SAN ANTONIO, TX 78232 JON ROBINSON 210-831-8564 J.ROBINSON@HORIZONDESIGN-SA.COM

HORIZON DESIGN AND DEVELOPMENT

TREE PRESERVATION PLAN LANDSCAPE PLANTING PLAN LANDSCAPE CONSTRUCTION DETAILS LANDSCAPE CONSTRUCTION DETAILS

# **ARCHITECTURAL**

LIQUE DESIGN STUDIO 816 CAMARON STREET, SUITE 123 SAN ANTONIO, TX 78212 **BRIAN SOWELL** 512-417-3062 BRIAN@LIQUE.US

ARCHITECTURAL SITE PLAN FIRST FLOOR - BUILDING A SECOND FLOOR - BUILDING A

FIRST FLOOR - BUILDING B SECOND FLOOR - BUILDING B FLOOR PLAN - ADU

REFLECTED CEILING PLAN BUILDING A REFLECTED CEILING PLAN BUILDING B REFLECTED CEILING PLAN - ADU

A-151 **ROOF PLANS** OVERALL ELEVATIONS EXTERIOR ELEVATIONS BUILDING A EXTERIOR ELEVATIONS BUILDING A

EXTERIOR ELEVATIONS BUILDING B EXTERIOR ELEVATIONS BUILDING B TYPICAL FOUNDATION DETAILS EXTERIOR ELEVATIONS - ADU BUILDING SECTIONS BUILDING A TYPICAL WOOD FRAMING DETAILS TYPICAL WOOD FRAMING DETAILS

BUILDING SECTION BUILDING A BUILDING SECTIONS - ADU WALL SECTIONS CASEWORK ELEVATIONS - BUILDING A CASEWORK ELEVATIONS - BUILDING B

> CASEWORK ELEVATIONS - ADU VERTICAL CIRCULATION VERTICAL CIRCULATION

> > GLAZING ELEVATIONS

# **CONSTRUCTION OBSERVATION REQUIRED**

GENERAL CONTRACTOR IS <u>REQUIRED</u> TO SCHEDULE & COORDINATE THE FOLLOWING <u>MANDATORY</u> CONSTRUCTION OBSERVATION SITE VISITS WITH ARCHITECT PRESENT. PROVIDE NOTICE TO ARCHITECT AT LEAST 48 HOURS PRIOR TO SUCH VISITS. PRIOR TO BEGINNING WORK, PROVIDE ARCHITECT & OWNER WITH A CRITICAL PATH SCHEDULE SHOWING THE FOLLOWING CONSTRUCTION MILESTONES:

## INITIALS REQ'D SITE VISIT / MILESTONE

PRE-CONSTRUCTION SITE MEETING CONCRETE FORMWORK, PRIOR TO POURING CONCRETE, PRIOR TO FRAMING & MASONRY ROUGH ELEC., MOUNTED BOXES PRIOR TO

DRY-IN PRIOR TO INSTALLING CLADDING PUNCH LIST REVIEW

SUBSTANTIAL COMPLETION PRIOR TO C.O.

ARCHITECT'S INITIALS ARE REQUIRED TO THE LEFT OF EACH SITE VISIT LISTED PRIOR TO PROCEEDING WITH SUBSEQUENT WORK, AND INDICATE ONLY THAT ARCHITECT WAS PRESENT & PROVIDED WITH THE OPPORTUNITY TO OBSERVE CONSTRUCTION AT THAT PHASE.

# TYPICAL WALL TYPES

CONSTRUCT ALL WALLS AS LABELED ON FLOOR

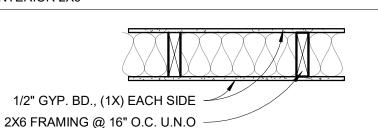
PLAN GRAPHICS FOR VISUAL REFERENCE ONLY PROVIDE INSULATION AT ALL EXTERIOR WALLS PER

SPECIFICATIONS. PROVIDE INSULATION IN ALL INTERIOR BATHROOM WALLS.

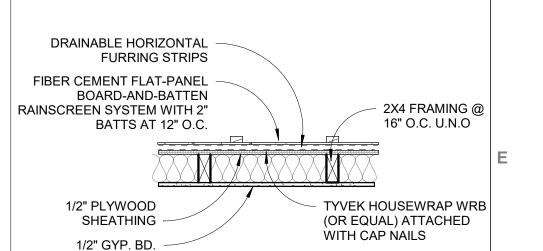
### **INTERIOR 2X4**

1/2" GYP. BD., (1X) EACH SIDE 2X4 FRAMING @ 16" O.C. U.N.O

**INTERIOR 2X6** 



**EXTERIOR 2X6** 



**COVER SHEET** 

G-001



OMPENSATION TO LIQUE DESIGN STUDIO, LLC

ISSUED FOR HISTORICAL BOARD | 2021.06.29 INTERMEDIATE CONSTRUCTION 2021.02.12 DOCUMENTS DATE DESCRIPTION SCHEDULE OF REVISIONS

### **CONSTRUCTION DOCUMENTS** HISTORICAL BOARD REVIEW

PROJECT NUMBER: 2020132 PROJECT DATE: 2021.06.29 PROJECT MANAGER: B. SOWELL

PROJECT TEAM: S. JURADO, E. SOWELL

### **GENERAL**

- 1. THIS CONSTRUCTION SET IS PRESENTED ON 22X34 SHEETS
- 2. ELECTRICAL AND PLUMBING LINES SHALL RUN CONCEALED AND FRAMING SHALL BE OF ADEQUATE DIMENSIONS TO ACCOMPLISH THIS RESULT WITHOUT CHANGES IN THE WALL PLANE OR CEILING PLANE
- . WHEN REFERENCE IS MADE TO A MATERIAL SYSTEM ALL PARTS AND MATERIALS PERTINENT TO THE MANUFACTURER'S SYSTEM SHALL BE FURNISHED AND INSTALLED ACCORDING TO THE MANUFACTURER'S INSTRUCTIONS AND SPECIFICATIONS.
- 4. ALL INFORMATION ON EXISTING CONDITIONS WAS SUPPLIED TO THE ARCHITECT BY THE OWNER. CONTRACTOR IS REQUESTED TO VERIFY, ON-SITE, ALL DIMENSIONS & CONDITIONS BEFORE STARTING CONSTRUCTION. REPORT ANY DISCREPANCIES IMMEDIATELY TO THE ARCHITECT.
- 5. FINISHES AND TEXTURES ARE TO BE SELECTED BY OWNER
- 6. REPAIR ANY DAMAGED AREAS PRIOR TO APPLYING FINISHES
- THE CONTRACT DOCUMENTS ARE COMPLIMENTARY, AND WHAT IS REQUIRED BY ONE SHALL BE AS BINDING AS IF REQUIRED BY ALL. ALL CONTRACT DOCUMENTS AND ENGINEERING DRAWINGS ARE TO BE USED TOGETHER. GENERAL CONTRACTOR AND SUBCONTRACTORS ARE RESPONSIBLE TO REVIEW COMPLETE SETS OF DOCUMENTS AND REPORT ANY DISCREPANCIES TO THE ARCHITECT PRIOR TO THE START OF CONSTRUCTION
- CONTRACTOR SHALL MAINTAIN A NEAT PREMISE AND SHALL THOUROUGHLY CLEAN ALL FINISHED SURFACES INSIDE AND OUTSIDE OF THE PROJECT. 9. ALL SUBCONTRACTORS ARE RESPONSIBLE FOR A
- COMPLETE JOB WITHIN THEIR DISCIPLINES AND SHALL NOTIFY THE CONTRACTOR AND THE OWNER OR HIS AUTHORIZED AGENT OF ANY NORMALLY REQUIRED ITEMS NOT SPECIFICALLY IDENTIFIED IN THE DRAWINGS
- 10. USE NUMERICAL DIMENSIONS SHOWN, DO NOT SCALE THE DRAWINGS.
- 11. ALL WORK AND MATERIALS ARE TO COMPLY IN EVERY RESPECT WITH THE LATEST REQUIREMENTS OF ALL APPLICABLE CITY, COUNTY, AND STATE CODES, LOCAL REGULATIONS AND THE DIRECTION OF THE BUILDING INSPECTOR FOR SUCH BUILDING LAWS. REGULATIONS AND DIRECTIONS ARE TO BE CONSIDERED AS PART OF THESE PLANS.
- 12. FOR ANY ITEM IDENTIFIED IN THE CONTRACT DOCUMENTS THAT IS REASONABLY INFERABLE AS A COMPONENT IN A SYSTEM AND REQUIRED FOR THE PERFORMANCE OF THAT SYSTEM. THE GENERAL CONTRACTOR SHALL INCLUDE ALL OTHER COMPONENTS IN THE WORK WHICH ARE NECESSAR FOR THE COMPLETION AND FULL OPERATIONAL PERFORMANCE OF THAT SYSTEM.
- 13. THE CONTRACTX DOCUMENTS INDICATE THE GENERAL DESIGN INTENT, BUT DO NOT NECESSARILY DESCRIBE ALL WORK REQUIRED FOR FULL PERFORMANCE AND COMPLETION. THE CONTRACTOR SHALL PROVIDE ALL ITEMS REQUIRED FOR THE PROPER EXECUTION AND COMPLETION OF THE WORK
- 14. THE GENERAL CONTRACTOR SHALL VERIFY IN THE FIELD AND COORDINATE BETWEEN THE TRADES, ALI CONDITIONS BOTH NEW AND EXISTING WHICH AFFECT WORK TO BE DONE OR RELEVANT THERETO INCLUDING BUT NOT LIMITED TO, PROPERTY LINE DIMENSIONS, SETBACKS, EASEMENTS. RESTRICTIONS, EXACT LOCATIONS OF ALL CONSTRUCTION, EXISTING AND NEW, EXISTENCE AND LOCATIONS OF ASBESTOS OR OTHER UNKNOWN TOXIC MATERIALS, DRIVEWAYS, WALKS, APRONS, UTILITIES, GRADES, AND DRAINAGE, THE CONTRACTOR IS RESPONSIBLE FOR THE DISCOVERY OF ASBESTOS AND OTHER REGULATED TOXIC MATERIALS AND SHALL BEAR ADMINISTRATIVE RESPONSIBILITY FOR CONFORMANCE TO FEDERAL STATE, AND LOCAL JURISDICTIONAL REQUIREMENTS REGARDING THE DISPOSITION OF HAZARDOUS MATERIALS.
- 15. SHOULD ANY QUESTIONS ARISE OR DISCREPANCIES ON THE DRAWINGS BE NOTED PRIOR TO BEGINNING OF CONSTRUCTION OR DURING ANY PHASE OF CONSTRUCTION, CONTRACTOR SHALL IMMEDIATELY NOTIFY THE ARCHITECT FOR REVIEW AND CLARIFICATION BEFORE PROCEEDING WITH THAT PORTION OF THE WORK OR ANY PART RELATED THERETO
- 16. CONTRACTOR SHALL OBTAIN AND BE RESPONSIBLE FOR ALL FEES AND PERMITS REQUIRED AND ASSOCIATED WITH ALL PHASES OF THE WORK AND WITHIN SCOPE OF THE CONTRACT DOCUMENTS INCLUDING BUT NOT LIMITED TO: BUILDING PERMIT FEES. MEP FEES. WATER FEES. SEWER FEES. DRIVEWAY FEES, AND SIDEWALK FEES. THE LOCATION OF UTILITIES SHOWN ON THE SITE PLANS ARE BASED ON INFORMATION AVAILABLE. CONTRACTOR SHALL VERIFY THE EXACT LOCATION
- OF ALL UTILITIES BEFORE STARTING CONSTRUCTION 17. DESIGN AND CONSTRUCTION PROCESSES TO COMPLY WITH HOA, LOCAL AND STATE REISDENTIAL BUILDING CODE REQUIREMENTS.
- 18. CONTRACTOR SHALL REPORT IMMEDIATELY, TO THE ARCHITECT (IN WRITING) ANY EXISTING CONDITIONS THAT MAY AFFECT PERFORMANCE OF THE EXISTING AND NEW STRUCTURES. 19. CONTRACTOR SHALL BE HELD RESPONSIBLE FOR
- THE EXECUTION OF THE PROJECT IN A COMPLETE AND WORKMAN LIKE MANNER, CONFORMING TO THE BEST STANDARDS OF PRACTICE IN VARIOUS TRADES 20. NO VEHICLE IS PERMITTED ON ADJACENT PROPERTY AND ANY DAMAGE DONE TO EXISTING DRIVES AND WALKS OR OTHER STRUCTURES WILL BE REPAIRED

OR REPLACED AND CHARGED TO THE PERSON OR

COMPANY RESPONSIBLE. 21. TRADE NAMES AND BRANDS NOTED ON THE CONTRACT DOCUMENTS ARE FOR QUALITY STANDARDS ONLY. SUBSTITUTIONS OF "EQUAL" PRODUCTS MAY BE MADE WITH THE OWNER'S PERMISSION.

### **FOUNDATIONS & CONCRETE SLABS**

1. SLOPE DRAINAGE 6" WITHIN THE FIRST 10 FT. FROM THE FOUNDATION WALL. IF PHYSICAL OBSTRUCTIONS OR LOT LINES PROHIBIT THE 10 FT. DISTANCE, A 2-5 PERCENT SLOPE SHALL BE PROVIDED TO AN APPROVED ALTERNATIVE METHOD OF DIVERTING THE WATER AWAY FROM THE FOUNDATION. IMPERVIOUS SURFACES SHALL ALSO BE SLOPED A MINIMUM OF 2 PERCENT FOR 10 FT. AWAY FROM STRUCTURES TO AN APPROVED DRAINAGE WAY.

# TREATMENT FOR WOOD FRAMING

- 1. WEATHER EXPOSED GLU-LAM, BEAMS AND POSTS SHALL BE PRESSURE TREATED OR SHALL BE WOOD OF NATURAL RESISTANCE TO DECAY.
- 2. ALL WOOD FRAMING CONTACTING THE GROUND OR CONCRETE TO BE TREATED. 3. ALL WOOD BLOCKING TO BE FIRE RETARDANT

- 1. THE FOLLOWING WINDOWS SHOULD BE FULLY TEMPERED:
- SLIDING/SWINGING GLASS DOORS GLAZING IN WALLS AND ENCLOSURES FACING
- HOT TUBS, SPAS, WHIRPOOLS, SAUNAS, STEAM ROOMS, BATHTUBS, SHOWERS AND SWIMMING POOLS WHERE THE GLAZING IS LEES THAN 60 INCHES ABOVE THE STANDING SURFACE WITHIN THE COMPARTMENT AND WITHIN 60 INCHES HORIZONTALLY OF THE WATER'S EDGE.
- GLAZING WITHIN A 24" ARC OF A DOOR THAT IS LESS THAN 60 INCHES ABOVE THE FLOOR SAFETY GLAZING REQUIRED ON A WALL LESS THAN 180 DEGREES FROM THE PLANE OF THE **DOOR** IN A CLOSED POSITION AND WITHIN 24" OF
- HINGE SIDE OF AN IN-SWING DOOR. GLAZING WHERE THE EXPOSED AREA IS GREATER THAN 9 SQFT.. BOTTOM IS LESS THAN 18 IN. AND AT LEAST 36 IN. ABOVE THE FLOOR, AND ADJACENT TO A WALKING SURFACE.
- WITHIN 60 IN. OF THE BOTTOM TREAD OF A STAIRWAY AND LESS THAN 36 IN. ABOVE THE LANDING. GLAZING IN GUARDS AND RAILINGS
- GLAZING ADJACENT TO STAIRWAYS, LANDINGS. AND RAMPS WITHIN 36 IN. HORIZONTALLY OF THE WALKING SURFACE LESS THAN 36 IN. ABOVE THE WALKING SURFACE.
- 2. EMERGENCY ESCAPE AND RESCUE OPENINGS SHALL HAVE A NET CLEAR OPENING OF NOT LESS THAN 5.7 SQFT. THE NET CLEAR OPENING DIMENSIONS REQUIERED SHALL BE OBTAINED BY THE NORMAL OPERATION OF THE EMERGENCY ESCAPE AND RESCUE OPENING FROM THE INSIDE THE NET CLEAR HEIGHT OF THE OPENING SHALL BE NOT LESS THAN 24 INCHES AND THE NET CLEAR WITH SHALL BE NOT LESS THAN 20 INCHES. WHERE A WINDOW IS PROVIDED AS AN EMERCENCY ESCAPE AND RESCUE OPENING, IT SHALL HAVE A SILL HEIGHT OF NOT MORE THAN 44" ABOVE THE

### WALLS

FLOOR.

- . POSITIVE CONNECTION SHALL BE PROVIDED TO **ENSURE AGAINST UPLIFT AND LATERAL**
- DISPLACEMENT 2. ALL FASTENERS USED FOR ATTACHMENT OF SIDING & INTO PRESSURE TREATED LIMBER SHALL BE OF A CORROSION RESISTANT TYPE
- 3. FIRE-BLOCK IN CONCEALED SPACES OF STUD WALLS/PARTITIONS, VERTICALLY AT CEILING/FLOOR LEVELS. & HORIZONTALLY AT 10 FT. INTERVALS. FIRE-BLOCK AT SOFFITS, DROP CEILINGS/SIMILAR LOCATIONS & IN CONCEALED SPACES AT THE TOP/BOTTOM OF STAIR STRINGERS.

### STAIRWAYS AND RAMPS

- . RISE SHALL BE MAXIMUM 7.75"; RUN SHALL BE 10" MINIMUM; HEADROOM 6'-8" MINIMUM; WIDTH 36" MINIMUM, 31.5" BETWEEN A HANDRAIL ON ONE SIDE AND 27" WITH HANDRAILS ON TWO SIDES. VARIATION BETWEEN RISER HEIGHTS 3/8" MAXIMUM. A NOSING NOT LESS THAN .75 INCHES BUT NO MORE THAN 1.25 INCHES SHALL BE PROVIDED ON STAIRWAYS WITH SOLID RISERS WHERE THE TREAD DEPTH IS LESS THAN 11 INCHES. THE LEADING EDGE OF TREADS SHALL PROJECT NO MORE THAN 1.25 INCHES BEYOND THE READ BELOW. OPEN RISERS ARE PERMITTED. PROVIDE THE OPENING BETWEEN THE TREADS DOES NOT PERMIT THE PASSAGE OF A 4" SPHERE. (OPENINGS ARE NOT LIMITED WHEN THE STAIR HAS A RISE OF 30" OR LESS)
- STAIRWAYS WITH 4 OR MORE RISERS SHALL HAVE A HANDRAIL ON ONE SIDE 34" TO 38" ABOVE THE TREAD NOSING. CIRCULAR HANDRAILS SHALL HAVE AN OUTSIDE DIAMETER OF 1.25"-2". A MINIMUM CLEARANCE OF 1.5" SHALL BE MAINTAINED FROM THE
- WALL OR OTHER SURFACE. . GUARDS SHALL BE 42" MINIMUM HEIGHT (UNLESS ACTING AS A HANDRAIL/GUARD FOR A STAIRWAY: THE GUARD HEIGHT MAU BE 34"-38" IN HEIGHT), WITH OPENINGS LESS THAN 4" CLEAR (GUARDS ON THE OPEN SIDES OF STAIRS MAY HAVE 4 3/8" OPENINGS)
- **USABLE SPACES UNDERNEATH** ENCLOSED/UNENCLOSED STAIRWAYS SHALL BE PROTECTED BY A MINIMUM OF 1/2" GYPSUM BOARD

### GARAGE AND CARPORT

1. GARAGE SHALL BE SEPARATED FROM THE DWELLING UNIT & ATTIC AREA BY 1/2" GYPSUM BOARD APPLIED TO THE GARAGE SIDE. GARAGE BENEATH HABITABLE ROOMS SHALL BE SEPARATED BY NOT LESS THAN 5/8" TYPE X GYPSUM BOARD. STRUCTURE SUPPORTING FLOOR/CEILING ASSEMBLIES USED FOR REQUIRED SEPARATIONS SHALL HAVE 1/2" GYPSUM BOARD INSTALLED MINIMUM. DOOR OPENINGS FROM THE GARAGE TO THE DWELLING SHALL BE SOLID WOOD/STEEL DOORS OR HONEYCOMB STEEL DOORS NO LESS THAN A 3/8" THICK OR A 20-MINUTE RATED FIRE DOOR. DOORS SHALL BE SELF-CLOSING & SELF-LATCHING.

# 2. DUCTS PENETRATING THE GARAGE TO DWELLING

- SEPARATION SHALL BE A MINIMUM OF 26 GAUGE WITH NO OPNINGS INTO THE GARAGE PENETRATIONS THROUGH THE GARAGE TO DWELLING
- SEPARATION WALL (OTHER THAN DUCTS AS LISTED ABOVE) SHALL BE FIRE-BLOCKED. GARAGE AND CARPORT FLOOR SURFACES SHALL BE
- NON-COMBUSTIBLE MATERIAL AND SLOPE TO DRAIN TOWARDS THE GARAGE DOOR OPENING. 6. APPLIANCES AND RECEPTACLES INSTALLED IN
- GARAGE GENERATING A GLOW, SPARK OR FLAME SHALL BE LOCATED 18" ABOVE FLOOR UNLESS IT IS LISTED AS FLAMMABLE VAPOR IGNITION RESISTANT. PROVIDE PROTECTIVE POST OR OTHER IMPACT BARRIER FROM VEHICLES.

### **ELECTRICAL**

- NO ELECTRICAL PANELS IN CLOSETS OF BATHROOMS. MAINTAIN A CLEARANCE OF 36" IN FRONT OF PANELS, 30" WIDE OR WIDTH OF **EQUIPMENT AND 6'-6" HIGH FOR HEADROOM**
- 2. PROVIDE A MINIMUM 3 LUG INTERSYSTEM BONDING BUSBAR AT THE MAIN ELECTRICAL SERVICE.
- 3. A CONCRETE-ENCASED ELECTRODE (UFER) CONSISTING OF 20' OF REBAR OR #4 COPPER WIRE PLACED IN THE BOTTOM OF A FOOTING IS REQUIRED FOR ALL NEW CONSTRUCTION. BOND ALL METAL GAS AND WATER PIPES TO GROUND. ALL GROUND CLAMPS SHALL BE ACCESSIBLE AND OF AN APPROVED TYPE.
- ALL 15/20 AMPERE RECEPTACLES INSTALLED SHALL BE LISTED TAMPER-RESISTANT RECEPTACLES.
- 5. ALL BRANCH CIRCUITS SUPPLYING 15/20 AMPERE OUTLETS IN FAMILY ROOMS, DINING ROOMS, LIVING ROOMS, PARLORS, LIBRARIES, DENS, BEDROOMS, SUNROOMS, RECREATION ROOMS, CLOSETS, HALLWAYS, KITCHENS, LAUNDRY ROOMS OR SOMILAR ROOMS/AREAS SHALL BE PROTECTED BY A LISTED COMBINATION TYPE ARC-FAULT CIRCUIT
- 6. PROVIDE A MINIMUM OF ONE 20A CIRCUIT TO BE USED FOR THE LAUNDRY RECEPTACLE. PROVIDE A MINIMUM OF ONE 20A CIRCUIT FOR BATHROOM RECEPTACLE OUTLETS.
- 7. PROVIDE AT LEAST 1 OUTLET IN BASEMENT, GARAGES, LAUNDRY ROOMS, DECKS, BALCONIES, PORCHES AND WITHIN 3' OF THE OUTSIDE OF EACH BATHROOM BASIN.
- 8. ALL DWELLINGS MUST HAVE ONE EXTERIOR OUTLET AT THE FRONT AND THE BACK OF THE DWELLING.
- 9. GARAGE RECEPTACLES SHALL NOT SERVE OUTLETS OUTSIDE THE GARAGE. **EXCEPTION: GARAGE** CIRCUIT MAY SERVE READILY ACCESSIBLE **OUTDOOR RECEPTACLE OUTLETS.** A MINIMUM OF 1 RECEPTACLE SHALL BE PROVIDED FOR EACH CAR SPACE
- 10. KITCHENS, DINING ROOMS, PANTRIES. BREAKFASTNOOKS, AND SIMILAR AREAS MUST HAVE A MINIMUM OF TWO 20A CIRCUITS. KITCHEN, PANTRY, BREAKFAST NOOKS, DINING ROOMS, WORK SURFACES AND SIMILAR AREAS COUNTER OUTLETS MUST BE INSTALLED IN EVERY COUNTER SPACE 12" OR WIDER, NOT GREATER THAN 20" ABOVE COUNTER. ISLAND COUNTER SPACES SHALL HAVE AT LEAST 1 RECEPTACLE OUTLET UNLESS A RANGE TOP OR SINK IS INSTALLED THEN 2 RECEPTACLES MAY BE REQUIRED, 1 RECEPTACLE IS REQUIRED FOR PENINSULAR COUNTER SPACES. RECEPTACLES SHALL BE LOCATED BEHIND KITCHEN SINKS IF THE COUNTER AREA DEPTH BEHING THE SINK IS MORE THAN 12" FOR STRAIGHT COUNTERS AND 18" FOR
- CORNER INSTALLATIONS. 1. RECEPTACLES SHALL BE INSTALLED AT 12" O.C. MAXIMUM IN WALLS STARTING 6' MAXIMUM FROM THE WALL END. WALLS LONGER THAN TWO FEET SHALL
- HAVE A RECEPTACLE. HALLWAY WALLS NO LONGER THAN 10' SHALL HAVE A RECEPTACLE IN HALLWAY. 12. ALL LIGHTING/FAN FIXTURES LOCATED IN WET OR DAMP LOCATIONS SHALL BE RATED FOR THE
- APPLICATION. 13. GFCI OUTLETS ARE REQUIRED: FOR ALL KITCHEN RECEPTACLES THAT ARE DESIGNED TO SERVE COUNTERTOP SURFACES, DISHWASHERS, BATHROOMS, IN UNDER-FLOOR SPACES OR BELOW GRADE LEVEL, IN UNFINISHED BASEMENTS, CRAWL SPACE LIGHTING OUTLETS, IN EXTERIOR OUTLETS, WITHIN 6' OF A LAUNDRY/UTILITY/WET BAR SINKS, LAUNDRY AREAS, AND IN ALL GARAGE OUTLETS INCLUDING OUTLETS DEDICATED TO A SINGLE
- DEVICE OR GARAGE DOOR OPENER. 14. CARBON-MONOXIDE ALARMS SHALL BE INSTALLED IN DWELLING UNITS WITH FUEL-BURNING APPLIANCES OR WITH ATTACHED GARAGES:
- A. OUTSIDE OF EACH SEPARATE SLEEPING AREA IN THE IMMEDIATE VICINITY OF BEDROOMS
- B. ON EVERY LEVEL OF A DWELLING UNIT **INCLUDING BASEMENTS** C. ALTERATIONS, REPAIRS, OR ADDITIONS
- EXCEEDING 1,000 DOLLAS (MAY BE BATTERY OPERATED)
- 15. SMOKE ALARMS SHALL BE INSTALLED: A. IN EACH ROOM USED FOR SLEEPING PURPOSES
- IN THE IMMEDIATE VICINITY OF BEDROOMS C. IN EACH STORY, INCLUDING BASEMENTS. 16. SHALL NOT BE INSTALLED WITHIN 20FT HORIZONTALLY OF COOKING APPLIANCES AND NO CLOSER THAN 3FT TO MECHANICAL REGISTERS,
- CEILING FANS AND BATHROOM DOORS WITH A BATHTUB OR SHOWER UNLESS THIS WOULD PREVENT PLACEMENT OF A SMOKE DETECTOR 17. ALL SMOKE AND CARBON-MONOXIDE ALARMS SHALL BE HARDWIRE WITH A BATTERY BACKUP (SMOKE
- 18. SMOKE DETECTORS WITHIN 10 FEET TO 20 FEET OF THE STOVE SHALL BE IONIZATION TYPE WITH ALARM SILENCING SWITCH. 19. ALL 15/20 AMPERE RECEPTACLES IN WET LOCATIONS SHALL HAVE IN-USE (BUBBLE) COVERS INSTALLED. ALL RECEPTACLES IN WET LOCATIONS SHALL ALSO

BE LISTED WEATHER-RESISTANT TYPE.

ALARMS SHALL HAVE A 10-YR SEALED BATTERY)

### PLUMBING

- UNDERFLOOR CLEANOUTS SHALL NOT BE MORE THAN 5' FROM AN UNDERFLOOR ACCESS, ACCESS
- DOOR OR TRAP DOOR ABS PIPING SHALL NOT BE EXPOSED TO DIRECT SUNLIGHT UNLESS PROTECTED BY WATER BASED
- SUNTHETIC LATEX PAINTS PVC PIPING SHALL NOT BE EXXPOSED TO DIRECT SUNLIGHT UNLESS PROTECTED BY WATER BASED SYNTHETIC LATEX PAINT, .04" THICK WRAP OR
- OTHERWISE PROTECTED FROM UV DEGRADATION. UNDERGROUND WATER SUPPLY LINES SHALL HAVE A 14 AWG BLUE TRACER WIRE
- THE ADJACENT SPACE NEXT TO SHOWERS WITHOUT THRESHOLDS SHALL BE CONSIDERED A "WET LOCATION" WHEN USING THE CRC, CBC AND THE DOMESTIC HOT WATER LINES SHALL BE INSULATED.
- INSULATION SHALL BE THE THICKNESS OF THE PIPE DIAMETER UP TO 2" IN SIZE AND MINIMUM 2" THICKNESS FOR PIPES LARGER THAN 2" IN DIAMETER
- PROVIDE ANTI-SIPHON VALVES ON ALL HOSE BIBS FLOOR DRAINS SHALL BE PROVIDED WITH A TRAP
- PROVIDE BACKFLOW PREVENTION DEVICE ON WATER SUPPLY LINE COMPLYING WITH 2018 IRC AND OTHER REQUIREMENTS OF THE AUTHORITY HAVING JURISDICTION

### MECHANICAL

VENT DRYER TO OUTSIDE OF BUILDING (NOT TO UNDER-FLOOR AREA), VENT LENGTH SHALL BE 14 FT MAXIMUM. SHALL TERMINATE A MINIMUM OF 3' FROM THE PROPERTY LINE AND ANY OPENING INTO THE BUILDING.

# **ABBREVIATIONS**

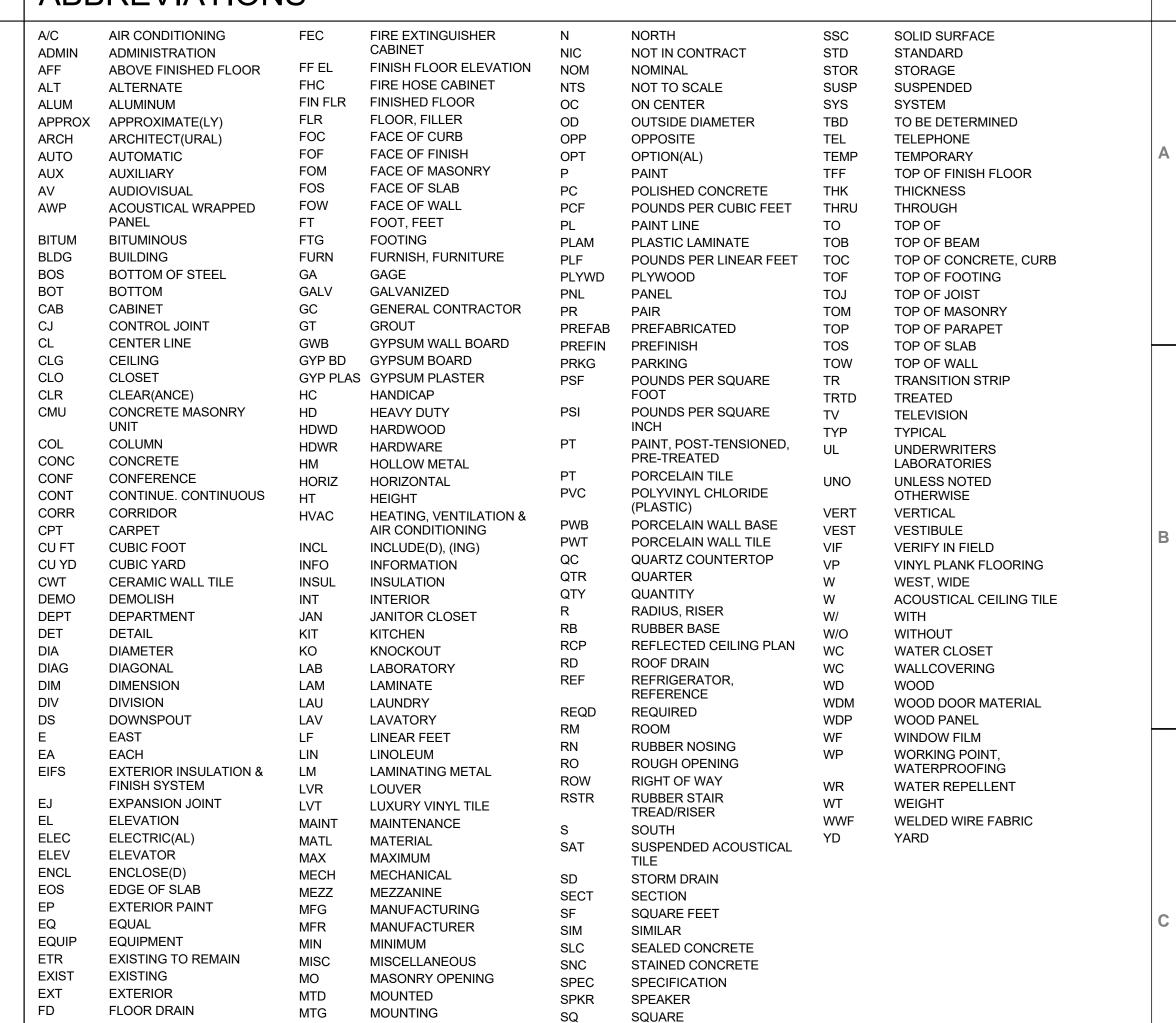
FIRE EXTINGUISHER

**SECTION SYMBOLS** 

MTL

METAL

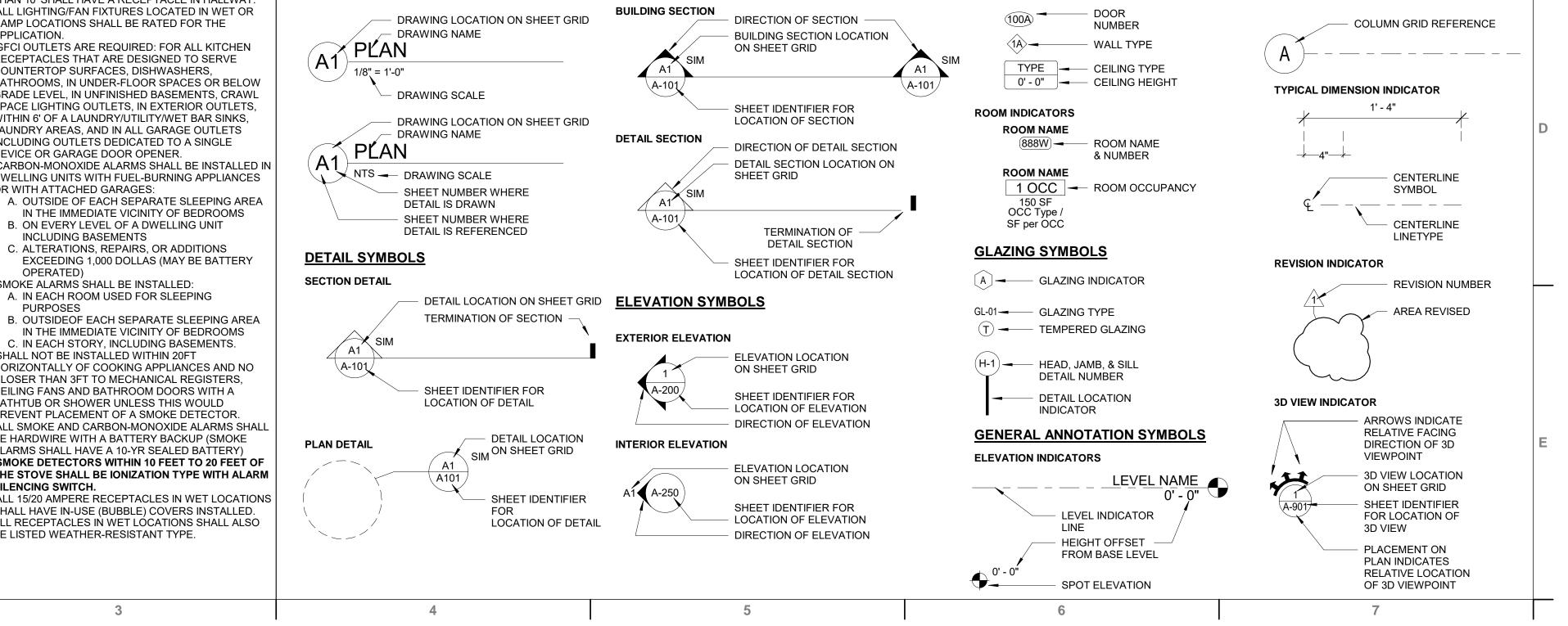
PLAN SYMBOLS



STAINLESS STEEL

# **GRAPHIC SYMBOL LEGEND**

DRAWING TITLE SYMBOLS



DESIGN STUDIO

WWW.LIQUE.US | 210.549.4207 LIQUE DESIGN STUDIO, LLC **TEXAS REGISTRATION NUMBER: BR 3647** 816 CAMARON ST., SUITE #123, SAN ANTONIO, TX 78212

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# 0

ISSUED FOR HISTORICAL BOARD | 2021.06.29 INTERMEDIATE CONSTRUCTION 2021.02.12 **DOCUMENTS** DESCRIPTION DATE SCHEDULE OF REVISIONS

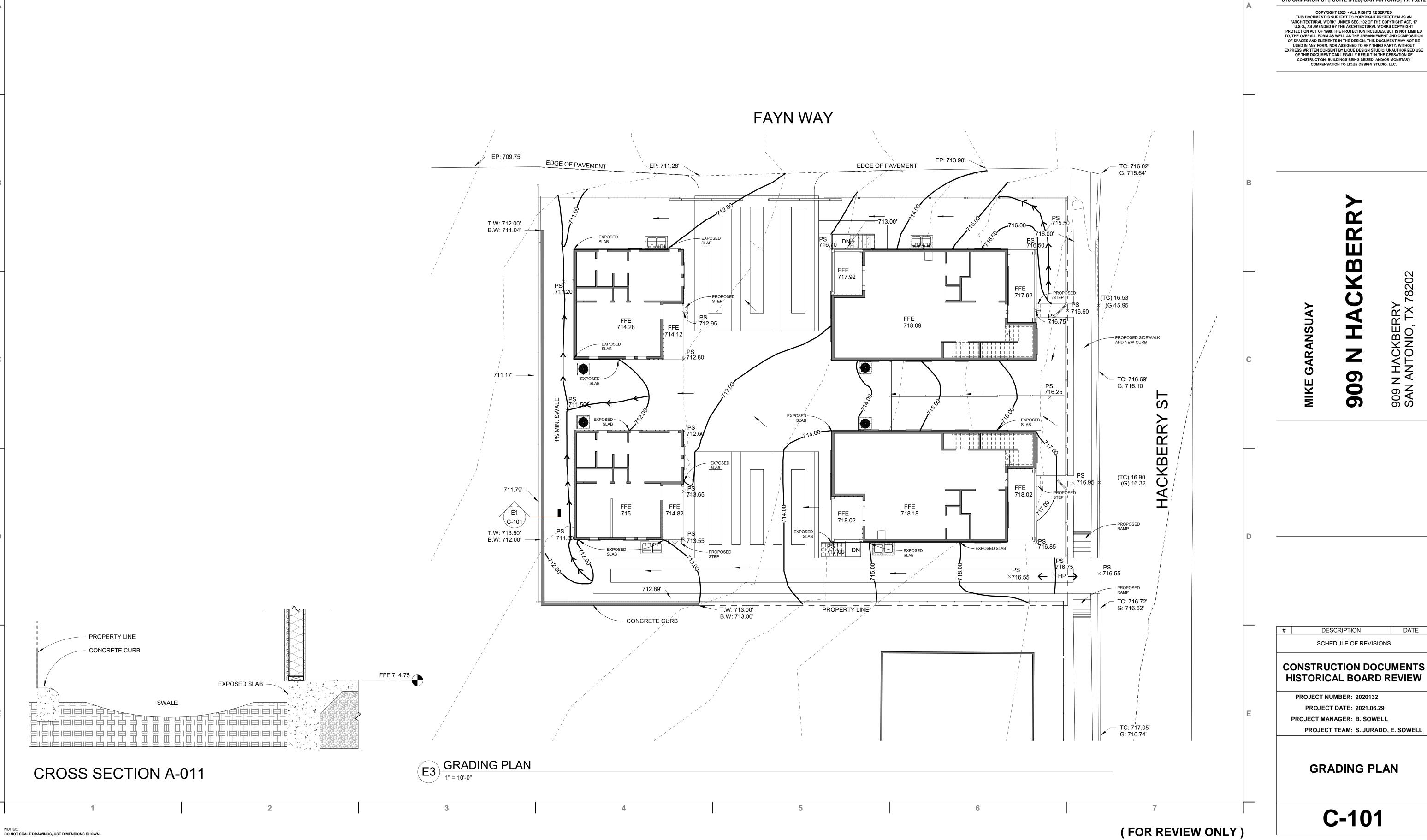
### CONSTRUCTION DOCUMENTS HISTORICAL BOARD REVIEW

PROJECT NUMBER: 2020132 PROJECT DATE: 2021.06.29 PROJECT MANAGER: B. SOWELL PROJECT TEAM: S. JURADO, E. SOWELL

**PROJECT STANDARDS & LEGENDS** 

G-002

**GENERAL ANNOTATION SYMBOLS CONT** 



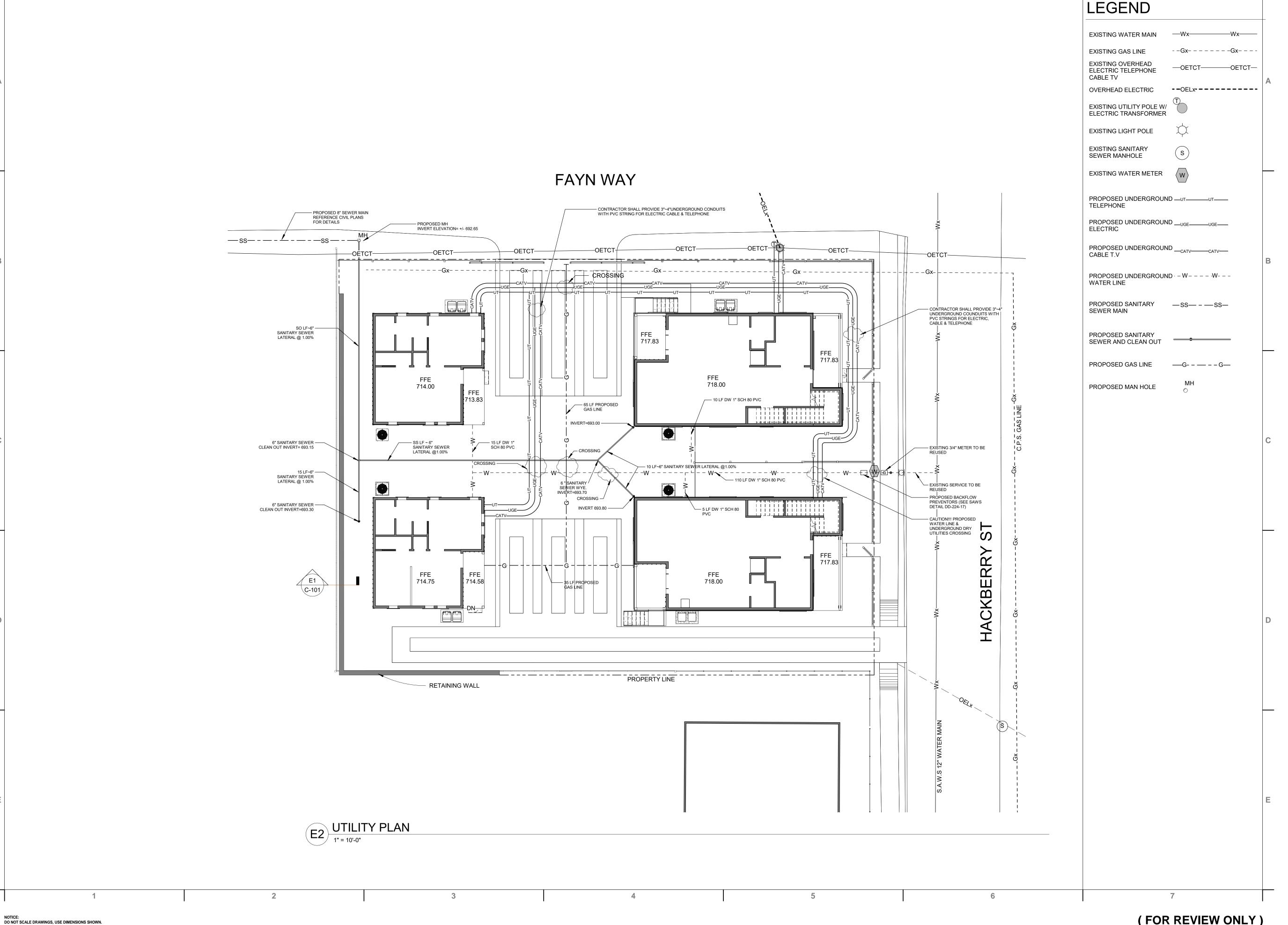
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DATE

### **CONSTRUCTION DOCUMENTS** HISTORICAL BOARD REVIEW

PROJECT MANAGER: B. SOWELL



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# KBERR

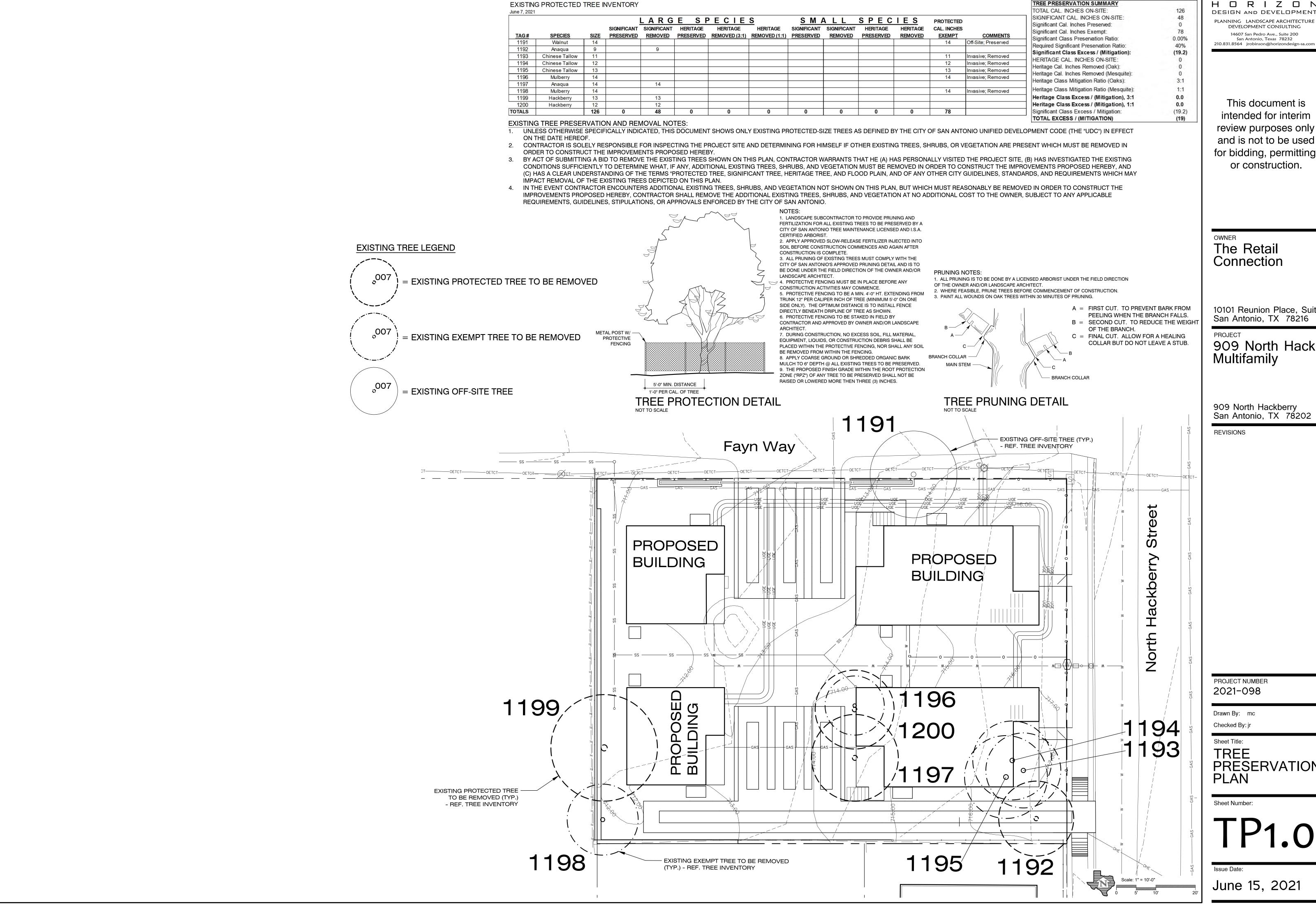
DATE DESCRIPTION SCHEDULE OF REVISIONS

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PROJECT NUMBER: 2020132 PROJECT DATE: 2021.06.29 PROJECT MANAGER: B. SOWELL PROJECT TEAM: S. JURADO, E. SOWELL

**UTILITY PLAN** 

C-102



HORIZON DESIGN AND DEVELOPMENT PLANNING LANDSCAPE ARCHITECTURE DEVELOPMENT CONSULTING 14607 San Pedro Ave., Suite 200 San Antonio, Texas 78232

> This document is intended for interim review purposes only and is not to be used for bidding, permitting, or construction.

The Retail Connection

10101 Reunion Place, Suite 160 San Antonio, TX 78216

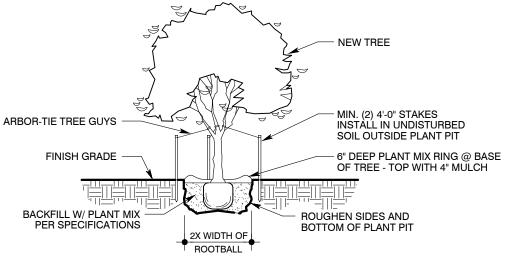
909 North Hackberry Multifamily

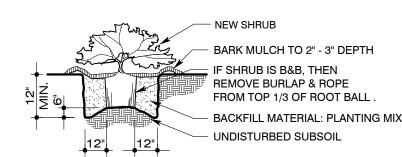
909 North Hackberry San Antonio, TX 78202

PROJECT NUMBER

Drawn By: mc

TREE PRESERVATION PLAN





# TREE PLANTING DETAIL

# SHRUB PLANTING DETAIL

20 POINTS

**25 POINTS** 

19 Cal. Inches

3.5 Cal. Inches

15.5 Cal. Inches

\$3,100

1/8" = 1'-0"

PLANT SCHEDULE SIZE = CALIF									
SYM.	SCIENTIFIC NAME	COMMON NAME	HGT.	SIZE	CONDITION	REMARKS			
TREES									
QL	Quercus laceyi	LACY OAK	-	2" CAL.	B and B	DECIDUOUS / SINGLE STEM			
SS	Sophora secundiflora	MOUNTAIN LAUREL	-	2" CAL.	B and B	EVERGREEN / MULTI-TRUNKED			
SHRU	BS								
LMY	Lantana montevidensis 'New Gold' 'NEW GOLD' LANTANA		-	1 GAL.		PERENNIAL / PLANT AT 30" O.C.			
MCR	Muhlenbergia capillaris 'Regal Mist'	'REGAL MIST' MUHLY	-	1 GAL.		ACCENT / PLANT AS SHOWN			
ROP	Rosmarinus officianallis 'Prostrata'	PROSTRATE ROSEMARY	-	1 GAL.		EVERGREEN / PLANT @ 24" O.C.			
RRZ	Rosa 'Radrazz'	'RADRAZZ' KNOCK-OUT ROSE	-	5 GAL.		EVERGREEN / PLANT AT 3'-0" O.C.			
SG	Salvia greggii	RED SALVIA	-	3 GAL.		EVERGREEN / PLANT AT 30" O.C.			
TS	Tecoma stans	ESPERANZA	-	5 GAL.		ACCENT / PLANT AS SHOWN			
GROL	GROUNDCOVERS AND GRASSES								
	Cynodon dactylon '419'	'419' HYBRID BERMUDAGRASS	-		SOLID SOD	SEE SPECIFICATIONS			
		DECOMPOSED GRANITE	-		APPLY TO 3" COMPACTED DEPTH OVER SUBGRADE W/ GEO-TEXTILE BARRIER AND PRE-EMERGENT HERBICIDE. FILL UNDER ADJACENT S				
		3" - 4" 'TEXAS BLEND' RIVER ROCK	-			OVER COMPACTED SUBGRADE W/ GEO-TEXTILE WEED EMERGENT HERBICIDE. FILL UNDER ADJACENT SHRUBS.			

### LANDSCAPE ORDINANCE COMPLIANCE 25 POINT MINIMUM - IDZ ZONING

# 1. PARKING LOT SHADING

TOTAL PARKING AREA = 1750 SF x 25% = 437.5 SF SHADING REQUIRED

50% CREDIT NEW TREES:

(2) LACY OAKS @ 550 SF X 50%: 137.5 SF (1) BUR OAKS @ 275 SF X 50%: TOTAL PARKING LOT SHADING PROVIDED: 687.5 SF (39.3%)

TOTAL:

TREE CANOPY ORDINANCE COMPLIANCE PROJECT SITE AREA = 11.016 SF x 15% (PER IDZ ZOINING) = 1653 SF TREE CANOPY REQUIRED

90% CREDIT NEW TREES

(3) LACY OAKS @ 550 SF x 90%: 1485 SF

(1) REDBUDS @ 275 SF x 90%: 247.5 SF **TOTAL TREE CANOPY PROVIDED:** 1732.5 SF (15.7%) TREE MITIGATION SUMMARY

MITIGATION REQUIRED PER TREE INVENTORY:

MITIGATION PROVIDED:

PARTIAL CREDIT NEW TREES (Less 1.5 Cal. Inches): 1 Cal. Inches (2) 2" LACY OAKS: (1) 2" MOUNTAIN LAUREL: 0.5 Cal. Inches

**FULL CREDIT NEW TREES:** (1) 2" LACY OAK:

**TOTAL MITIGATION PROVIDED: TOTAL MITIGATION REMAINING:** TIMES x \$200 PER CAL. INCH= PAYMENT DUE TO CITY TREE FUND:

TREE PRESERVATION ORDINANCE COMPLIANCE REFERENCE TP1.0 **BUFFER ORDINANCE COMPLIANCE** 

# PROPERTY IS ZONED IDZ, NO BUFFER REQUIRED.

**GENERAL NOTES:** 

REFER TO SPECIFICATIONS FOR ALL CONTRACT PLANTING.

- INSTALL APPROVED IMPORTED PLANTING MIX TO MIN. DEPTH OF 6" IN ALL AREAS SCHEDULED AS LANDSCAPE PLANTING AREAS.
- INSTALL APPROVED IMPORTED TOPSOIL TO 4" DEPTH IN ALL TURFGRASS AREAS. 4. CONTRACTOR IS RESPONSIBLE FOR VERIFYING ALL QUANTITIES IN THE FIELD PRIOR TO INSTALLATION AND MUST REPORT ANY DEVIATION IN
- SITE CONDITIONS TO THE LANDSCAPE ARCHITECT BEFORE PROCEEDING WITH WORK IN THE AFFECTED AREA. WHERE SHOWN ON THESE PLANS, UTILITY INFORMATION IS PROVIDED FOR REFERENCE ONLY. REF. CIVIL AND MEP PLANS FOR ALL UTILITY

2 Cal. Inches

VERIFY LOCATION AND DEPTH OF ALL EXISTING AND PROPOSED UTITILIES PRIOR TO ANY EXCAVATION. IN THE EVENT POTENTIAL CONFLICT(S) OCCUR BETWEEN UTILITIES AND LANDSCAPE IMPROVEMENTS, IMMEDIATELY CEASE WORK IN THE AFFECTED AREA, REPORT THE CONFLICT(S) TO THE OWNER'S REPRESENTATIVE, AND DO NOT PROCEED UNTIL RECEIPT OF SPECIFIC WRITTEN DIRECTION.

### **URBAN DEER NOTES:**

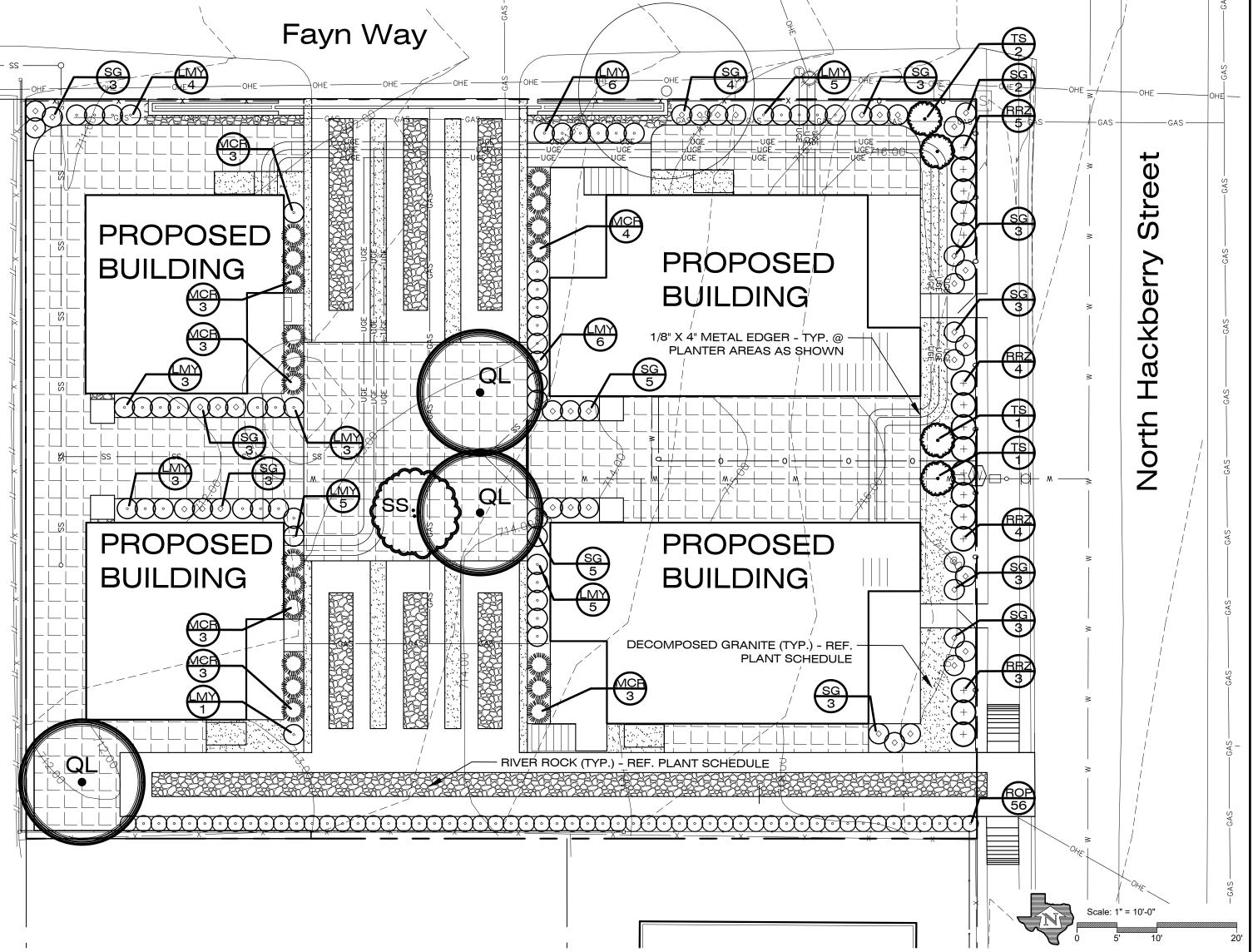
- AT THE TIME THESE DOCUMENTS WERE PREPARED THE LANDSCAPE ARCHITECT WAS NOT AWARE OF A LOCAL URBAN DEER POPULATION. 2. IN THE EVENT AN URBAN DEER POPULATION IS DISCOVERED, CONTRACTOR IS SOLELY RESPONSIBLE FOR PROTECTING ALL NEWLY-INSTALLED
- PLANTS THROUGH THE 30-DAY MAINTENANCE PERIOD. 3. APPLY "LIQUID FENCE" (OR APPROVED EQUAL) TO ALL PLANTS AS NEEDED TO DISCOURAGE BROWSING BY DEER.
- 4. ANY NEWLY-INSTALLED PLANTS EATEN OR BROWSED BY DEER PRIOR TO THE EXPIRATION OF THE 30-DAY MAINTENANCE PERIOD SHALL BE REPLACED BY THE CONTRACTOR AT NO COST TO THE OWNER.

### OVERHEAD ELECTRIC NOTES:

- 1. ALL PROPOSED LARGE SPECIES TREES (AS DEFINED BY THE UNIFIED DEVELOPMENT CODE IN EFFECT HEREOF) SHALL BE PLANTED NO
- 2. CONTRACTOR IS SOLELY RESPONSIBLE FOR FIELD LOCATING ALL OVERHEAD ELECTRIC UTILITY LINES AND ENSURING THAT NO LARGE SPECIES TREES ARE PLANTED WITHIN 20' OF ANY OVERHEAD ELECTRIC UTILITY LINES.

CLOSER THAN 20' TO ALL OVERHEAD ELECTRIC UTILITY LINES.

- WHERE CITY INSPECTORS FIND ANY PROPOSED LARGE SPECIES TREES TO BE IN VIOLATION OF PROXIMITY TO OVERHEAD ELECTRIC
- UTILITY LINES, THE CONTRACTOR SHALL RELOCATE TREES AT NO ADDITIONAL COST TO THE OWNER.



HORIZON DESIGN AND DEVELOPMENT PLANNING LANDSCAPE ARCHITECTURE DEVELOPMENT CONSULTING 14607 San Pedro Ave., Suite 200 San Antonio, Texas 78232 210.831.8564 jrobinson@horizondesign-sa.com

This document is intended for interim review purposes only and is not to be used for bidding, permitting, or construction.

The Retail Connection

10101 Reunion Place, Suite 160 San Antonio, TX 78216

909 North Hackberry Multifamily

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REVISIONS

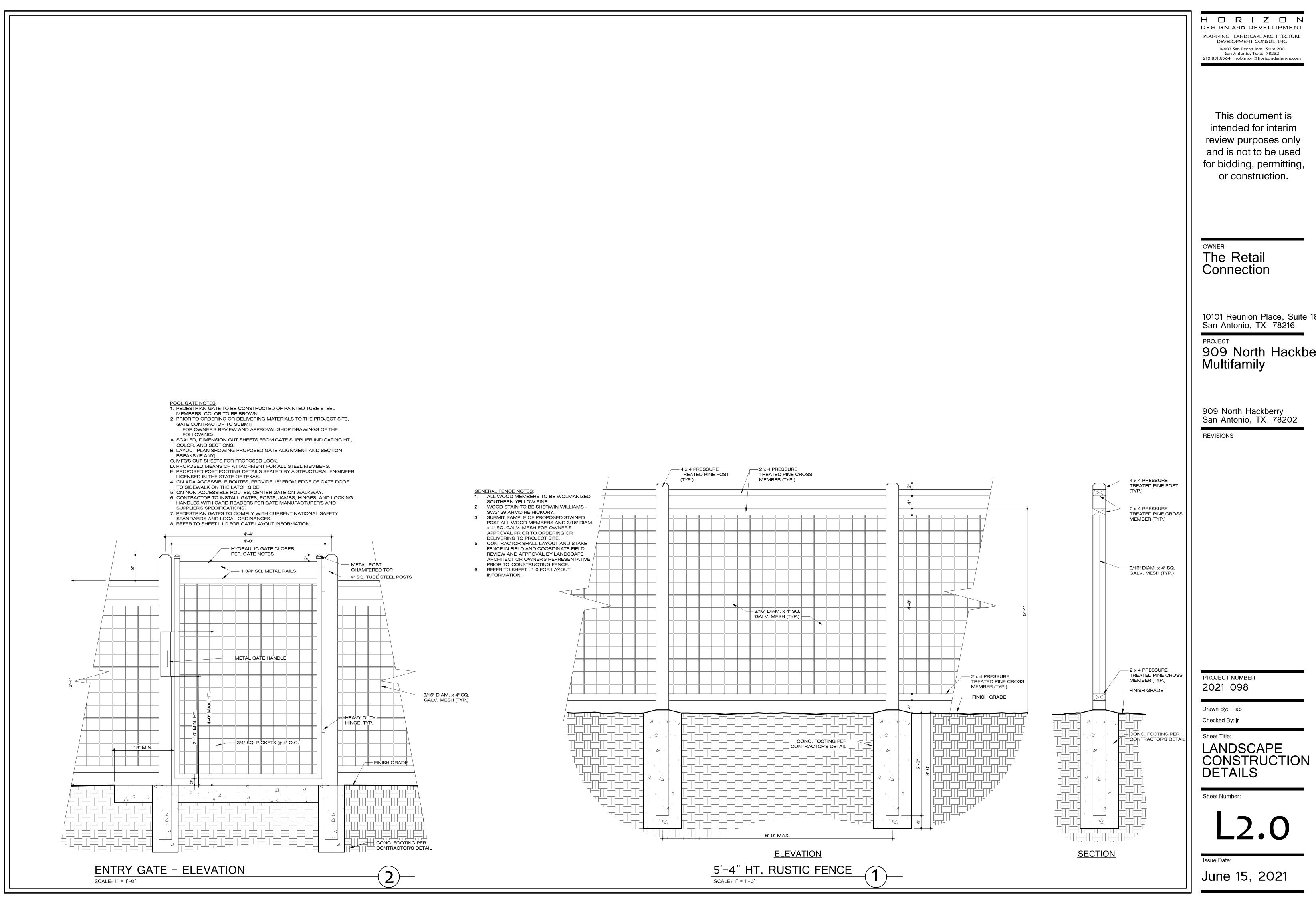
PROJECT NUMBER 2021-098

Drawn By: ab Checked By: jr

Sheet Title:

LANDSCAPE PLANTING **PLAN** 

Sheet Number:



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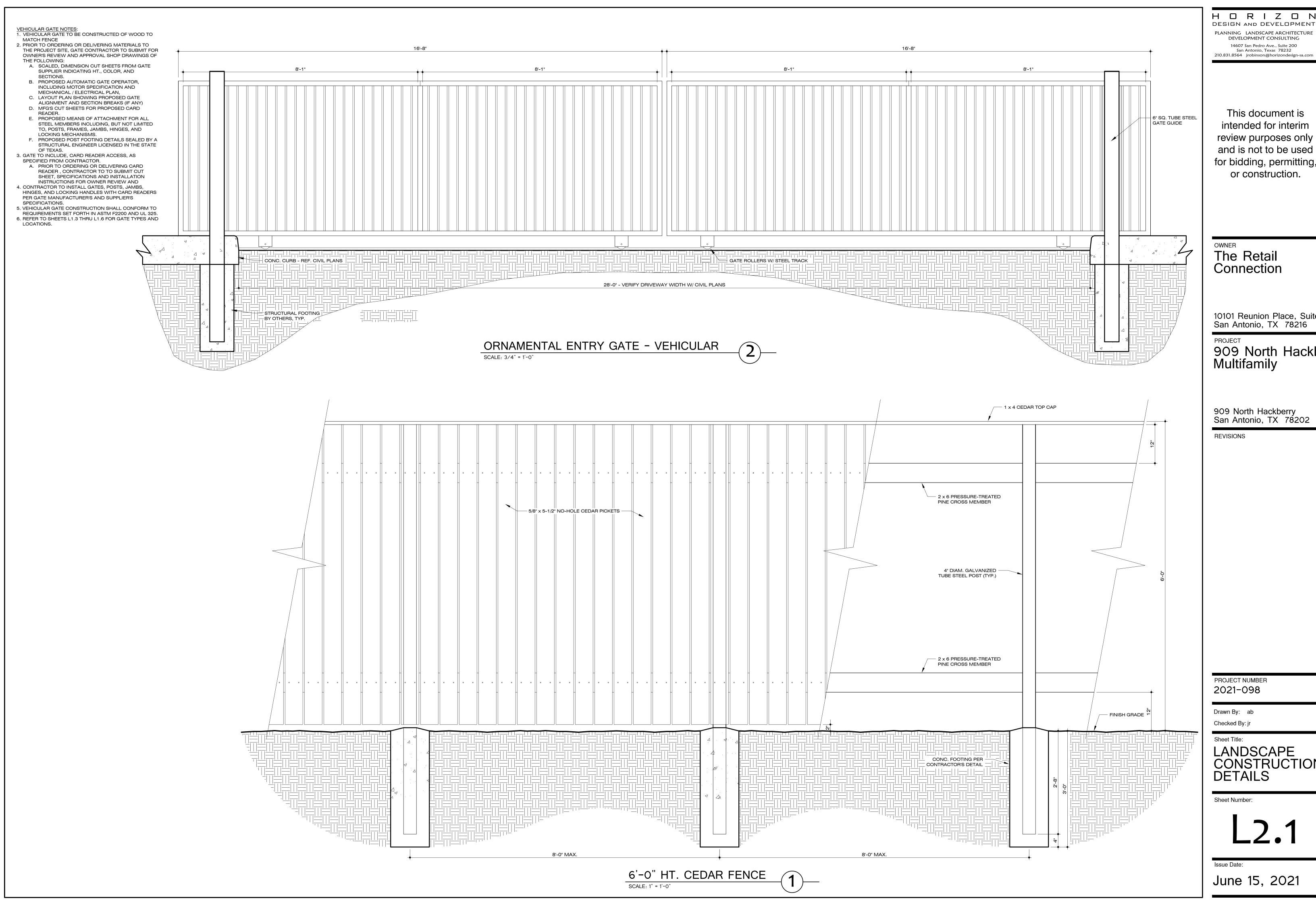
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LANDSCAPE CONSTRUCTION DETAILS

THE FOLLOWING GENERAL NOTES CONSTITUTE A MAJOR PART OF THE PLANS AND SPECIFICATIONS. STRICT COMPLIANCE WITH THESE NOTES IS ESSENTIAL TO THE PROPER CONSTRUCTION OF THIS BUILDING.

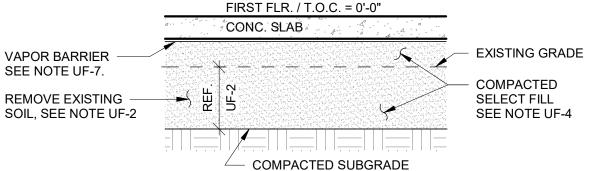
- G-1 BUILDING CODE: IRC 2018 EDITION WITH CITY OF SAN ANTONIO AMENDMENT.
- THE DETAILS DESIGNATED AS "TYPICAL DETAILS". APPLY GENERALLY TO THE DRAWINGS IN ALL AREAS WHERE CONDITIONS ARE SIMILAR TO THOSE DESCRIBED
- THE GENERAL CONTRACTOR SHALL VERIFY AND COORDINATE REQUIREMENTS OF OTHER TRADES (ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, ETC.) WITH THE STRUCTURAL DOCUMENTS PRIOR TO FABRICATION OR INSTALLATION OF ANY STRUCTURAL MEMBERS.
- G-4 THE CONTRACTOR AND FABRICATOR SHALL VERIFY ALL QUANTITIES, DIMENSIONS AND CONDITIONS THOROUGHLY WITH THE CONTRACT DOCUMENTS AND THEN NOTIFY THE ARCHITECT / ENGINEER OF ANY DISCREPANCIES OR INCONSISTENCIES BEFORE SUBMITTING SHOP DRAWINGS AND PROCEEDING WITH THE WORK. DO NOT SCALE DRAWINGS FOR DIMENSIONS.
- COMPLETED SHOP DRAWINGS SHALL BE PROVIDED, AS SPECIFIED, FOR ALL FABRICATED ITEMS AND SHALL BE REVIEWED BY THE GENERAL CONTRACTOR PRIOR TO FABRICATION. STRUCTURAL DRAWINGS SHALL NOT BE REPRODUCED FOR SHOP DRAWINGS. USE OF STRUCTURAL DRAWINGS WITHOUT PERMISSION IS GROUNDS FOR REJECTION OF SHOP DRAWINGS. THE STRUCTURAL ENGINEER WILL REVIEW SHOP DRAWINGS FOR THE LIMITED PURPOSE OF CHECKING FOR CONFORMANCE WITH INFORMATION GIVEN AND THE DESIGN CONCEPT EXPRESSED IN THE CONTRACT DOCUMENTS. THEREFORE, ALL CLOUDED DIMENSIONS, INDICATED ON ANY SHOP DRAWINGS, THAT ARE RELATIVE TO EXISTING STRUCTURES SHALL BE FIELD VERIFIED BY THE CONTRACTOR AND FABRICATOR. AS A MINIMUM, THE FOLLOWING SHOP DRAWINGS SHALL BE SUBMITTED AS WELL AS SHOP DRAWINGS LISTED IN THE DEFERRED SUBMITTAL SECTION OF THESE NOTES:
  - A. CONCRETE MIX DESIGN FOR EACH TYPE OF CONCRETE TO BE USED. B. CONCRETE REINFORCING STEEL SHOP DRAWINGS INCLUDING PLACEMENT
  - DRAWINGS AND CUT SHEETS. C. PREFABRICATED WOOD FLOOR AND ROOF TRUSSES. (GENERAL CONTRACTOR
  - SHALL SUBMIT TO THE CITY PRIOR TO CERTIFICATE OF OCCUPANCY) D. LAMINATED VENEER LUMBER (LVL) SHOP DRAWINGS AND PRODUCT DATA.
- SHOP DRAWINGS NOT PREVIOUSLY REVIEWED BY THE GENERAL CONTRACTOR SHALL BE RETURNED WITHOUT REVIEW BY THE STRUCTURAL ENGINEER. THE STRUCTURAL ENGINEER DOES NOT BEAR ANY RESPONSIBILITY TO THE STRUCTURAL MEMBERS BUILT WITHOUT APPROVED SHOP DRAWINGS.
- THE GENERAL CONTRACTOR SHALL INSPECT JOB FOR COMPLETION BEFORE SCHEDULING ANY OBSERVATION BY THE ENGINEER.
- SEE THE ARCHITECTURAL AND MECHANICAL DRAWINGS FOR LOCATIONS AND SIZES OF SLAB OPENINGS, SLEEVES, INSERTS, ANCHORS AND BOLTS REQUIRED BY VARIOUS TRADES.
- ALL PLUMBING CONDUITS AT FOUNDATION SHOULD HAVE FLEXIBLE CONNECTIONS TO SUSTAIN A MAXIMUM DIFFERENTIAL MOVEMENT OF 1 INCH.
- G-10 THE STRUCTURE HAS BEEN DESIGNED TO RESIST DESIGN LOADS ONLY AS A COMPLETED STRUCTURE. THE CONTRACTOR SHALL CONSIDER ALL CONSTRUCTION LOADS APPLIED TO THE PARTIALLY COMPLETED STRUCTURE UNTIL ALL PERMANENT CONNECTIONS ARE MADE, AND ENCLOSED PERMANENTLY AS PER CONSTRUCTION DOCUMENTS. TEMPORARY BRACING SHALL BE PROVIDED BY THE CONTRACTOR IN ALL DIRECTIONS. WHEN REQUIRED, BY THE CONSTRUCTION DOCUMENTS OR THE STRUCTURAL ENGINEER, THE CONTRACTOR SHALL PROVIDE CALCULATIONS SEALED BY A LICENSED PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF TEXAS WHICH VERIFY THE MEANS OF STRUCTURALLY MAINTAINING THE INTEGRITY OF THE COMPLETED PORTION OF
- G-11 THE CONTRACTOR SHALL BE RESPONSIBLE FOR CHECKING THE ADEQUACY OF <code>FHE</code> STRUCTURE TO SUPPORT ALL CONSTRUCTION LOADS. THE <code>SIRUCIUF</code> ENGINEER IS NOT RESPONSIBLE TO DESIGN OR CHECK THE STRUCTURE FOR CONSTRUCTION ACTIVITIES.
- G-12 ALL EXPOSED MISCELLANEOUS STEEL AND LINTEL ANGLES SHALL BE CLEANED AND GALVANIZED. APPLY ZINC COATING BY THE HOT-DIP PROCESS AND ACCORDING TO A.S.T.M. A123. WHEN APPLICABLE FIELD WELDS, BOLTED CONNECTIONS AND ABRADED AREAS SHALL BE CLEANED AND "TOUCHED UP" WITH GALVANIZING REPAIR PAINT IN ACCORDANCE WITH A.S.T.M. A780. THE GALVANIZING REPAIR PAINT SHALL HAVE A HIGH ZINC-DUST CONTENT WITH DRY FILM CONTAINING NO LESS THAN 95% ZINC-DUST BY WEIGHT, AND COMPLYING WITH THE DOD-P-21035A OR SSPC-PAINT 20.
- G-13 THE ENGINEER SHALL NOT HAVE CONTROL OF, AND SHALL NOT BE RESPONSIBLE FOR, CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, OR PROCEDURES, FOR SAFETY PRECAUTIONS AND PROGRAMS IN CONNECTION WITH THE WORK. FOR THE ACTS OR OMISSIONS OF THE CONTRACTOR. SUBCONTRACTOR, OR ANY OTHER PERSONS PERFORMING ANY OF THE WORK, OR FOR THE FAILURE OF ANY OF THEM TO CARRY OUT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.
- G-14 PERIODIC SITE OBSERVATIONS BY FIELD REPRESENTATIVES OF SPB ENGINEERING, LLC ARE SOLELY FOR THE PURPOSE OF DETERMINING IF THE WORK OF THE CONTRACTOR IS PROCEEDING IN ACCORDANCE WITH THE STRUCTURAL CONTRACT DOCUMENTS. THESE LIMITED SITE OBSERVATIONS ARE NOT INTENDED TO BE A CHECK OF THE QUALITY OR QUANTITY OF THE WORK, BUT RATHER PERIODIC IN AN EFFORT TO INFORM THE OWNER OF DEFECTS AND DEFICIENCIES IN THE WORK OF THE CONTRACTOR.
- G-15 ASSUMPTIONS HAVE BEEN MADE BY THIS OFFICE REGARDING EXISTING SITE CONDITIONS. THE ACTUAL CONDITIONS MAY VARY FROM THOSE ASSUMED. FIELD VERIFICATION OF EXISTING CONDITIONS MAY BE REQUIRED TO PROVIDE ADEQUATE SHOP DRAWINGS. THE CONTRACTOR IS TO COORDINATE EFFORTS AS REQUIRED AND IS TO REPORT ANY DISCREPANCIES REGARDING THE EXISTING CONDITIONS TO THE ENGINEER FOR POSSIBLE MODIFICATIONS NEEDED TO THE CONTRACT DRAWINGS.
- G-16 PROTECT ALL REMAINING EXISTING STRUCTURES. ANY DAMAGE TO AN EXISTING STRUCTURE SHALL BE REPAIRED TO EQUIVALENT OR BETTER CONDITION.
- G-17 PROVIDE CONTROL JOINTS AT 12'-0" ON CENTER MAXIMUM FOR ALL BRITTLE FINISHES, UNLESS NOTED OTHERWISE BY THE ARCHITECT.
- G-18 IF CONFLICT EXISTS BETWEEN DRAWINGS AND NOTES, THE STRICTEST REQUIREMENTS SHALL GOVERN.

### **BUILDING PAD PREPARATION:**

A SUBSURFACE SOIL STUDY WAS PREPARED BY ROCK ENGINEERING & TESTING LABORATORY, INC. (RETLO, THEIR PROJECT NUMBER FOR THIS SITE IS 221091 AND WAS COMPLETED ONFEBRUARY 24, 2021. THIS GEOTECHNICAL REPORT AND ITS SUPPLEMENTS WERE USED IN THE DESIGN OF THE STRUCTURAL FOUNDATIONS FOR THIS PROJECT. THE GENERAL CONTRACTOR SHALL OBTAIN A COPY OF THIS REPORT PRIOR TO THE BEGINNING OF ANY FOUNDATION WORK.

- PROVIDE TEMPORARY PROVISIONS FOR DRAINAGE OF THE BUILDING PAD AREA DURING CONSTRUCTION AND PERMANENT DRAINAGE AWAY FROM THE BUILDING AFTER CONSTRUCTION.
- UF-2 AT THE ENTIRE AREA OCCUPIED BY THE BUILDING (AND FOR A DISTANCE OF 5.0 FT. OUTSIDE OF THE BUILDING), REMOVE ALL ORGANIC AND OTHER DELETERIOUS MATERIALS. DO NOT USE FOR UNDERFLOOR FILL. REMOVE ADDITIONAL SOILS PER GEOTECHNICAL REPORT TO ACHIEVE THE PVR MENTIONED IN UF-10. THE EXPOSED SUBGRADE SHALL BE RELATIVELY LEVEL.
- UF-3 THE EXPOSED SUBGRADE SHALL BE SCARIFIED AND COMPACTED PER THE GEOTECHNICAL REPORT MENTIONED ABOVE.
- BRING THE BUILDING PAD TO UNDERSIDE OF SLAB WITH SELECT STRUCTURAL FILL MATERIAL AS SPECIFIED PER THE GEOTECHNICAL REPORT TO ACHIEVE A MAX. PVR
- PERFORM ALL EARTHWORK DESCRIBED ABOVE BEFORE TRENCHING FOR GRADE BEAMS, MEP ITEMS, OR UTILITY LINES.
- EXCAVATE BEAM TRENCHES TO MEET PLANNED DIMENSIONS. PRIOR TO PLACEMENT OF CONCRETE, HAND COMPACT BOTTOM OF BEAM TRENCHES PER THE GEOTECHNICAL REPORT STANDING WATER SHOULD NOT BE PERMITTED IN THE BEAM TRENCHES AFTER FINAL COMPACTION AND BEFORE PLACEMENT OF CONCRETE. REMOVE ALL LOOSE MATERIALS AND UNSUITABLE SOILS DUE TO RAINFALL OR BY DESICCATION.
- PLACE 15 MIL "STEGO WRAP" OR REVIEWED EQUIVALENT ON TOP OF SELECT FILL. SMOOTH SUBGRADE TO PREVENT PROTRUSIONS THAT MAY CAUSE DAMAGE OR RUPTURE FILM. LAY FILM ON SUBGRADE INCLUDING BEAM AND FOOTING SOFFITS AND SIDES OF BEAMS AND FOOTINGS USING WIDEST PRACTICAL WIDTHS. LAP EDGES OF FILM 6" WITH TOP LAP PLACED IN DIRECTION OF CONCRETE FLOW AND TAPE ALL JOINTS. CUT FILM AROUND PIPES AND ROUGH-INS AND SEAL CUTS WITH PRESSURE SENSITIVE TAPE.
- UF-8 AT AREAS OUTSIDE THE BUILDING LINE, SLOPE THE TOP SURFACE OF FILL A MIN. 5% FOR A DISTANCE OF 10 FEET TO MATCH FINISH GRADE SLOPE AND HOLD DOWN A MINIMUM OF 10 INCHES BELOW FINISH FLOOR LINE, GUTTER DOWNSPOUTS EXTEND AT LEAST THREE (3) FEET PAST THE EDGE OF BUILDING, UNLESS NOTED OTHERWISE ON THE CIVIL ENGINEERS CONSTRUCTION DOCUMENTS.
- THE OWNER SHALL EMPLOY AN INDEPENDENT TESTING LABORATORY TO DETERMINE IF THE SOIL CONDITIONS AT THE ACTUAL ADDITION LOCATION ARE CONSISTENT WITH THE BORINGS IN THE SUBSURFACE SOIL STUDY AND TO TAKE DENSITY TESTS FOR SUBGRADE AND EACH LIFT OF SELECT FILL.
- UF-10 THE FOLLOWING DESIGN PARAMETERS WHERE USED TO DESIGN THE FOUNDATION o ALLOWABLE SOIL BEARING PRESSURE: 2,000 PSF o MAXIMUM PVR FOR SLAB ON GROUND IS TO BE 2"
- UF-11 IF UTILITY TRENCHES ARE REQUIRED, WE RECOMMEND THAT MEASURES BE TAKEN TO PROHIBIT TRANSMITTING WATER UNDER THE BUILDING PAD. REFER TO THE GEOTECHNICAL ENGINEERING STUDY OR CONTACT GEOTECHNICAL ENGINEER FOR BACKFILL REQUIREMENTS.

### UF-12 BUILDING PAD DETAIL:



### SCHEDULE OF SITE OBSERVATIONS BY THE STRUCTURAL ENGINEER:

SEE NOTE UF-3.

- ALL STRUCTURAL ELEMENTS OF THE BUILDING SHALL BE OBSERVED BY THE STRUCTURAL ENGINEER'S REPRESENTATIVE DURING THE CONSTRUCTION PHASE, SO THAT A FINAL LETTER OF COMPLIANCE CAN BE PROVIDED TO THE OWNER AND / OR BUILDING AUTHORITY.
- SO-2 PRIOR TO THE BEGINNING OF CONSTRUCTION, THE GENERAL CONTRACTOR SHALL ARRANGE A MEETING WITH THE STRUCTURAL ENGINEER TO SET UP A SCHEDULE FOR THE FOLLOWING OBSERVATIONS:
  - A. CONCRETE: FOR EACH CONCRETE POUR UNLESS NOTED OTHERWISE BY THE ENGINEER. SEE CONCRETE AND CONCRETE REINFORCEMENT C-5.
  - B. TIMBER FRAMING: AFTER ALL WOOD FRAMING AND CONNECTIONS ARE MADE BUT BEFORE APPLYING SHEATHING.
  - C. NOTIFY THE ARCHITECT AT LEAST 24 HOURS BEFORE EACH SITE OBSERVATION IS REQUIRED TO ALLOW TIME FOR ARRANGEMENTS TO BE MADE WITH ENGINEER FOR SITE OBSERVATION.
- THESE STRUCTURAL OBSERVATIONS ARE THE REQUIREMENTS OF THE STRUCTURAL ENGINEER AND DOES NOT INCLUDE OR WAIVE THE RESPONSIBILITY FOR OTHER INSPECTIONS REQUIRED IN ACCORDANCE WITH THE IRC OR THE CITY OF SAN ANTONIO.

## DEFERRED DESIGN SUBMITTAL

DD-1 SUBMITTALS LISTED IN DD-2 ARE TO BE DESIGNED, DETAILED, SIGNED AND SEALED BY A LICENSED PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF TEXAS. SEE PLANS AND SPECIFICATIONS FOR DESIGN REQUIREMENTS OF THESE ELEMENTS.

DD-2

RESPONSIBLE FOR SHOP <u>DRAWING REVIEW</u>

RESPONSIBLE FOR

o WOOD FLOOR & STRUCTURAL ENGINEER AND BUILDING INSPECTOR ROOF TRUSSES BUILDING INSPECTOR (COSA)

### **CONCRETE AND CONCRETE REINFORCEMENT:**

- STRUCTURAL CONCRETE SHALL BE IN ACCORDANCE WITH THE CODE APPLICABLE EDITION OF "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE (ACI 318)", THE AMERICAN CONCRETE INSTITUTE.
- ALL CONCRETE REINFORCEMENT SHALL BE NEW DOMESTIC DEFORMED BILLET STEEL, CONFORMING TO ASTM A 615, GRADE 60, EXCEPT WELDABLE REBARS ASTM A706, GR. 60, WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185, GRADE 70°
- DETAIL REINFORCING BARS AND PROVIDE BAR SUPPORTS AND SPACERS IN ACCORDANCE WITH ACI 315.
- ALL REINFORCING SHALL BE PROPERLY CHAIRED AND TIED PER ACI 315 (SP66) AND CRSI (PLACING REINFORCING BARS) PRIOR TO PLACING CONCRETE.
- PLACEMENT OF ALL REINFORCING STEEL SHALL BE OBSERVED BY THE ENGINEER PRIOR TO CONCRETE PLACEMENT UNLESS APPROVED OTHERWISE.
- ALL CONCRETE SHALL BE NORMAL WEIGHT STONE AGGREGATE CONCRETE UNLESS NOTED OTHERWISE. AGGREGATE SHALL MEET ASTM C33 REQUIREMENTS. AND SHALL BE 3/4" TO 1 1/2" NOMINAL AGGREGATE SIZE. CONCRETE ON METAL DECK IS TO UTILIZE 3/4" MAXIMUM AGGREGATE. PROVIDE ADMIXTURES AS REQUIRED TO IMPROVE WORKABILITY. THE GENERAL CONTRACTOR SHALL COORDINATE THE CONCRETE SLUMP WITH PLACEMENT REQUIREMENTS UNLESS NOTED OTHERWISE IN STRUCTURAL DOCUMENTS. PLASTIC CONCRETE TEMPERATURE SHALL NOT EXCEED 90 DEGREES PRIOR TO PLACEMENT. ALL CONCRETE SHALL BE CURED FOR A MINIMUM OF 7 DAYS USING MOIST CURING PROCEDURES OR CURING COMPOUNDS WHICH WILL NOT INTERFERE WITH THE BONDING OF FINISH TILE FLOORS. NO FLY ASH SHALL BE USED AT ARCHITECTURALLY EXPOSED CONCRETE WITHOUT PRIOR APPROVAL FROM ARCHITECT. THE FLYASH CONTENT SHALL NOT EXCEED THE PERCENTAGE OF CEMENTITIOUS MATERIALS SHOWN BELOW. IN ADDITION TO ABOVE, THE CONCRETE SHALL MEET THE FOLLOWING REQUIREMENTS:

f`c MAX W/C FLYASH CONTENT DESCRIPTION OF USE SLAB-ON-GRADE 3,000 PSI N/A **FOOTINGS** 3,000 PSI

PROVIDE A SET OF CYLINDERS IN ACCORDANCE WITH ASTM C 31 TO BE TAKEN BY AN INDEPENDENT TESTING LAB AT THE FREQUENCY SPECIFIED IN ACI 318 AND THE GOVERNING BUILDING CODE WITH LOCAL AMENDMENTS. COMPRESSION TEST RESULTS SHALL BE REPORTED TO THE ENGINEER WITHIN 24 HOURS.

25% MAX

25% MAX

N/A

- NO SUBSEQUENT CONSTRUCTION WILL BE ALLOWED UNTIL CONCRETE HAS REACHED 75% OF DESIGN STRENGTH.
- PORTLAND CEMENT SHALL CONFORM TO ASTM C150, TYPE I/II.
- NO WELDING OF REINFORCING BARS OR TORCHING TO BEND REINFORCING BARS SHALL BE ALLOWED WITHOUT THE SPECIFIC APPROVAL OF THE STRUCTURAL **ENGINEER**
- C-11 CONCRETE COVER SHOULD BE AS FOLLOWS:
  - A. FOOTINGS AND OTHER PRINCIPAL STRUCTURAL MEMBERS IN WHICH CONCRETE IS CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH - 3 INCHES.

B. WHERE CONCRETE SURFACES, AFTER REMOVAL OF FORMS, ARE EXPOSED TO WEATHER OR EARTH:

o BARS 3/4" AND LARGER IN DIAMETER. . 2 INCHES

o BARS SMALLER THAN 5/8" IN DIAMETER . . 1 1/2 INCHES

C. WHERE SURFACES ARE NOT DIRECTLY EXPOSED TO WEATHER OR EARTH o SLAB ON GRADE (FROM TOP OF SLAB) ... . 1 1/2 INCHES

o SLABS, WALLS, JOISTS No. 14 AND No. 18 BARS . 1 1/2 INCHES No. 11 BARS AND SMALLER .. . 3/4 INCHES o BEAMS, COLUMNS PRIMARY REINF., TIES, STIRRUPS, SPIRALS ..................... 1 1/2 INCHES

- MECHANICAL AND ELECTRICAL CONDUIT CAN NOT BE PLACED IN BEAMS PARALLEI TO BEAM REINFORCING. PROVIDE A MINIMUM OF 1 1/2" CLEAR BETWEEN CONDUIT AND PARALLEL REINFORCING. DO NOT "BUNDLE" CONDUITS. CONDUITS SHALL BE PLACED IN THE MIDDLE ONE THIRD OF THE SLAB THICKNESS OR BEAM DEPTH.
- C-13 SET AND BUILD INTO FORM WORK ANCHORAGE DEVICES AND OTHER EMBEDDED ITEMS REQUIRED FOR OTHER WORK THAT IS ATTACHED TO OR SUPPORTED BY CAST-IN-PLACE CONCRETE. REBAR PROJECTING FROM CONCRETE SHALL BE SECURED IN PLACE PRIOR TO PLACING CONCRETE.
- IF NOT SHOWN ON PLAN, THE CONTRACTOR SHALL SUBMIT A PROPOSED CONSTRUCTION JOINT LAYOUT FOR REVIEW. MAXIMUM SQUARE FOOTAGE SHALL NOT EXCEED 15,000 UNLESS APPROVED BY ENGINEER.

### LAMINATED VENEER LUMBER

- LAMINATED VENEER LUMBER (LVL) SHALL BE PERFORMANCE RATED LVL. LVL'S SHALL BE FURNISHED AND INSTALLED AS SHOWN ON THE PLANS AND IN ACCORDANCE WITH THE SPECIFICATIONS OF THE LVL MANUFACTURER.
- LV-2 THE CONTRACTOR SHALL USE APPROVED HARDWARE AND CONNECTIONS AS SPECIFIED ON THE PLANS AND IN ACCORDANCE WITH THE SPECIFICATIONS OF THE LVL MANUFACTURER.
- LV-3 PRODUCT QUALITY SHALL CONFORM TO THE MANUFACTURER'S APPROVED QUALITY CONTROL MANUAL, WITH CERTIFICATION SERVICES PROVIDED BY APA EWS IN ACCORDANCE WITH BUILDING CODE REQUIREMENTS AND THE APPLICABLE CODE EVALUATION REPORT.
- LVL'S SHALL BE MARKED WITH THE APA EWS TRADEMARK, INDICATING CONFORMANCE WITH THE MANUFACTURER'S EVALUATION REPORT.
- LVL'S SHALL BE PROTECTED FROM DIRECT EXPOSURE TO WEATHER PRIOR TO INSTALLATION.
- STRUCTURAL PROPERTIES OF THE LVL'S SHALL BE EVALUATED USING METHODS SPECIFIED IN ASTM D5456 FOR STRUCTURAL COMPOSITE LUMBER.
- DESIGN PROPERTIES FOR APA EWS PERFORMANCE RATED LVL'S SHALL MEET OR **EXCEED THE FOLLOWING STRESS VALUES:**

Fb . . . . . . . 2,400 PSI Fv . . . . . . . . . . . 285 PS E . . . . . . 1,800,000 PSI

### **DESIGN LOADS:**

- DL-1 DEAD LOADS INCLUDE THE WEIGHT OF CONSTRUCTION MATERIALS INCORPORATED INTO THE BUILDING, INCLUDING BUT NOT LIMITED TO WALLS, FLOORS. ROOFS. CEILINGS. STAIRWAYS, BUILT-IN PARTITIONS, FINISHES, CLADDING AND OTHER SIMILARLY ARCHITECTURAL AND STRUCTURAL ITEMS AND FIXED SERVICE EQUIPMENT. ALL DEAD LOADS ARE CONSIDERED PERMANENT LOADS. MINIMUM ROOF DEAD LOAD IS 20 PSF OR ACTUAL LOAD WHICHEVER IS LARGER.
- DL-2 DEAD LOADS FOR MECHANICAL UNITS ARE BASED ON THE WEIGHTS OF EQUIPMENT AS INDICATED ON THE STRUCTURAL DRAWINGS (INCLUDING THE WEIGHT OF CONCRETE PADS, WHERE INDICATED). ANY CHANGES IN TYPE, SIZE, WEIGHT, LOCATION OR NUMBER OF PIECES OF EQUIPMENT SHOULD BE REPORTED TO THE ARCHITECT FOR VERIFICATION OF THE ADEQUACY OF SUPPORTING MEMBERS PRIOR TO THE PLACEMENT OF SUCH EQUIPMENT.

40 PSF

- DL-3 UNIFORM DESIGN LIVE LOADING IS AS FOLLOWS: o ROOFS (STEEPER THAN 4" / FT). 16 PSF o UNINHABITABLE ATTICS WITHOUT STORAGE . 10 PSF UNINHABITABLE ATTICS WITH LIMITED STORAGE. ... 20 PSF o BATHROOMS o PRIVATE ROOMS & CORRIDORS SERVING THEM .. ... 40 PSF o PUBLIC ROOMS & CORRIDORS SERVING THEM .... 100 PSF o ALL OTHER AREAS. **40 PSF**
- DL-4 ROOF LIVE LOADS MAY BE REDUCED.

o STAIRS

- DL-5 SNOW LOAD: o GROUND SNOW LOAD, Pg. 5 PSF
- DL-6 WIND LOADS: RISK CATEGORY o ULTIMATE DESIGN WIND SPEED. Vult .. .. 115 MPH o ALLOWABLE DESIGN WIND SPEED, Vasd ...... .... 89 MPH EXPOSURE CATEGORY
- o INTERNAL PRESSURE COEFFICIENT ....... +/- 0.18 FOR COMPONENTS AND CLADDING GROSS WIND PRESSURE, USE APPLICABLE EXTERIOR PRESSURE COEFFICIENTS.
- EARTHQUAKE DESIGN DATA: o SEISMIC IMPORTANCE FACTOR le . RISK CATEGORY o MAPPED SPECTRAL RESPONSE ACCELERATIONS: . 0.051g . 0.023g o SITE CLASS "D"
- o SPECTRAL RESPONSE COEFFICIENTS . 0.054g Sd1 ... 0.036a
- SEISMIC DESIGN CATEGORY "A" o BASIC SEISMIC FORCE RESISTING SYSTEM - STRUCTURAL CONCRETE AND WOOD FRAMING SYSTEM NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE
  - o DESIGN BASE SHEAR, V = N/A o SEISMIC RESPONSE COEFFICIENT, Cs = N/A o RESPONSE MODIFICATION COEFFICIENT, R = N/A o ANALYSIS PROCEDURE - N/A
- DL-8 UNLESS SPECIFICALLY NOTED, THERE ARE NO PROVISIONS FOR FUTURE FLOORS, ROOFS OR OTHER LOADS.

### **ABBREVIATIONS**

E.F.

E.J.

ELEV

**EQUIP** 

E.W.

EXIST

EXP.

EXT.

FDN.

F.D.

F.S.

FIN.

FLR.

FT.

HORIZ.

HCA

- EACH FACE

- ELEVATION

EQUIPMENT

EACH WAY

**EXPANSION** 

EXTERIOR

- FAR SIDE

- FINISH

FIELD

- FLOOR

FOOTING

FOUNDATION

FLOOR DRAIN

- FOOT OR FEET

- GALVANIZED

HORIZONTAL

HEADED CONCRETE ANCHOR

EXISTING

· EQUAL

EXPANSION JOINT

<u>\$</u>	- AND	I.D.	
@ C	- AT		- INCH
3	- CENTERLINE	INT.	- INTERIOR
	- DEGREE	JST.	- JOIST
Ø	- DIAMETER	JT.	- JOINT
A.B.	- ANCHOR BOLT	L	- ANGLE
APPROX.	- APPROXIMATE	LBS.	- POUND
ARCH.	- ARCHITECT/ARCHITECTURAL	LF	- LINEAR FOOT
ALT.	- ARCHITECT/ARCHITECTURAL - ALTERNATE	LLH	- LONG LEG HORIZONTAL
3.L.	- BUILDING LINE	LLV	- LONG LEG VERTICAL
			- MAXIMUM
3.W.	- BOTH WAYS	MECH.	- MECHANICAL
3.O.B.	- BOTTOM OF BEAM		- MANUFACTURER
BOT.	- BEAM - BOTH WAYS - BOTTOM OF BEAM - BOTTOM	MID	- MIDDLE
BLDG	- BUILDING	MIN.	- MINIMUM
	- BASEMENT	MISC	- MISCELLANEOUS
			- MASONRY
RTWN	- BETWEEN		
CANT	- BETWEEN - CANTILEVER - CAST-IN-PLACE	NTS	- NEAR SIDE - NOT TO SCALE
CIP	- CAST-IN-PLACE	0.0	- ON CENTER
	- CEILING		- OUTSIDE DIAMETER
CLR.		-	- OPPOSITE HAND
CMU			- OPENING
COL.	- COLUMN	OPP.	
	- CONCRETE		PREFABRICATED
CONTR.		PSF	_
D.J.	- CONSTRUCTION JOINT	PSI	
	- CONNECTION	PL.	
CONST	- CONSTRUCTION	RAD	- RADIUS
ONT.	- CONSTRUCTION - CONTINUOUS	REE	- RADIUS - REFERENCE
D.	- DEEP	REINE	- REINFORCING/REINFORCED
			- REQUIRED
DIA.	- DIAMETER		- SPACES/SPACING
DIAG.	- DIAGONAL		- SCHEDULE
DIAG. DIM.	- DIMENSION		- SCHEDOLE - SECTION
	- DEAD LOAD		- SECTION - SHEET/SHEATHING
	- DOUBLE		- SHEET/SHEATHING - SIMILAR
DBL. DN.	- DOUBLE	SPEC.	
	- DOWEL	OL.	- SLOPE - STANDARD STRUCTURAL STEEL
-	- DRAWING	333 CTIFF	- STANDARD STRUCTURAL STEEL - STIFFENERS
ĒA.	- EACH	STIFF.	- STIFFENERS

STIR.

SQ.

STD.

STR.

SYM.

T.O.C.

T.O.J.

T.O.S.

T.O.W.

TYP.

UNO.

VERT.

W/

WWW.LIQUE.US LIQUE DESIGN STUDIO, LLC

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DATE DESCRIPTION

SCHEDULE OF REVISIONS

### INTERMEDIATE REVIEW

PROJECT NUMBER: 210117 PROJECT DATE: 2021.06.01 PROJECT MANAGER: SPB PROJECT TEAM: SPB, DLB

STRUCTURAL NOTES

**S-101** 

- STIRRUPS

- STANDARD

STRUCT. - STRUCTURE/STRUCTURAL

- TOP AND BOTTOM

- THICK / THICKNESS

- UNLESS NOTED OTHERWISE

- TOP OF CONCRETE

- TOP OF JOIST

- TOP OF STEEL

TOP OF WALL

- TYPICAL

- WITH

VERTICAL

WORK POINT

W.W.F. - WELDED WIRE FABRIC

- SYMMETRICAL

- SQUARE

- STEEL

- TREAD

- STAIR

### POST-INSTALLED CONCRETE / MASONRY ANCHORS:

- PI-1 POST-INSTALLED ANCHORS SHALL ONLY BE USED WHERE SPECIFIED ON THE CONSTRUCTION DOCUMENTS. IF ADHESIVE / MECHANICAL ANCHOR IS GENERICALLY CALLED OUT ON THE CONSTRUCTION DOCUMENTS. ANY ANCHOR MENTIONED BELOW IS ACCEPTABLE. IF SPECIFIC ANCHOR IS CALLED FOR, SUBSTITUTION MUST BE APPROVED BY THE STRUCTURAL ENGINEER OF RECORD
- ANCHOR CAPACITY USED IN DESIGN SHALL BE BASED ON THE TECHNICAL DATA PUBLISHED BY THE ANCHOR MANUFACTURER OR SUCH OTHER METHOD AS APPROVED BY THE STRUCTURAL ENGINEER OF RECORD. ANCHOR CAPACITY IS DEPENDANT UPON SPACING BETWEEN ADJACENT ANCHORS AND PROXIMITY OF ANCHORS TO EDGE OF CONCRETE. INSTALL ANCHORS IN ACCORDANCE WITH SPACING AND EDGE CLEARANCES INDICATED ON THE DRAWINGS.
- CARE SHALL BE TAKEN IN PLACING POST-INSTALLED ANCHORS TO AVOID CONFLICTS WITH EXISTING REBAR. HOLES SHALL BE DRILLED AND CLEANED IN ACCORDANCE WITH THE MANUFACTURER'S WRITTEN INSTRUCTIONS. INSTALL ANCHORS PER THE MANUFACTURER INSTRUCTIONS, AS INCLUDED IN THE ANCHOR PACKAGING. OVERHEAD ADHESIVE ANCHORS MUST BE INSTALLED IN ACCORDANCE WITH THE ANCHOR MANUFACTURER'S RECOMMENDATIONS EXISTING REINFORCING BARS IN THE CONCRETE STRUCTURE MAY CONFLICT WITH SPECIFIC ANCHOR LOCATIONS. UNLESS NOTED ON THE DRAWINGS THAT THE BARS CAN BE CUT, THE CONTRACTOR SHALL REVIEW THE EXISTING STRUCTURAL DRAWINGS AND SHALL UNDERTAKE TO LOCATE THE POSITION OF THE REINFORCING BARS AT THE LOCATIONS OF THE CONCRETE ANCHORS, BY FERROSCAN, GPR, X-RAY OR OTHER MEANS ACCEPTABLE TO THE STRUCTURAL ENGINEER-OF-RECORD.
- SUBSTITUTION REQUESTS FOR PRODUCTS OTHER THAN THOSE SPECIFIED BELOW SHALL BE SUBMITTED BY THE CONTRACTOR TO THE STRUCTURAL ENGINEER-OF-RECORD ALONG WITH CALCULATIONS THAT ARE PREPARED AND SEALED BY A REGISTERED PROFESSIONAL ENGINEER. THE CALCULATIONS SHALL DEMONSTRATE THAT THE SUBSTITUTED PRODUCT IS CAPABLE OF ACHIEVING THE PERTINENT EQUIVALENT PERFORMANCE VALUES (MINIMUM) OF THE SPECIFIED PRODUCT USING THE APPROPRIATE DESIGN PROCEDURE AND / OR STANDARD(S) AS REQUIRED BY THE BUILDING CODE. SUBSTITUTION REQUESTS FOR ALTERNATE PRODUCTS MUST BE APPROVED IN WRITING BY THE STRUCTURAL ENGINEER OF RECORD PRIOR TO USE. SUBSTITUTIONS WILL ALSO BE EVALUATED BY THEIR HAVING AN ICC ESR SHOWING COMPLIANCE WITH THE RELEVANT BUILDING CODE FOR SEISMIC USES, LOAD RESISTANCE, INSTALLATION CATEGORY, AND AVAILABILITY OF COMPREHENSIVE INSTALLATION INSTRUCTIONS. ADHESIVE ANCHOR EVALUATION WILL ALSO CONSIDER CREEP, IN-SERVICE TEMPERATURE AND INSTALLATION TEMPERATURE.
- THE CONTRACTOR SHALL ARRANGE AN ANCHOR MANUFACTURER'S REPRESENTATIVE TO PROVIDE ONSITE INSTALLATION TRAINING FOR ALL OF THEIR ANCHORING PRODUCTS SPECIFIED. THE STRUCTURAL ENGINEER OF RECORD MUST RECEIVE DOCUMENTED CONFIRMATION THAT ALL OF THE CONTRACTOR'S PERSONNEL WHO INSTALL ANCHORS ARE TRAINED PRIOR TO THE COMMENCEMENT OF INSTALLING ANCHORS.
- PI-6 THE CONTRACTOR SHALL OBTAIN APPROVAL FROM THE STRUCTURAL ENGINEER-OF-RECORD PRIOR TO INSTALLING POST-INSTALLED ANCHORS IN PLACE OF MISSING OR MISPLACED CAST-IN-PLACE ANCHORS.
- MECHANICAL ANCHORS FOR CONCRETE SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ACI 355.2 AND ICC-ES AC193 FOR CRACKED CONCRETE RECOGNITION.
- PRE-APPROVED MECHANICAL ANCHORS FOR CONCRETE INCLUDE: A) SIMPSON STRONG-TIE "TITEN-HD" AND "TITEN-HD ROD HANGER" B) SIMPSON STRONG-TIE "STRONG-BOLT" AND "STRONG-BOLT 2" C) SIMPSON STRONG-TIE "TORQ-CUT" D) HILTI "KWIK HUS-EZ" OR "KWIK HUS-EZ-1" SCREW ANCHORS E) HILTI "KWIK BOLT TZ" EXPANSION ANCHOR F) HILTI "HDA" UNDERCUT ANCHOR
  - G) HILTI "HSL-3" EXPANSION ANCHOR H) DEWALT "POWER BOLT" I) DEWALT "POWER-STUD +SD1" J) DEWALT "SCREW BOLT"

B) HILTI "HIT-RE 500-SD" ADHESIVE

- ADHESIVE ANCHORS FOR CONCRETE SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ACI 355.4 AND ICC-ES AC308 FOR CRACKED CONCRETE RECOGNITION.
- PI-10 PRE-APPROVED ADHESIVE ANCHORS FOR CONCRETE INCLUDE: A) SIMPSON STRONG-TIE "SET-XP" AND AT-XP"
  - C) HILTI "HIT-HY 200" SAFE SET SYSTEM WITH HILTI "HIT-Z" ROD. a. NO CLEANING IS REQUIRED FOR HIT-Z ANCHORS FOR TEMPERATURES ABOVE 41°F.
    - b. FOR TEMPERATURE BELOW 41°F FOR HIT-Z ANCHOR INSTALLATIONS, USE HILTI TE-CD OR TE-YD HOLLOW DRILL BITS WITH VC 20/40 VACUUM SYSTEM. c. FOR ALL TEMPERATURES FOR REBAR INSTALLATIONS, USE HILTI TE-CD OR TE-YD HOLLOW DRILL BITS WITH VC 20/40 VACUUM
  - D) HILTI "HIT-HY 200" SAFE SET SYSTEM WITH HILTI HOLLOW DRILL BIT
  - E) DEWALT PURE 110+ ADHESIVE
  - F) DEWALT AC200+ ADHESIVE G) DEWALT PURE 110+ DUST X SYSTEM
- PI-11 MECHANICAL ANCHORS FOR SOLID-GROUTED CONCRETE MASONRY SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ICC-ES AC01 OR
- PI-12 PRE-APPROVED MECHANICAL ANCHORS FOR SOLID-GROUTED CONCRETE
  - MASONRY INCLUDE: A) SIMPSON STRONG-TIE "STRONG-BOLT 2", "WEDGE-ALL" AND TITEN HD" B) HILTI "KWIK HUS EZ" SCREW ANCHOR C) HILTI "KWIK BOLT 3" EXPANSION ANCHOR D) DEWALT "POWER-STUD +SD1"
- E) DEWALT "SCREW BOLT" PI-13 ADHESIVE ANCHORS FOR SOLID-GROUTED CONCRETE MASONRY SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ICC-ES AC58.
- PI-14 PRE-APPROVED ADHESIVE ANCHORS FOR SOLID-GROUTED CONCRETE MASONRY INCLUDE:
  - A) SIMPSON STRONG-TIE "SET" AND "SET-XP" B) HILTI "HIT-HY 210" MASONRY ADHESIVE ANCHORING SYSTEM (UNGROUTED CMU CELLS ONLY) C) DEWALT AC100+ ADHESIVE ANCHORING SYSTEM
- PI-15 ADHESIVE ANCHORS FOR HOLLOW CONCRETE MASONRY/UNREINFORCED CLAY BRICK MASONRY WITH SCREEN TUBES SHALL BE TESTED AND QUALIFIED IN ACCORDANCE WITH ICC-ES AC58 OR AC60, AS APPROPRIATE. THE APPROPRIATE SCREEN TUBE SHALL BE USED AS RECOMMENDED BY THE ADHESIVE MANUFACTURER.

### TIMBER FRAMING:

- T-1 TIMBER FRAMING AND PLYWOOD SHEATHING SHALL CONFORM TO THE CODE APPLICABLE EDITIONS OF THE AMERICAN PLYWOOD ASSOCIATION (APA) PLYWOOD DESIGN SPECIFICATION, AND THE NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION WITH THE N.F.P.A. SUPPLEMENT.
- STUDS SHALL BE DOUBLED AT ALL ANGLES, INTERSECTING WALLS, CORNERS, BEAM SUPPORTS AND AROUND ALL OPENINGS. REFER DETAIL 1/S-501.
- BEAMS MADE UP OF SEVERAL 2X PIECES SHALL HAVE NO SPLICES EXCEPT OVER T-3 SUPPORTS. NAIL BOARDS TOGETHER PER DETAIL 2 / S-501.
- ALL SILL PLATES SHALL BE #2 TREATED SOUTHERN PINE AND SHALL BE ATTACHED T-4 TO THE FOUNDATION WITH 1/2" DIAMETER ANCHOR BOLTS WITH A MINIMUM OF 7" EMBEDMENT INTO THE CONCRETE. SPACE ANCHOR BOLTS AT 4'-0"o.c MAXIMUM. THERE SHALL BE A MINIMUM OF 2 BOLTS PER SILL PIECE WITH ONE BOLT LOCATED WITHIN 12" OF EACH END OF EACH SILL PIECE UNLESS NOTED OTHERWISE ON THE FRAMING PLANS OR DETAILS.
- T-5 PLACE SINGLE PLATE AT BOTTOM OF ALL STUD WALLS.

DOUGLAS FIR-LARCH WITH 15% MAX. MOISTURE.

- T-6 NAILING AND THE ATTACHMENT OF ALL FRAMING MEMBERS SHALL BE AS SPECIFIED ON THE FASTENING SCHEDULE. COMMON WIRE NAILS OR SPIKES OR GALVANIZED BOX NAILS SHALL BE USED FOR ALL FRAMING UNLESS NOTED OR SCHEDULED
- ALL EXTERIOR WALLS AT CORNERS AND MAIN CROSS STUD PARTITIONS SHALL BE BRACED AT LEAST EVERY 25 FEET AND AT EACH END WITH 4'-0" WIDE x 1/2" THICK PLYWOOD DIAPHRAGM FROM THE TOP PLATE TO THE BOTTOM PLATE. FASTEN TO
- STUDS W/ 10d NAILS AT 6"o.c. MAX. AND AT 12"o.c. TO BLOCKING. ALL TIMBER FRAMING (EXCEPT WALL STUDS) MEMBERS SHALL BE NO. 2 SOUTHERN YELLOW PINE WITH 15% MAXIMUM MOISTURE UNLESS NOTED OTHERWISE. STUDS
- ALL HEADERS AND BEAMS INCLUDING PARALLAM BEAMS SHALL BE LATERALLY SUPPORTED ON THEIR COMPRESSION EDGE AT INTERVALS OF 24"o.c. OR CLOSER.

SHALL BE S-P-F (SPRUCE-PINE-FIR) GRADE 2 OR BETTER. COLUMNS SHALL BE NO. 2

- T-10 ALL STUD WALLS SHALL BE A MINIMUM OF 2x6 SPACED AT 16"o.c. FOR HEIGHT NOT EXCEEDING 13 FEET, UNLESS NOTED OTHERWISE ON FRAMING PLAN OR ARCHITECTURAL DRAWINGS. USE 2x8 SPACED AT 16"o.c. FOR HEIGHT NOT EXCEEDING 20 FEET. ALL STUD WALLS SHALL BE CONTINUOUS AND HAVE SOLID 2x WOOD BLOCKING AT MID-HEIGHT. SPACING OF BLOCKING SHALL NOT EXCEED 6'-0" VERTICALLY.
- SOLID 2x WOOD BLOCKING / BRIDGING SHALL BE PROVIDED AT ENDS AND AT POINT OF SUPPORTS OF ALL WOOD JOISTS AND SHALL BE PLACED BETWEEN SUPPORTS IN ROWS NOT EXCEEDING 8'-0"o.c. MAXIMUM.

### T-12

PLYWOOD ROOF SHEATHING SHALL BE 5/8" THICK APA STRUCTURAL 1 32/16 RATED EXTERIOR SHEATHING WITH EXTERIOR GLUE. PLYWOOD SHEATHING EXPOSED AT OVERHANGS OR OTHERWISE PERMANENTLY EXPOSED TO WEATHER SHALL BE C-C EXT. - DFPA GRADE OR BETTER. INSTALL WITH LONG DIMENSION OF PANEL ACROSS SUPPORTS. SPACE PANEL ENDS AND PANEL EDGES 1/8" UNLESS OTHERWISE RECOMMENDED BY THE PANEL MANUFACTURER. SUITABLE EDGE SUPPORT SHALL BE PROVIDED IN ACCORDANCE WITH RECOMMENDATIONS OF THE AMERICAN PLYWOOD ASSOCIATION BY USE OF PLY CLIPS, TONGUE AND GROOVE PANELS, OR LUMBER BLOCKING BETWEEN JOIST. PANEL END JOINTS SHALL OCCUR OVER FRAMING. PLYWOOD ROOF SHEATHING SHALL BE ATTACHED TO SUPPORT FRAMING USING 8d NAILS. NAIL SPACING SHALL BE 6" ALONG PANEL EDGES AND 12" AT INTERMEDIATE SUPPORTS.

T-13 USE THE FOLLOWING MINIMUM MEMBERS UNLESS NOTED OTHERWISE. 2x8 ROOF RAFTERS AT 24"o.c. 2x6 CEILING JOISTS AT 16"o.c.

- PLYWOOD FLOOR SHEATHING SHALL BE 3/4" WHERE SHOWN ON PLANS 16"o.c. APA RATED STURD-1-FLOOR, EXPOSURE 1. GLUE PANELS AND FASTEN WITH 8d NAILS SPACED AT 12"o.c. ALONG ALL SUPPORTS. LEAVE 1/8" SPACE AT ALL PANEL EDGES INSTALL PANELS CONTINUOUSLY OVER TWO OR MORE SPANS WITH THE LONG DIMENSION OR STRENGTH AXIS ACROSS SUPPORTS.
- T-15 ALL WOOD IN CONTACT WITH STEEL, MASONRY CONCRETE, AND / OR GROUND SHALL BE PRESSURE PRESERVATIVE TREATED AND RATED FOR GROUND
- T-16 UNLESS OTHERWISE SHOWN ON PLANS, WOOD LINTELS OVER OPENINGS SHALL BE DOUBLE 2x8 WITH 1/2" PLYWOOD HEADERS FOR SPANS UNDER 6'-0". FOR SPANS 6'-0" TO 8'-0", WOOD LINTELS SHALL BE DOUBLE 2x12 WITH 1/2" PLYWOOD HEADERS.
- HEADERS, JOISTS, RAFTERS AND OTHER FRAMING MEMBER CONNECTIONS (OTHER THAN BEARING CONNECTIONS) SHALL USE METAL HANGERS PER THE MANUFACTURERS' RECOMMENDATIONS. SEE PLANS AND DETAILS FOR REQUIRED

### PREFABRICATED WOOD ROOF TRUSSES:

- WT-1 PREFABRICATED WOOD TRUSSES SHALL BE DESIGNED BY A REGISTERED PROFESSIONAL ENGINEER, LICENSED IN TEXAS, IN ACCORDANCE WITH THE DESIGN SPECIFICATION FOR LIGHT METAL PLATE CONNECTED WOOD TRUSSES, CODE APPLICABLE TPI EDITION FOR THE TRUSS PLATE INSTITUTE
- WT-2 TRUSS MEMBERS AND CONNECTIONS SHALL BE PROPORTIONED, WITH A MAXIMUM ALLOWABLE STRESS INCREASE FOR LOAD DURATION OF 25 PERCENT, TO WITHSTAND THE FOLLOWING SUPERIMPOSED LOADS. LOADS SHOWN DO NOT INCLUDE TRUSS SELF WEIGHT.
  - DEAD LOAD .. .. 5 P.S.F. APPLIED TO TOP CHORD OF TRUSS (INCLUDING TRUSS OWN WEIGHT & METAL ROOF) . 10 P.S.F. APPLIED TO BOTTOM CHORD OF TRUSS
    - . 20 P.S.F. APPLIED TO TOP CHORD OF TRUSS . 10 P.S.F. APPLIED TO BOTTOM CHORD OF TRUSS (NOT ACTING SIMULTANEOUSLY WITH TOP CHORD LIVE
      - . 17 P.S.F. NET UPLIFT APPLIED TO TOP CHORD OF TRUSS
- WT-3 THE TRUSS MANUFACTURER SHALL SUBMIT THE FOLLOWING CERTIFICATIONS, SEALED BY THE ENGINEER RESPONSIBLE FOR DESIGN, FOR THE ARCHITECTS APPROVAL PRIOR TO FABRICATION OF ANY MATERIALS:
  - A. CERTIFICATION THAT THE MANUFACTURER IS LICENSED TO FABRICATE TRUSSES UTILIZING THE CONNECTOR SYSTEM PROPOSED.
  - B. CERTIFICATION THAT THE TRUSSES ARE DESIGNED TO MEET THE LOAD CRITERIA SPECIFIED HEREIN.

TEMPORARY BRACING AND LAYOUT PRIOR TO FABRICATION OF MATERIALS.

- WT-4 FABRICATION AND INSTALLATION DRAWINGS SHALL BE SUBMITTED TO THE CONTRACTOR AND DESIGN TEAM FOR REVIEW AND APPROVAL OF SIZE, SHAPE.
- TOP CHORDS SHALL BE DESIGNED TO RESIST BENDING INDUCED BY THE ROOF DEAD LOAD AND LIVE LOAD.
- WT-6 DEFLECTIONS DUE TO LIVE LOAD SHALL BE LIMITED TO L/360.
- CONNECT TRUSSES TO BEARING WALLS OR SUPPORT BEAMS AT EACH END WITH FRAMING ANCHOR TO BE SPECIFIED AFTER TRUSS SHOP DRAWING SUBMITTAL CONTRACTOR SHOULD INCLUDE A MINIMUM \$2,500.00 ALLOWANCE FOR MATERIALS AND INCLUDE ALL LABOR IN HIS BID.
- PROVIDE LONGITUDINAL AND DIAGONAL WOOD BRACES TO TRUSSED RAFTER RIDGES AT 20'-0" o.c. MAXIMUM. TRUSSED RAFTER CONNECTIONS SHALL HAVE A MINIMUM OF 2 - 16D NAILS UNTIL PERMANENT ROOF SHEATHING IS APPLIED. SEE DETAIL 1 / S-502.
- WT-9 PROVIDE BOTTOM CHORD BRACING AS SHOWN ON DETAIL 2 / S-502 AS TRUSSES ARE SET IN PLACE. BOTTOM CHORD BRACING TO BE LEFT IN PLACE AFTER ERECTION IS COMPLETED EXCEPT SOLID SHEATHING IS ATTACHED DIRECTLY TO BOTTOM OF TRUSS TO ACT AS BRACING.
- WT-10 CARE SHALL BE TAKEN DURING THE DELIVERY AND INSTALLATION OF THE TRUSSES TO PREVENT EXCESSIVE LATERAL BENDING AND POSSIBLE JOINT DAMAGE. TRUSSES SHALL BE HANDLED, INSTALLED, AND BRACED IN ACCORDANCE WITH TRUSS PLATE INSTITUTE HIB-91 COMMENTARY AND RECOMMENDATIONS FOR HANDLING INSTALLING AND BRACING METAL PLATE CONNECTED WOOD TRUSSES. DESIGN FOR TRUSS BRACING SHALL BE IN ACCORDANCE WITH TRUSS PLATE INSTITUTE DSB-89 RECOMMENDED DESIGN SPECIFICATION FOR TEMPORARY BRACING OF METAL PLATE CONNECTED WOOD TRUSSES. ALL DAMAGED TRUSSES SHALL BE REPLACED. ONLY REPAIRS AUTHORIZED BY THE TRUSS MANUFACTURER'S ENGINEER WILL BE ALLOWED. REPAIR DETAILS BY THE TRUSS MANUFACTURER'S ENGINEER SHALL BE SUBMITTED TO THE DESIGN TEAM FOR REVIEW PRIOR TO MAKING REPAIRS.
- WT-11 ALL TRUSSES SHALL BE INSTALLED PLUMB, SQUARE AND PROPERLY ALIGNED AT THE SPECIFIED SPACING. BRACING SHALL BE PROVIDED BOTH DURING ERECTION AND AFTER PERMANENT INSTALLATION HAS BEEN COMPLETED. GABLE END OR FIRST TRUSS BRACING, LATERAL BRACING, CROSS BRACING AND DIAGONAL BRACING CAN BE USED TO PROVIDE TEMPORARY AND / OR PERMANENT BRACING.

### PREFABRICATED WOOD FLOOR TRUSSES:

- FT-1 PREFABRICATED WOOD TRUSSES SHALL BE DESIGNED BY A REGISTERED SPECIFICATION FOR LIGHT METAL PLATE CONNECTED WOOD TRUSSES. CODE APPLICABLE TPI EDITION FOR THE TRUSS PLATE INSTITUTE.
- ALLOWABLE STRESS INCREASE FOR LOAD DURATION OF 25 PERCENT, TO WITHSTAND THE FOLLOWING SUPERIMPOSED LOADS. LOADS SHOWN DO NOT
  - DEAD LOAD . .. 20 P.S.F. APPLIED TO TOP CHORD OF TRUSS (INCLUDING TRUSS OWN WEIGHT & METAL ROOF)
  - .. 40 P.S.F. APPLIED TO TOP CHORD OF TRUSS LIVE LOAD ... . 10 P.S.F. APPLIED TO BOTTOM CHORD OF TRUSS (NOT ACTING SIMULTANEOUSLY WITH TOP CHORD LIVE
- 120 PLF. WHERE SECOND FLOOR WALLS ARE SUPPORTED BY WOOD FLOOR TRUSSES, THE FLOOR TRUSS DESIGNER SHALL SHALL DESIGN THE FLOOR TRUSSES WITH ADDED DEAD LOADING FROM SECOND FLOOR WALLS.
- FT-4 THE TRUSS MANUFACTURER SHALL SUBMIT THE FOLLOWING CERTIFICATIONS. SEALED BY THE ENGINEER RESPONSIBLE FOR DESIGN, FOR THE ARCHITECTS APPROVAL PRIOR TO FABRICATION OF ANY MATERIALS:
  - A. CERTIFICATION THAT THE MANUFACTURER IS LICENSED TO FABRICATE
  - B. CERTIFICATION THAT THE TRUSSES ARE DESIGNED TO MEET THE LOAD CRITERIA SPECIFIED HEREIN.
- CONTRACTOR AND DESIGN TEAM FOR REVIEW AND APPROVAL OF SIZE, SHAPE, TEMPORARY BRACING AND LAYOUT PRIOR TO FABRICATION OF MATERIALS.
- DEAD LOAD AND LIVE LOAD.
- FT-7 DEFLECTIONS DUE TO LIVE LOAD SHALL BE LIMITED TO L/480.
- FT-8 CONNECT TRUSSES TO BEARING WALLS OR SUPPORT BEAMS AT EACH END WITH FRAMING ANCHOR TO BE SPECIFIED AFTER TRUSS SHOP DRAWING SUBMITTAL. CONTRACTOR SHOULD INCLUDE A MINIMUM \$1,500.00 ALLOWANCE FOR MATERIALS AND INCLUDE ALL LABOR IN HIS BID.
- FT-9 CARE SHALL BE TAKEN DURING THE DELIVERY AND INSTALLATION OF THE TRUSSES TO PREVENT EXCESSIVE LATERAL BENDING AND POSSIBLE JOINT DAMAGE. TRUSSES SHALL BE HANDLED, INSTALLED, AND BRACED IN ACCORDANCE WITH TRUSS PLATE INSTITUTE HIB-91 COMMENTARY AND RECOMMENDATIONS FOR HANDLING INSTALLING AND BRACING METAL PLATE CONNECTED WOOD TRUSSES. DESIGN FOR TRUSS BRACING SHALL BE IN ACCORDANCE WITH TRUSS PLATE INSTITUTE DSB-89 RECOMMENDED DESIGN SPECIFICATION FOR TEMPORARY SHALL BE REPLACED. ONLY REPAIRS AUTHORIZED BY THE TRUSS MANUFACTURER'S ENGINEER WILL BE ALLOWED. REPAIR DETAILS BY THE TRUSS MANUFACTURER'S ENGINEER SHALL BE SUBMITTED TO THE DESIGN TEAM FOR REVIEW PRIOR TO MAKING REPAIRS.
- FT-10 ALL TRUSSES SHALL BE INSTALLED PLUMB. SQUARE AND PROPERLY ALIGNED AT THE SPECIFIED SPACING. BRACING SHALL BE PROVIDED BOTH DURING ERECTION AND AFTER PERMANENT INSTALLATION HAS BEEN COMPLETED. FIRST TRUSS BRACING, LATERAL BRACING, CROSS BRACING AND DIAGONAL BRACING CAN BE USED TO PROVIDE TEMPORARY AND / OR PERMANENT BRACING.

## PRE-FILLED SACKED (BAGGED) SELECT FILL AND EPS NOTES:

- PF-1 SACKS FOR SELECT FILL ARE TO BE MIN. 6" MIL. THICK HIGH DENSITY POLYPROPYLENE OR WAX IMPREGNATED PAPER BAG OR POLYETHYLENE FREE OF HOLES OR TEARS.
- PLATE COMPACTOR. DO NOT USE HEAVY DUTY ROLLER COMPACTOR.
- PF-3 EXPANDED POLYSTYRENE (EPS19 OR GREATER), CONFORMING TO THE REQUIREMENTS OF ASTM D6817, CAN BE USED IN PLACE OF PRE-FILLED SACKED (BAGGED) SELECT FILL. SUBMIT EPS PRODUCT TO THE DESIGN TEAM FOR REVIEW AND APPROVAL PRIOR TO INSTALLATION.

- PROFESSIONAL ENGINEER, LICENSED IN TEXAS, IN ACCORDANCE WITH THE DESIGN
- FT-2 TRUSS MEMBERS AND CONNECTIONS SHALL BE PROPORTIONED, WITH A MAXIMUM INCLUDE TRUSS SELF WEIGHT.
  - . 10 P.S.F. APPLIED TO BOTTOM CHORD OF TRUSS
- ON PLANS DENOTES NON-LOAD-BEARING INTERIOR WALLS WEIGHING
- - TRUSSES UTILIZING THE CONNECTOR SYSTEM PROPOSED.
- FT-5 FABRICATION AND INSTALLATION DRAWINGS SHALL BE SUBMITTED TO THE
- FT-6 TOP CHORDS SHALL BE DESIGNED TO RESIST BENDING INDUCED BY THE FLOOR

- BRACING OF METAL PLATE CONNECTED WOOD TRUSSES. ALL DAMAGED TRUSSES

# (WHERE SPECIFICALLY NOTED AS BAGGED FILL ON SECTIONS OR DETAILS.)

- PF-2 COMPACT OVER SACK FILL AREAS WITH MINIMUM OF 2 PASSES OF HAND HELD

LIQUE DESIGN STUDIO, LLC **TEXAS REGISTRATION NUMBER: BR 3647** 

**816 CAMARON ST., SUITE #123,** SAN ANTONIO. TX 78212 (210) 373-9383

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### SPB ENGINEERING, LLC F-8020

INTERIM REVIEW ONLY DOCUMENT INCOMPLETE: NOT INTENDED FOR PERMIT BIDDING OR CONSTRUCTION. STEPHEN P. BOURASSA, P.E. TEXAS LIC. NO. 92624 2021.06.01



San Antonio, Texas 78231

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DATE DESCRIPTION SCHEDULE OF REVISIONS

# INTERMEDIATE REVIEW

PROJECT NUMBER: 210117 PROJECT DATE: 2021.06.01

PROJECT MANAGER: SPB PROJECT TEAM: SPB, DLB

STRUCTURAL NOTES

**FASTENING SCHEDULE:** 

	<u>F/</u>	ASTENING SCHEDULE:		Г
ITEM	DESCRIPTION OF BUILDING ELEMENTS	NUMBER AND TYPE OF FASTENER a,b,c  ROOF	SPACING AND LOCATION	ITE
				-
	Blocking between ceiling joists, rafters or trusses to top plate or other framing below	3 - 8d common (2 1/2" x 0.131"); or 3 - 10d box (3" x 0.128"); or 3 - 3" x 0.131" nails; or 3 - 3" 14 gage staples, 7/16" crown	Each end, toe nail	2
1	Blocking between rafters or truss not at the wall	2 - 8d common (2 1/2" x 0.131"); or 2 - 3" x 0.131" nails; or 2 - 3" 14 gage staples, 7/16" crown	Each end, toe nail	2
	top plate, to rafter or truss	2 - 16d common (3 1/2" x 0.162"); or 3 - 3" x 0.131" nails; or 3 - 3" 14 gage staples, 7/16" crown	End nail	
	Flat blocking to truss and web filler	16d common (3 1/2" x 0.162") @ 6" o.c.; or 3" x 0.131" nails @ 6" o.c.; or 3" 14 gage staples, 7/16" crown @ 6" o.c.	Face nail	2
2	Ceiling joists to top plate	3 - 8d common (2 1/2" x 0.131"); or 3 - 10d box (3" x 0.128"); or 3 - 3" x 0.131" nails; or 3 - 3" 14 gage staples, 7/16" crown	Each joist, toe nail	2
3	Ceiling joist not attached to parallel rafter, laps over partitions (no thrust)	3 - 16d common (3 1/2" x 0.162"); or 4 - 10d box (3" x 0.162"); or 4 - 3" x 0.131" nails; or 4 - 3" 14 gage staples, 7/16" crown	Face nail	
4	Ceiling joist attached to parallel rafter (heel joint)	Refer to specific details on the drawings	Face nail	
5	Collar tie to rafter, face nail or 1 1/4" x 20 ga. ridge strap to rafter	4 - 10d box (3" x 0.128"); or 3 - 10d common (3" x 0.148"); or 4 - 3" x 0.131" nails; or 4 - 3" 14 gage staples, 7/16" crown	Face nail each rafter	2
6	Rafter or roof truss to top plate	3 - 16d box (3 1/2" x 0.135"); or 3 - 10d common (3" x 0.148"); or 4 - 10d box (3" x 0.128"); or 4 - 3" x 0.131" nails; or	2 toe nails on one side and 1 toe nail on opposite side of each	
		4 - 3" 14 gage staples, 7/16" crown	rafter or truss i	2
		2 - 16d common (3 1/2" x 0.162"); or 3 - 10d box (3" x 0.128"); or 3 - 3" x 0.131" nails; or 3 - 3" 14 gage staples, 7/16" crown	End nail	2
7	Roof rafters to ridge, valley or hip rafters; or roof rafter to minimum 2-inch ridge beam	3 - 10d common (3" x 0.148"); or 3 - 16d box (3 1/2" x 0.135"); or 4 - 10d box (3" x 0.128"); or 4 - 3" x 0.131" nails; or 4 - 3" 14 gage staples, 7/16" crown	Toe nail	3
		WALL		
		16d common (3 1/2" x 0.162")	24" o.c. face nail	
8	Stud to stud (not at braced wall panels)	10d box (3" x 0.128"); or 3" x 0.131" nails; or 3 - 3" 14 gage staples, 7/16" crown	16" o.c. face nail	
		16d common (3 1/2" x 0.162")	16" o.c. face nail	3
9	Stud to stud and abbuting studs at intersecting	16d box (3 1/2" x 0.135")	12" o.c. face nail	3
3	wall corners (at braced wall panels)	3" x 0.131" nails; or	12" o.c. face nail	3
		3 - 3" 14 gage staples, 7/16" crown		
10	Built-up header (2" to 2" header) (unless detailed otherwise on the drawings)	16d common (3 1/2" x 0.162")  16d box (3 1/2" x 0.135")	16" o.c. each edge, face nail 12" o.c. each face, face nail	
		4 - 8d common (2 1/2" x 0.131"); or		
11	Continuous header to stud	4 - 10d box (3" x 0.128")	Toe nail	3
		16d common (3 1/2" x 0.162")	16" o.c. face nail	
12	Top plate to top plate	10d box (3" x 0.128"); or 3" x 0.131" nails; or 3" 14 gage staples, 7/16" crown	12" o.c. face nail	
13	Top plate to top plate, at end joints	8 - 16d common (3 1/2" x 0.162"); or 12 - 10d box (3" x 0.128"); or 12 - 3" x 0.131" nails; or 12 - 3" 14 gage staples, 7/16" crown	Each side of end joint, face nail (minimum 24" lap splice length each side of end joint)	3
		16d common (3 1/2" x 0.162")	16" o.c. face nail	3
14	Bottom plate to joist, rim joist, band joist or blocking (not at braced wall panels)	16d box (3 1/2" x 0.135"); or 3" x 0.131" nails; or 3" 14 gage staples, 7/16" crown	12" o.c. face nail	
15	Bottom plate to joist, rim joist, band joist or blocking at braced wall panels	2 - 16d common (3 1/2" x 0.162"); or 3 - 16d box (3 1/2" x 0.135"); or 4 - 3" x 0.131" nails; or 4 - 3" 14 gage staples, 7/16" crown	16" o.c. face nail	3
16	Stud to top or bottom plate	4 - 8d common (2 1/2" x 0.131"); or 4 - 10d box (3" x 0.128"); or 4 - 3" x 0.131" nails; or 4 - 3" 14 gage staples, 7/16" crown	Toe nail	3
10	oted to top or bottom plate	2 - 16d common (3 1/2" x 0.162"); or 3 - 10d box (3" x 0.128"); or 3 - 3" x 0.131" nails; or 3 - 3" 14 gage staples, 7/16" crown	End nail	3
17	Top or bottom plate to stud	2 - 16d common (3 1/2" x 0.162"); or 3 - 10d box (3" x 0.128"); or 3 - 3" x 0.131" nails; or 3 - 3" 14 gage staples, 7/16" crown	End nail	4
18	Top plates, laps at corners and intersection	2 - 16d common (3 1/2" x 0.162"); or 3 - 10d box (3" x 0.128"); or 3 - 3" x 0.131" nails; or 3 - 3" 14 gage staples, 7/16" crown	Face nail	4
19	1" brace to each stud and plate	2 - 8d common (2 1/2" x 0.131"); or 2 - 10d box (3" x 0.128"); or 2 - 3" x 0.131" nails; or 2 - 3" 14 gage staples, 7/16" crown	Face nail	
	I	1		

	DESCRIPTION OF BUILDING ELEMENTS	NUMBER AND TYPE OF FASTENER a,b,c		PACING AND LOCATION		
		FLOOR				
22	Joist to sill, top plate, or girder	4 - 8d box (2 1/2" x 0.113:); or 3 - 8d common (2 1/2" x 0.131"); or 3 - 10d box (3" x 0.128"); or 3 - 3" x 0.131" nails		Toe nail		
		8d box (2 1/2" x 0.113")	4"	o.c. toe nail		
23	Rim joist, band joist, or blocking to top plate, sill or other framing below	8d common (2 1/2" x 0.131"); or 10d box (3" x 0.128"); or 3" x 0.131" nails; or 3" 14 gage staples, 7/16" crown	6" o.c. toe nail			
24	1" x 6" subfloor or less to each joist	3 - 8d box (2 1/2" x 0.113"); or 2 - 8d common (2 1/2" x 0.131"); or 3 - 10d box (3" x 0.128"); or 2 - 1 3/4" 16 gage staples, 1" crown	Face nail			
25	2" subfloor to joist or girder	3 - 16d box (3 1/2" x 0.135"); or 2 - 16d common (3 1/2" x 0.162")	Blind and face nail			
26	2" planks (plank & beam - floor & roof)	3 - 16d box (3 1/2" x 0.135"); or 2 - 16d common (3 1/2" x 0.162")	At each	n bearing, face na		
		20d common (4" x 0.192")		face nail at top ar taggered on sides		
27	Built-up girders and beams, 2" lumber layers (unlessdetailed otherwise on the drawings)	10d box (3" x 0.128"); or 3" x 0.131" nails; or 3" 14 gage staples, 7/16" crown		face nail at top an taggered on sides		
		And: 2 - 20d common (4" x 0.192:); or 3 - 10d box (3" x 0.128"); or 3 - 3" x 0.131" nails; or 3 - 3" 14 gage staples, 7/16" crown	Ends and at each splice, face nail			
28	Ledger strip supporting joists or rafters	3 - 16d common (3 1/2" x 0.162"); or 4 - 10d box (3" x 0.128"); or 4 - 3" x 0.131" nails; or 4 - 3" 14 gage staples, 7/16" crown	Each jois	st or rafter, face n		
29	Joist to band joist or rim joist	3 - 16d common (3 1/2" x 0.162"); or 4 - 10d box (3" x 0.128"); or 4 - 3" x 0.131" nails; or 4 - 3" 14 gage staples, 7/16" crown		End nail		
30	Bridging or blocking to joist, rafter or truss	2 - 8d common (2 1/2" x 0.131"); or 2 - 10d box (3" x 0.128"); or 2 - 3" x 0.131" nails; or 2 - 3" 14 gage staples, 7/16" crown	Eac	ch end, toe nail		
		NELS (WSP), SUBFLOOR, ROOF AND INTERIOR WALL AND PARTICLEBOARD WALL SHEATHING TO FRAMING	j			
	OILATIINO TO TRAINING	AND I ANTIOLEBOARD WALL OTLATTING TO I RAMING	Edges	Intermediate		
		4d casing (1 1/2" v 0 080"); or	(inches)			
31	1/4"	4d casing (1 1/2" x 0.080"); or 4d finish (1 1/2" x 0.072")	(inches)	supports (inch		
31	3/8"		,			
		4d finish (1 1/2" x 0.072")  6d casing (2" x 0.099"); or	6	12 12 12 <sup>f</sup>		
		4d finish (1 1/2" x 0.072")  6d casing (2" x 0.099"); or 6d finish (Panel supports at 24 inches)  6d common or deformed (2" x 0.113") (subfloor and wall) i  8d common (2 1/2" x 0.131") (roof)	6	12 12 12 <sup>f</sup> 12 <sup>f</sup>		
32	3/8"	4d finish (1 1/2" x 0.072")  6d casing (2" x 0.099"); or 6d finish (Panel supports at 24 inches)  6d common or deformed (2" x 0.113") (subfloor and wall) i  8d common (2 1/2" x 0.131") (roof)  RSRS-01 nails (2 3/8" x 0.113") (roof)	6 6 6 6	12 12 12 <sup>f</sup> 12 <sup>f</sup> 12 <sup>f</sup>		
32		4d finish (1 1/2" x 0.072")  6d casing (2" x 0.099"); or 6d finish (Panel supports at 24 inches)  6d common or deformed (2" x 0.113") (subfloor and wall) i  8d common (2 1/2" x 0.131") (roof)  RSRS-01 nails (2 3/8" x 0.113") (roof) i  2 3/8" x 0.113" nails (subfloor and wall)	6 6 6 6 6	12 12 12 12 12 12 12 12 12		
32	3/8"	4d finish (1 1/2" x 0.072")  6d casing (2" x 0.099"); or 6d finish (Panel supports at 24 inches)  6d common or deformed (2" x 0.113") (subfloor and wall)  8d common (2 1/2" x 0.131") (roof)  RSRS-01 nails (2 3/8" x 0.113") (roof)  2 3/8" x 0.113" nails (subfloor and wall)  1 3/4" 16 gage staples, 7/16" crown (subfloor and wall)	6 6 6 6 6 4	12 12 12 12 12 12 12 12 12 8		
32	3/8"	4d finish (1 1/2" x 0.072")  6d casing (2" x 0.099"); or 6d finish (Panel supports at 24 inches)  6d common or deformed (2" x 0.113") (subfloor and wall)  8d common (2 1/2" x 0.131") (roof)  RSRS-01 nails (2 3/8" x 0.113") (roof)  2 3/8" x 0.113" nails (subfloor and wall)  1 3/4" 16 gage staples, 7/16" crown (subfloor and wall)  2 3/8" x 0.113" nails (roof)	6 6 6 6 6 4 4	12 12 12 12 12 12 12 12 12 8 8		
	3/8"	4d finish (1 1/2" x 0.072")  6d casing (2" x 0.099"); or 6d finish (Panel supports at 24 inches)  6d common or deformed (2" x 0.113") (subfloor and wall)  8d common (2 1/2" x 0.131") (roof)  RSRS-01 nails (2 3/8" x 0.113") (roof)  2 3/8" x 0.113" nails (subfloor and wall)  1 3/4" 16 gage staples, 7/16" crown (subfloor and wall)  2 3/8" x 0.113" nails (roof)  1 3/4" 16 gage staples, 7/16" crown (roof)	6 6 6 6 6 4 4 3	12 12 12 12 12 12 12 12 12 8 8 8		
32	3/8"	4d finish (1 1/2" x 0.072")  6d casing (2" x 0.099"); or 6d finish (Panel supports at 24 inches)  6d common or deformed (2" x 0.113") (subfloor and wall)  8d common (2 1/2" x 0.131") (roof)  RSRS-01 nails (2 3/8" x 0.113") (roof)  2 3/8" x 0.113" nails (subfloor and wall)  1 3/4" 16 gage staples, 7/16" crown (subfloor and wall)  2 3/8" x 0.113" nails (roof)  1 3/4" 16 gage staples, 7/16" crown (roof)  8d common (2 1/2" x 0.131"); or 6d deformed (2" x 0.113") (subfloor and wall)	6 6 6 6 6 4 4	12 12 12 12 12 12 12 12 12 8 8		
32	3/8" - 1/2"	4d finish (1 1/2" x 0.072")  6d casing (2" x 0.099"); or 6d finish (Panel supports at 24 inches)  6d common or deformed (2" x 0.113") (subfloor and wall)  8d common (2 1/2" x 0.131") (roof)  RSRS-01 nails (2 3/8" x 0.113") (roof)  2 3/8" x 0.113" nails (subfloor and wall)  1 3/4" 16 gage staples, 7/16" crown (subfloor and wall)  2 3/8" x 0.113" nails (roof)  1 3/4" 16 gage staples, 7/16" crown (roof)  8d common (2 1/2" x 0.131"); or	6 6 6 6 6 4 4 3	12 12 12 12 12 12 12 12 8 8 8		
32	3/8" - 1/2"	4d finish (1 1/2" x 0.072")  6d casing (2" x 0.099"); or 6d finish (Panel supports at 24 inches)  6d common or deformed (2" x 0.113") (subfloor and wall)  8d common (2 1/2" x 0.131") (roof)  RSRS-01 nails (2 3/8" x 0.113") (roof)  2 3/8" x 0.113" nails (subfloor and wall)  1 3/4" 16 gage staples, 7/16" crown (subfloor and wall)  2 3/8" x 0.113" nails (roof)  1 3/4" 16 gage staples, 7/16" crown (roof)  8d common (2 1/2" x 0.131"); or 6d deformed (2" x 0.113") (subfloor and wall)  2 3/8" x 0.113" nails; or	6 6 6 6 6 4 4 3	12 12 12 12 12 12 12 12 12 8 8 8 6 12		
33 33	3/8" - 1/2"  19/32" - 3/4"  7/8" - 1 1/4"	4d finish (1 1/2" x 0.072")  6d casing (2" x 0.099"); or 6d finish (Panel supports at 24 inches)  6d common or deformed (2" x 0.113") (subfloor and wall)  8d common (2 1/2" x 0.131") (roof)  RSRS-01 nails (2 3/8" x 0.113") (roof)  2 3/8" x 0.113" nails (subfloor and wall)  1 3/4" 16 gage staples, 7/16" crown (subfloor and wall)  2 3/8" x 0.113" nails (roof)  1 3/4" 16 gage staples, 7/16" crown (roof)  8d common (2 1/2" x 0.131"); or 6d deformed (2" x 0.113") (subfloor and wall)  2 3/8" x 0.113" nails; or 2" 16 gage staples, 7/16" crown  10d common (3" x 0.148"); or	6 6 6 6 6 4 4 3 6	12 12 12 12 12 12 12 12 12 8 8 8 6 12		
33 33 34	3/8" - 1/2"  19/32" - 3/4"  7/8" - 1 1/4"	4d finish (1 1/2" x 0.072")  6d casing (2" x 0.099"); or 6d finish (Panel supports at 24 inches)  6d common or deformed (2" x 0.113") (subfloor and wall)  8d common (2 1/2" x 0.131") (roof)  RSRS-01 nails (2 3/8" x 0.113") (roof)  2 3/8" x 0.113" nails (subfloor and wall)  1 3/4" 16 gage staples, 7/16" crown (subfloor and wall)  2 3/8" x 0.113" nails (roof)  1 3/4" 16 gage staples, 7/16" crown (roof)  8d common (2 1/2" x 0.131"); or 6d deformed (2" x 0.113") (subfloor and wall)  2 3/8" x 0.113" nails; or 2" 16 gage staples, 7/16" crown  10d common (3" x 0.148"); or 8d deformed (2 1/2" x 0.131")  IER EXTERIOR WALL SHEATHING	6 6 6 6 6 4 4 3 6	12 12 12 12 12 12 12 12 12 8 8 8 6 12		
32 33 34 35	3/8" - 1/2"  19/32" - 3/4"  7/8" - 1 1/4"  OTH	4d finish (1 1/2" x 0.072")  6d casing (2" x 0.099"); or 6d finish (Panel supports at 24 inches)  6d common or deformed (2" x 0.113") (subfloor and wall)  8d common (2 1/2" x 0.131") (roof)  RSRS-01 nails (2 3/8" x 0.113") (roof)  2 3/8" x 0.113" nails (subfloor and wall)  1 3/4" 16 gage staples, 7/16" crown (subfloor and wall)  2 3/8" x 0.113" nails (roof)  1 3/4" 16 gage staples, 7/16" crown (roof)  8d common (2 1/2" x 0.131"); or 6d deformed (2" x 0.113") (subfloor and wall)  2 3/8" x 0.113" nails; or 2" 16 gage staples, 7/16" crown  10d common (3" x 0.148"); or 8d deformed (2 1/2" x 0.131")  IER EXTERIOR WALL SHEATHING  1 1/2" galvanized roof nails (7/16" head diameter); or 1 1/4" 16 gage staple, 7/16" or 1" crown  1 3/4" galvanized roof nails (7/16" head diameter); or	6 6 6 6 6 4 4 3 6	12 12 12 12 12 12 12 12 8 8 6 12 8		
32 33 34 35	3/8" - 1/2"  19/32" - 3/4"  7/8" - 1 1/4"  OTH  1/2" fiberboard sheathing <sup>k</sup>	4d finish (1 1/2" x 0.072")  6d casing (2" x 0.099"); or 6d finish (Panel supports at 24 inches)  6d common or deformed (2" x 0.113") (subfloor and wall)  8d common (2 1/2" x 0.131") (roof)  RSRS-01 nails (2 3/8" x 0.113") (roof)  2 3/8" x 0.113" nails (subfloor and wall)  1 3/4" 16 gage staples, 7/16" crown (subfloor and wall)  2 3/8" x 0.113" nails (roof)  1 3/4" 16 gage staples, 7/16" crown (roof)  8d common (2 1/2" x 0.131"); or 6d deformed (2" x 0.113") (subfloor and wall)  2 3/8" x 0.113" nails; or 2" 16 gage staples, 7/16" crown  10d common (3" x 0.148"); or 8d deformed (2 1/2" x 0.131")  IER EXTERIOR WALL SHEATHING  1 1/2" galvanized roof nails (7/16" head diameter); or 1 1/4" 16 gage staple, 7/16" or 1" crown  1 3/4" galvanized roof nails (7/16" head diameter); or 1 1/2" 16 gage staple, 7/16" or 1" crown	6 6 6 6 6 4 4 3 6 4 6	12 12 12 12 12 12 12 12 8 8 8 6 12 8 12		
32 33 34 35 36 37	3/8" - 1/2"  19/32" - 3/4"  7/8" - 1 1/4"  OTH  1/2" fiberboard sheathing <sup>k</sup> 25/32" fiberboard sheathing <sup>k</sup> WOOD STRUCTURAL PANELS	4d finish (1 1/2" x 0.072")  6d casing (2" x 0.099"); or 6d finish (Panel supports at 24 inches)  6d common or deformed (2" x 0.113") (subfloor and wall)  8d common (2 1/2" x 0.131") (roof)  RSRS-01 nails (2 3/8" x 0.113") (roof)  2 3/8" x 0.113" nails (subfloor and wall)  1 3/4" 16 gage staples, 7/16" crown (subfloor and wall)  2 3/8" x 0.113" nails (roof)  1 3/4" 16 gage staples, 7/16" crown (roof)  8d common (2 1/2" x 0.131"); or 6d deformed (2" x 0.113") (subfloor and wall)  2 3/8" x 0.113" nails; or 2" 16 gage staples, 7/16" crown  10d common (3" x 0.148"); or 8d deformed (2 1/2" x 0.131")  IER EXTERIOR WALL SHEATHING  1 1/2" galvanized roof nails (7/16" head diameter); or 1 1/4" 16 gage staple, 7/16" or 1" crown  1 3/4" galvanized roof nails (7/16" head diameter); or 1 1/2" 16 gage staple, 7/16" or 1" crown  6d common (2 1/2" x 0.131")  COMBINATION SUBFLOOR UNDERLAYMENT TO FRAM  8d common (2 1/2" x 0.131"); or	6 6 6 6 6 4 4 3 6 4 6	12 12 12 12 12 12 12 12 8 8 8 6 12 8 12		
32 33 34 35	3/8" - 1/2"  19/32" - 3/4"  7/8" - 1 1/4"  OTH  1/2" fiberboard sheathing <sup>k</sup>	4d finish (1 1/2" x 0.072")  6d casing (2" x 0.099"); or 6d finish (Panel supports at 24 inches)  6d common or deformed (2" x 0.113") (subfloor and wall)  8d common (2 1/2" x 0.131") (roof)  RSRS-01 nails (2 3/8" x 0.113") (roof)  2 3/8" x 0.113" nails (subfloor and wall)  1 3/4" 16 gage staples, 7/16" crown (subfloor and wall)  2 3/8" x 0.113" nails (roof)  1 3/4" 16 gage staples, 7/16" crown (roof)  8d common (2 1/2" x 0.131"); or 6d deformed (2" x 0.113") (subfloor and wall)  2 3/8" x 0.113" nails; or 2" 16 gage staples, 7/16" crown  10d common (3" x 0.148"); or 8d deformed (2 1/2" x 0.131")  EER EXTERIOR WALL SHEATHING  1 1/2" galvanized roof nails (7/16" head diameter); or 1 1/4" 16 gage staple, 7/16" or 1" crown  1 3/4" galvanized roof nails (7/16" head diameter); or 1 1/2" 16 gage staple, 7/16" or 1" crown  COMBINATION SUBFLOOR UNDERLAYMENT TO FRAM  8d common (2 1/2" x 0.131"); or 6d deformed (2" x 0.113")  8d common (2 1/2" x 0.131"); or	6 6 6 6 6 4 4 3 6 4 6	12 12 12 12 12 12 12 12 8 8 8 6 12 8 12		
32 33 34 35 36 37	3/8" - 1/2"  19/32" - 3/4"  7/8" - 1 1/4"  OTH  1/2" fiberboard sheathing k  25/32" fiberboard sheathing k  WOOD STRUCTURAL PANELS	4d finish (1 1/2" x 0.072")  6d casing (2" x 0.099"); or 6d finish (Panel supports at 24 inches)  6d common or deformed (2" x 0.113") (subfloor and wall)  8d common (2 1/2" x 0.131") (roof)  RSRS-01 nails (2 3/8" x 0.113") (roof)  2 3/8" x 0.113" nails (subfloor and wall)  1 3/4" 16 gage staples, 7/16" crown (subfloor and wall)  2 3/8" x 0.113" nails (roof)  1 3/4" 16 gage staples, 7/16" crown (roof)  8d common (2 1/2" x 0.131"); or 6d deformed (2" x 0.113") (subfloor and wall)  2 3/8" x 0.113" nails; or 2" 16 gage staples, 7/16" crown  10d common (3" x 0.148"); or 8d deformed (2 1/2" x 0.131")  IER EXTERIOR WALL SHEATHING  1 1/2" galvanized roof nails (7/16" head diameter); or 1 1/4" 16 gage staple, 7/16" or 1" crown  1 3/4" galvanized roof nails (7/16" head diameter); or 1 1/2" 16 gage staple, 7/16" or 1" crown  COMBINATION SUBFLOOR UNDERLAYMENT TO FRAM  8d common (2 1/2" x 0.131"); or 6d deformed (2" x 0.113")  8d common (2 1/2" x 0.131"); or 8d deformed (3" x 0.148"); or	6 6 6 6 6 4 4 3 6 4 6 3 3	12 12 12 12 12 12 12 12 8 8 8 6 12 8 12 6 12		
32 33 34 35 36 37 38 39	3/8" - 1/2"  19/32" - 3/4"  7/8" - 1 1/4"  OTH  1/2" fiberboard sheathing k  25/32" fiberboard sheathing k  WOOD STRUCTURAL PANELS  3/4" and less  7/8" - 1"	4d finish (1 1/2" x 0.072")  6d casing (2" x 0.099"); or 6d finish (Panel supports at 24 inches)  6d common or deformed (2" x 0.113") (subfloor and wall)  8d common (2 1/2" x 0.131") (roof)  RSRS-01 nails (2 3/8" x 0.113") (roof)  2 3/8" x 0.113" nails (subfloor and wall)  1 3/4" 16 gage staples, 7/16" crown (subfloor and wall)  2 3/8" x 0.113" nails (roof)  1 3/4" 16 gage staples, 7/16" crown (roof)  8d common (2 1/2" x 0.131"); or 6d deformed (2" x 0.113") (subfloor and wall)  2 3/8" x 0.113" nails; or 2" 16 gage staples, 7/16" crown  10d common (3" x 0.148"); or 8d deformed (2 1/2" x 0.131")  IER EXTERIOR WALL SHEATHING  1 1/2" galvanized roof nails (7/16" head diameter); or 1 1/4" 16 gage staple, 7/16" or 1" crown  1 3/4" galvanized roof nails (7/16" head diameter); or 1 1/2" 16 gage staple, 7/16" or 1" crown  COMBINATION SUBFLOOR UNDERLAYMENT TO FRAM 8d common (2 1/2" x 0.131"); or 6d deformed (2" x 0.113")  8d common (2 1/2" x 0.131"); or 8d deformed (2 1/2" x 0.131")	6 6 6 6 6 4 4 3 6 4 6 3 3 3	12 12 12 12 12 12 8 8 8 6 12 8 12 6 12		
32 33 34 35 36 37 38 39	3/8" - 1/2"  19/32" - 3/4"  7/8" - 1 1/4"  OTH  1/2" fiberboard sheathing k  25/32" fiberboard sheathing k  WOOD STRUCTURAL PANELS  3/4" and less  7/8" - 1"	4d finish (1 1/2" x 0.072")  6d casing (2" x 0.099"); or 6d finish (Panel supports at 24 inches)  6d common or deformed (2" x 0.113") (subfloor and wall)  8d common (2 1/2" x 0.131") (roof)  RSRS-01 nails (2 3/8" x 0.113") (roof)  2 3/8" x 0.113" nails (subfloor and wall)  1 3/4" 16 gage staples, 7/16" crown (subfloor and wall)  2 3/8" x 0.113" nails (roof)  1 3/4" 16 gage staples, 7/16" crown (roof)  8d common (2 1/2" x 0.131"); or 6d deformed (2" x 0.113") (subfloor and wall)  2 3/8" x 0.113" nails; or 2" 16 gage staples, 7/16" crown  10d common (3" x 0.148"); or 8d deformed (2 1/2" x 0.131")  IER EXTERIOR WALL SHEATHING  1 1/2" galvanized roof nails (7/16" head diameter); or 1 1/4" 16 gage staple, 7/16" or 1" crown  1 3/4" galvanized roof nails (7/16" head diameter); or 1 1/2" 16 gage staple, 7/16" or 1" crown  COMBINATION SUBFLOOR UNDERLAYMENT TO FRAM  8d common (2 1/2" x 0.131"); or 6d deformed (2" x 0.113")  8d common (2 1/2" x 0.131"); or 8d deformed (3" x 0.148"); or	6 6 6 6 6 4 4 3 6 4 6 3 3 3	12 12 12 12 12 12 12 12 8 8 8 6 12 8 12 6 12		

For SI: 1 inch = 25.4 mm, 1 foot = 304.8mm

- a. Nails are smooth-common, box or deformed shanks except where otherwise stated. Nails used for framing and sheathing connections shall have minimum average bending yield strengths as shown: 80 ksi for shank diameter of 0.192 inch (20d common nail), 90 ksi for shank diameters larger than 0.177 inc, and 100 ksi for shank diameters of 0.142 inch or less.
- b. Staples are 16 gage wire and have a minimum 7/16-inch on diameter crown width.
- c. Nails shall be spaced at not more than 6 inches on center at all supports where spans are 48 inches or greater.
- d. Four-foot by 8-foot or 4-foot by 9-foot panels shall be applied vertically.
- e. Refer to specific details for spacing of fasteners not included in this table.
- f. Where the ultimate design wind speed is 130 mph or less, nails for attaching wood structural panel roof sheathing to gable end wall framing shall be spaced 6 inches on center. Where the ultimate design wind speed is greater than 130 mph, nails for attaching panel roof sheathing to intermediate supports shall be spaced at 6 inches on center for minimum 48-inch distance from ridges, eaves and gable end walls; and 4 inches on center to gable end wall framing.
- g. Gypsum sheathing shall conform to ASTM C 1396 and shall be inaccordance with GA 253. Fiberboard sheathing shall conform to ASTM C 208
- h. Spacing of fasteners on floor sheathing panel egdes applies to panel edges supported by framing members and required blocking and at floor perimeters only. Spacing of fasteners on roof sheathing panel egdes applies to panel edges supported by framing members and required blocking. Blocking of roof or floor sheathing panel edges perpendicular to the framing members need be provided unless detailed otherwise.
- i. Where a rafter is fastened to an adjacent parallel ceiling joits in accordance with this schedule, provide two toe nails on one side of the rafter and toe nails from the ceiling joist to top plate in accordance with this schedule. The toe nail on the opposite side of the rafter shall not be required.
- Nails spaced at 6 inches at intermediate supports where spans are 48 inches or more. For nailing of wood structural panel and particleboard diaphragms and shear walls unless detailed otherwise on the drawings. Nails for wall sheathing are permitted to be common, box or casing
- k. Spacing shall be 6 inches on center on the edges and 12 inches on center at intermediate supports for nonstructural applications. Panel supports at 16 inches (20 inches if strength axis in the long direction of the panel, unless otherwise marked).
- I. RSRS-01 is a Roof Sheathing Ring Shank nail meeting the specifications in ASTM F1667.
- m. Nailing requirements indicated in the FASTENING SCHEDULE are required minimums unless otherwise detailed on the drawings.

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2021.06.01

# HACKBERRY

909 N HAC

SPB ENGINEERING, LLC
Structural Consultants
s Firm Registration Number: F-8020

Texas Firm Registration Number: F-8020
Mailing Address: 14439 N.W. Military Hwy., Suite 108 - 417
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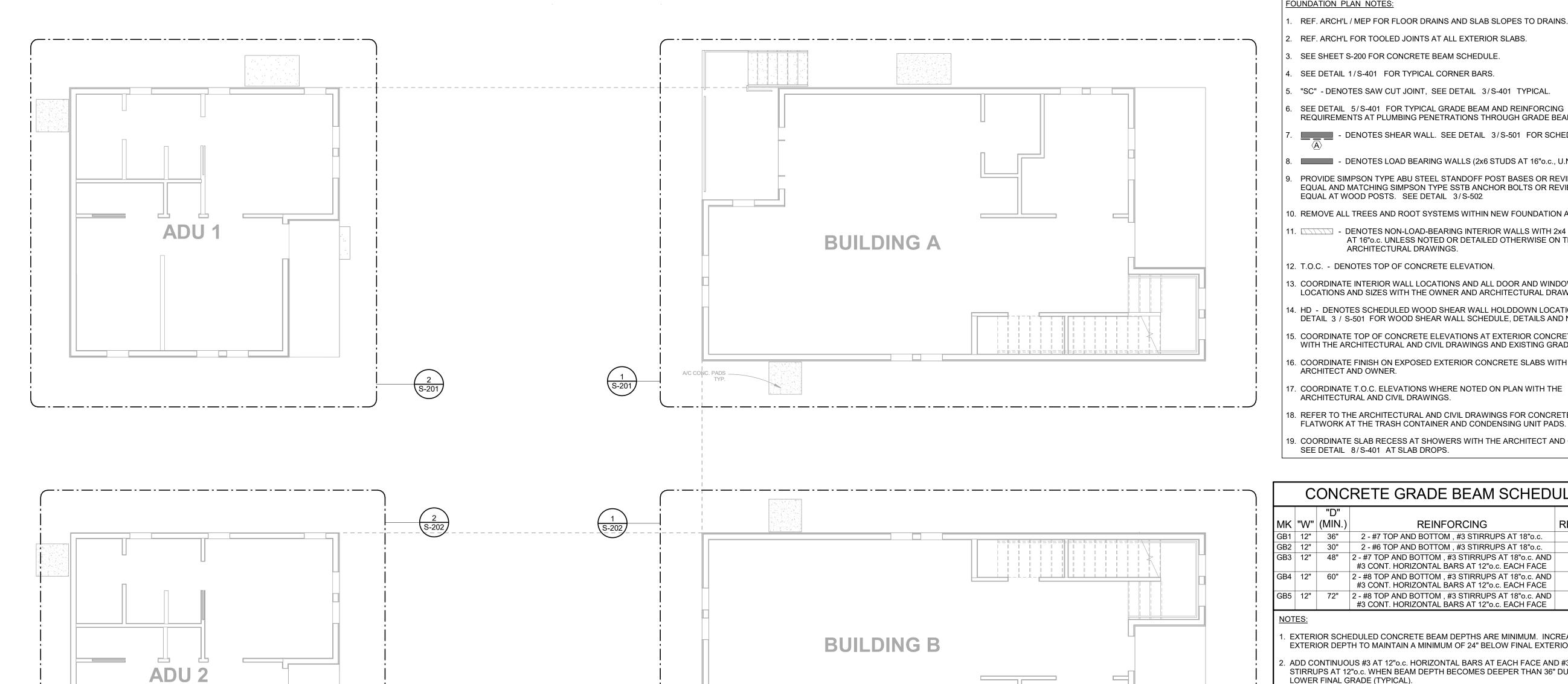
SCHEDULE OF REVISIONS

### INTERMEDIATE REVIEW

DESCRIPTION

PROJECT NUMBER: 210117
PROJECT DATE: 2021.06.01
PROJECT MANAGER: SPB
PROJECT TEAM: SPB, DLB

**FASTENING SCHEDULE** 



**FOUNDATION PLAN NOTES:** 

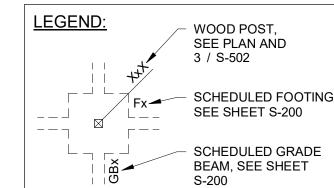
- REF. ARCH'L / MEP FOR FLOOR DRAINS AND SLAB SLOPES TO DRAINS.
- REF. ARCH'L FOR TOOLED JOINTS AT ALL EXTERIOR SLABS.
- SEE SHEET S-200 FOR CONCRETE BEAM SCHEDULE.
  - 4. SEE DETAIL 1/S-401 FOR TYPICAL CORNER BARS.
  - 5. "SC" DENOTES SAW CUT JOINT, SEE DETAIL 3/S-401 TYPICAL.
  - SEE DETAIL 5/S-401 FOR TYPICAL GRADE BEAM AND REINFORCING REQUIREMENTS AT PLUMBING PENETRATIONS THROUGH GRADE BEAMS.
  - DENOTES SHEAR WALL. SEE DETAIL 3/S-501 FOR SCHEDULE.
  - DENOTES LOAD BEARING WALLS (2x6 STUDS AT 16"o.c., U.N.O.)
  - PROVIDE SIMPSON TYPE ABU STEEL STANDOFF POST BASES OR REVIEWED EQUAL AND MATCHING SIMPSON TYPE SSTB ANCHOR BOLTS OR REVIEWED EQUAL AT WOOD POSTS. SEE DETAIL 3/S-502.
  - 10. REMOVE ALL TREES AND ROOT SYSTEMS WITHIN NEW FOUNDATION AREAS.
  - 11. DENOTES NON-LOAD-BEARING INTERIOR WALLS WITH 2x4 STUDS AT 16"o.c. UNLESS NOTED OR DETAILED OTHERWISE ON THE ARCHITECTURAL DRAWINGS.
  - 12. T.O.C. DENOTES TOP OF CONCRETE ELEVATION.
  - 13. COORDINATE INTERIOR WALL LOCATIONS AND ALL DOOR AND WINDOW LOCATIONS AND SIZES WITH THE OWNER AND ARCHITECTURAL DRAWINGS.
  - 14. HD DENOTES SCHEDULED WOOD SHEAR WALL HOLDDOWN LOCATION. SEE DETAIL 3 / S-501 FOR WOOD SHEAR WALL SCHEDULE, DETAILS AND NOTES.
  - 15. COORDINATE TOP OF CONCRETE ELEVATIONS AT EXTERIOR CONCRETE SLABS
  - WITH THE ARCHITECTURAL AND CIVIL DRAWINGS AND EXISTING GRADES. 6. COORDINATE FINISH ON EXPOSED EXTERIOR CONCRETE SLABS WITH THE
  - ARCHITECT AND OWNER.
  - ARCHITECTURAL AND CIVIL DRAWINGS.
  - 18. REFER TO THE ARCHITECTURAL AND CIVIL DRAWINGS FOR CONCRETE FLATWORK AT THE TRASH CONTAINER AND CONDENSING UNIT PADS.
- 19. COORDINATE SLAB RECESS AT SHOWERS WITH THE ARCHITECT AND OWNER. SEE DETAIL 8/S-401 AT SLAB DROPS.

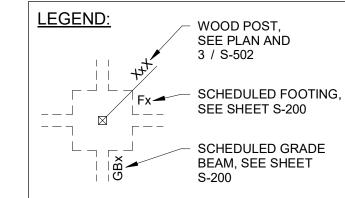
# CONCRETE GRADE BEAM SCHEDULE

		"D"		
MK	"W"	(MIN.)	REINFORCING	REMARKS
GB1	12"	36"	2 - #7 TOP AND BOTTOM , #3 STIRRUPS AT 18"o.c.	
GB2	12"	30"	2 - #6 TOP AND BOTTOM , #3 STIRRUPS AT 18"o.c.	
GB3	12"	48"	2 - #7 TOP AND BOTTOM , #3 STIRRUPS AT 18"o.c. AND #3 CONT. HORIZONTAL BARS AT 12"o.c. EACH FACE	
GB4	12"	60"	2 - #8 TOP AND BOTTOM , #3 STIRRUPS AT 18"o.c. AND #3 CONT. HORIZONTAL BARS AT 12"o.c. EACH FACE	
GB5	12"	72"	2 - #8 TOP AND BOTTOM , #3 STIRRUPS AT 18"o.c. AND #3 CONT. HORIZONTAL BARS AT 12"o.c. EACH FACE	

### NOTES:

- . EXTERIOR SCHEDULED CONCRETE BEAM DEPTHS ARE MINIMUM. INCREASE EXTERIOR DEPTH TO MAINTAIN A MINIMUM OF 24" BELOW FINAL EXTERIOR GRADE.
- ADD CONTINUOUS #3 AT 12"o.c. HORIZONTAL BARS AT EACH FACE AND #3 STIRRUPS AT 12"o.c. WHEN BEAM DEPTH BECOMES DEEPER THAN 36" DUE TO LOWER FINAL GRADE (TYPICAL).
- 8. PROVIDE 40 BAR DIAMETER LAPS AT ALL SPLICES IN CONTINUOUS TOP AND BOTTOM BARS AT GRADE BEAMS.







F8

F9

F10

8'-0"

8'-6"

9'-0"

7 - # 5 E.W.

7 - # 5 E.W.

7 - # 5 E.W.

F00	TING S	CHEDULE	<u>LEGEND</u>	<b>"a"</b>		NOTES:
TYPE	" a "	REINFORCING	]	<b> </b>	1	FOOTING DEPTH SHALL BE THE SAME DEPTH AS THE
F1	3'-0"	4 - # 5 E.W.		lii l	ļ1	DEEPEST INTERSECTING GRADE BEAM.
F2	4'-0"	4 - # 5 E.W.				2. PROVIDE BOTTOM MAT OF REINFORCING AS SCHEDULED
F3	5'-0"	5 - # 5 E.W.		_   g _	<del> </del>	WHERE FOOTINGS ARE POURED MONOLITHICALLY WITH
F4	6'-0"	5 - # 5 E.W.			<u> </u>	THE BUILDING FOUNDATION GRADE BEAMS.
F5	6'-6"	6 - # 5 E.W.	<u> </u>			2. PROVIDE TOD AND POTTOM MATS OF DEINICOSCING AS
F6	7'-0"	6 - # 5 E.W.				PROVIDE TOP AND BOTTOM MATS OF REINFORCING AS     SCHEDULED WHERE FOOTINGS ARE ISOLATED.
F7	7'-6"	7 - # 5 E.W.	<b>」</b> ∷			SOME SEES WHERE I SOME OF WILLIAM IS SEED.
	01.011	7 11 5 5 14	I			4. THE EGOTING BEADING ELEVATION / OUBEAGE OUTLINE BE

PROPERTY LINE

ADU 1

LEVEL.

"a"

4. THE FOOTING BEARING ELEVATION / SURFACE SHALL BE

BLDG. A

ADU 2

BLDG. B

KEY PLAN

(NOT FOR CONSTRUCTION)

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DATE DESCRIPTION

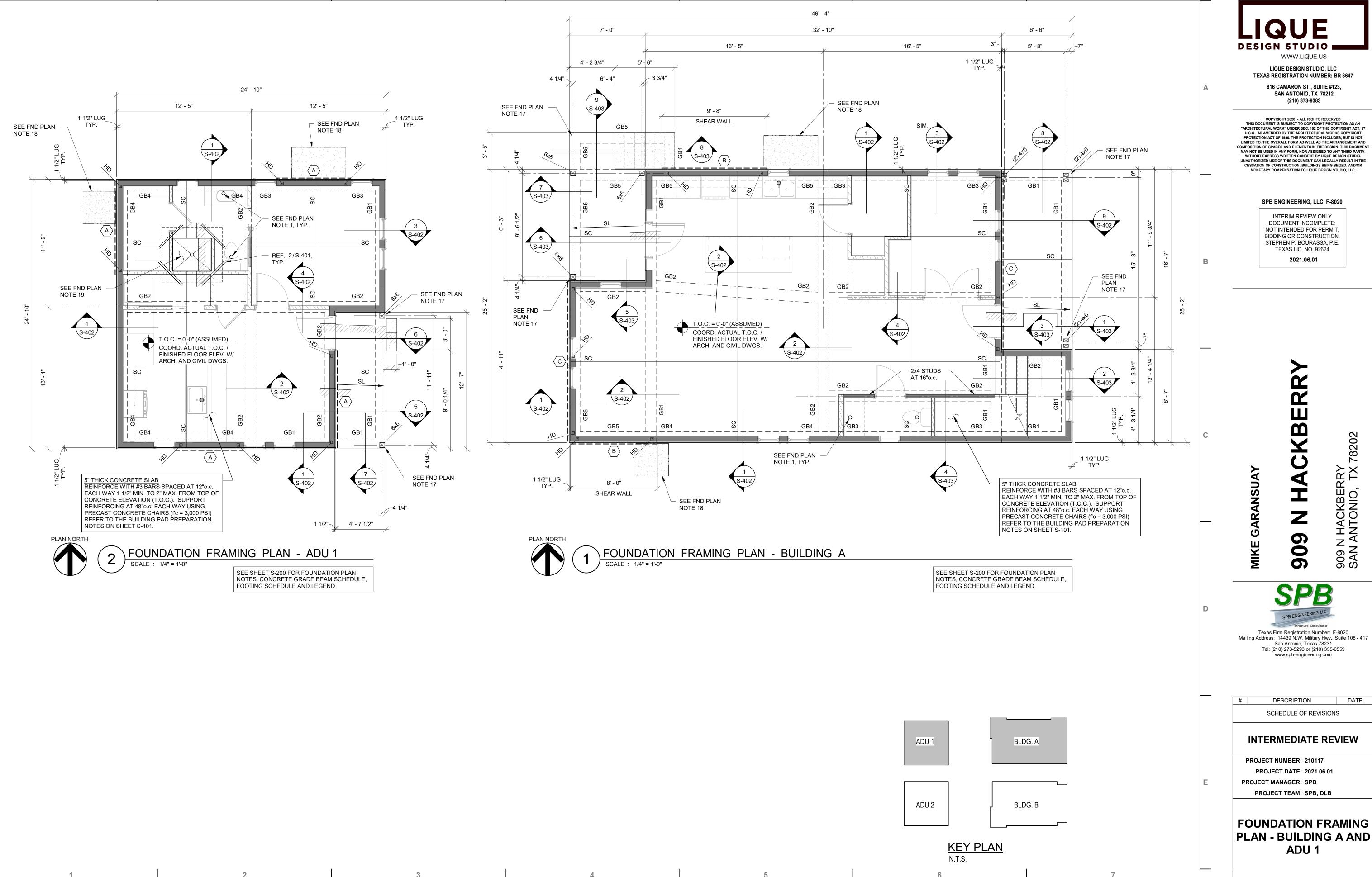
SCHEDULE OF REVISIONS

### INTERMEDIATE REVIEW

PROJECT NUMBER: 210117 PROJECT DATE: 2021.06.01 PROJECT MANAGER: SPB PROJECT TEAM: SPB, DLB

OVERALL FOUNDATION

**PLAN** 



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SCHEDULE OF REVISIONS INTERMEDIATE REVIEW PROJECT NUMBER: 210117 PROJECT DATE: 2021.06.01

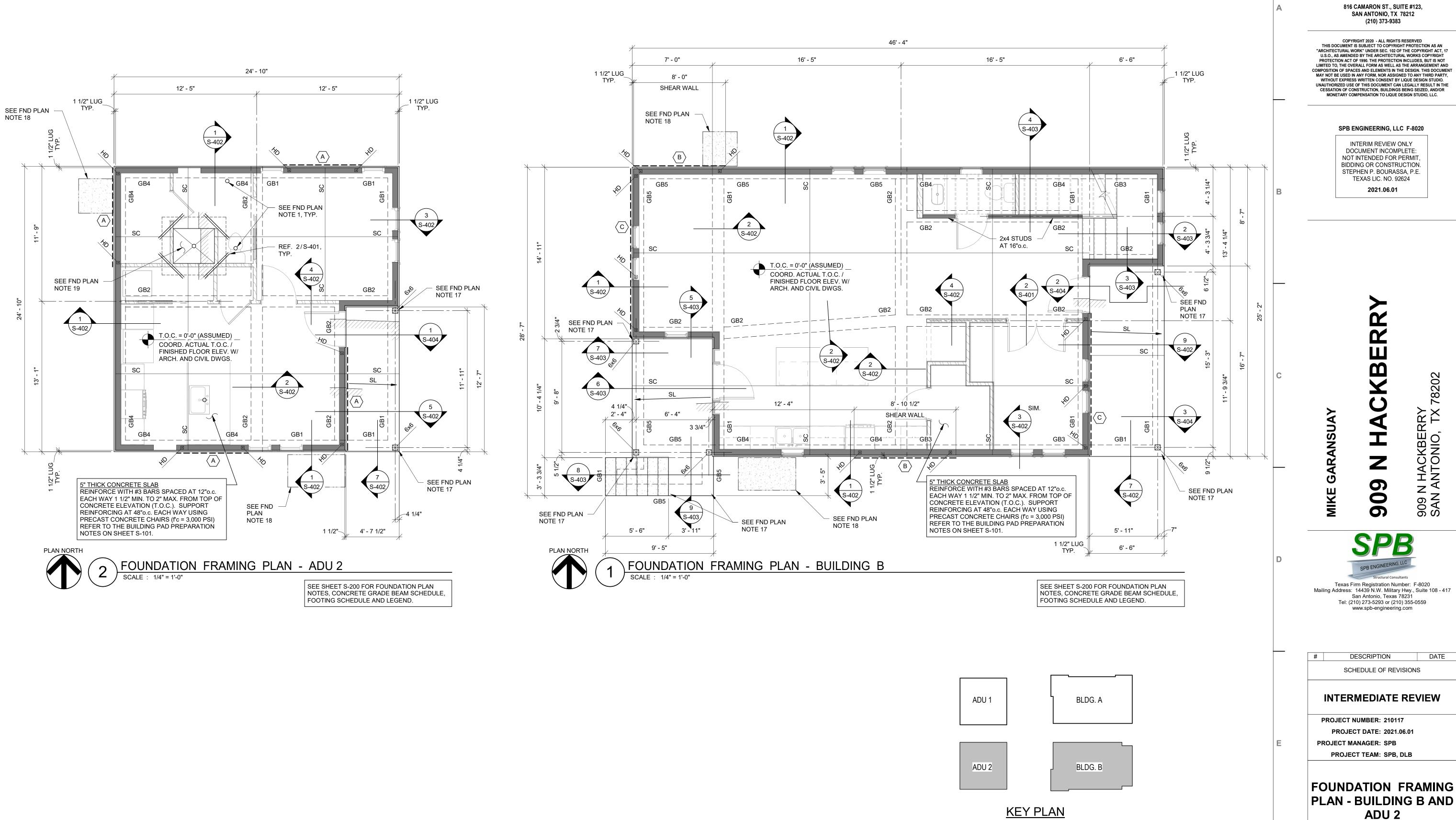
DESCRIPTION

DATE

**FOUNDATION FRAMING PLAN - BUILDING A AND** ADU 1

**S-201** 

(NOT FOR CONSTRUCTION)



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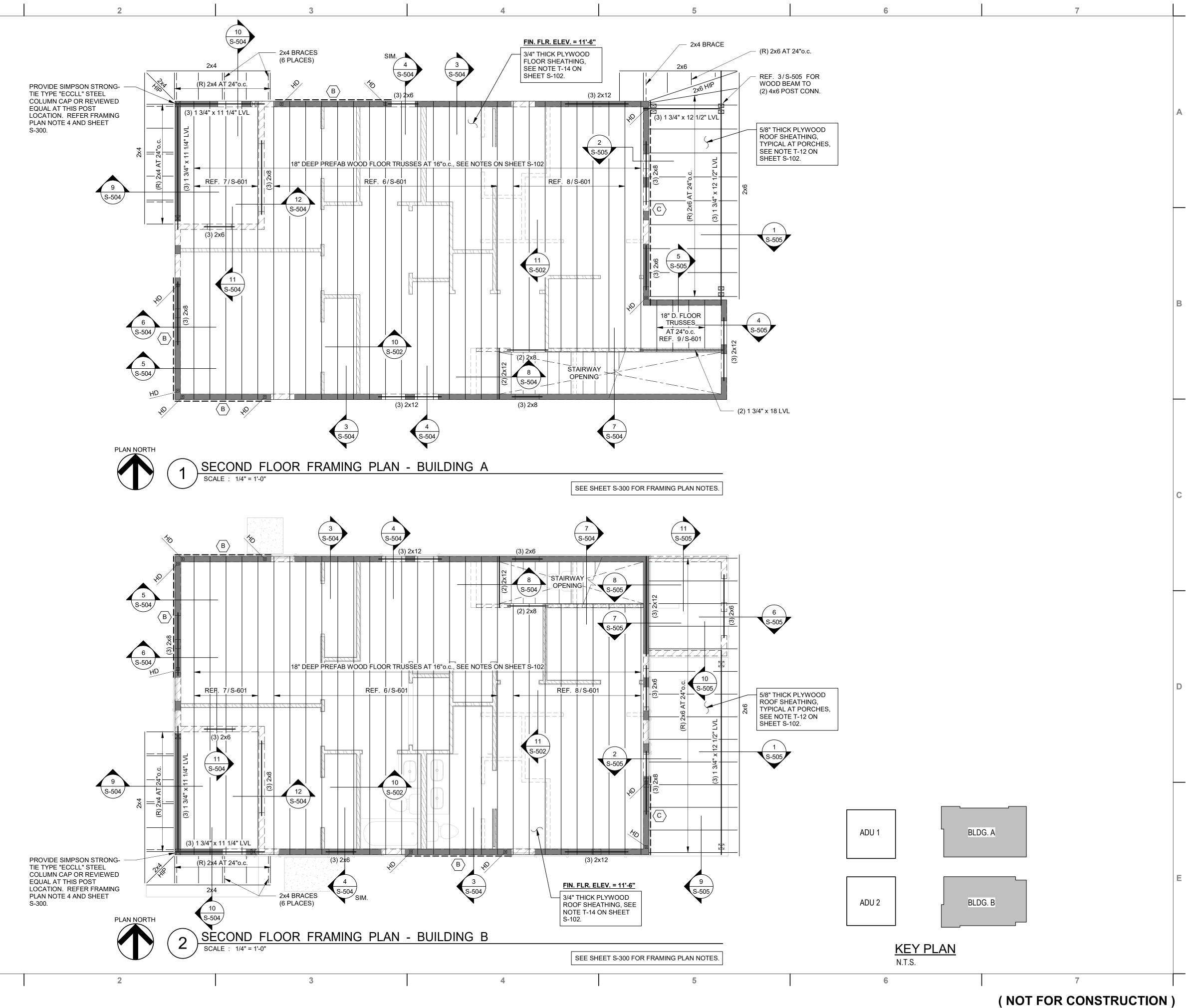
DESCRIPTION DATE SCHEDULE OF REVISIONS INTERMEDIATE REVIEW PROJECT NUMBER: 210117 PROJECT DATE: 2021.06.01

PROJECT MANAGER: SPB

PROJECT TEAM: SPB, DLB FOUNDATION FRAMING

**S-202** 

ADU 2



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2021.06.01

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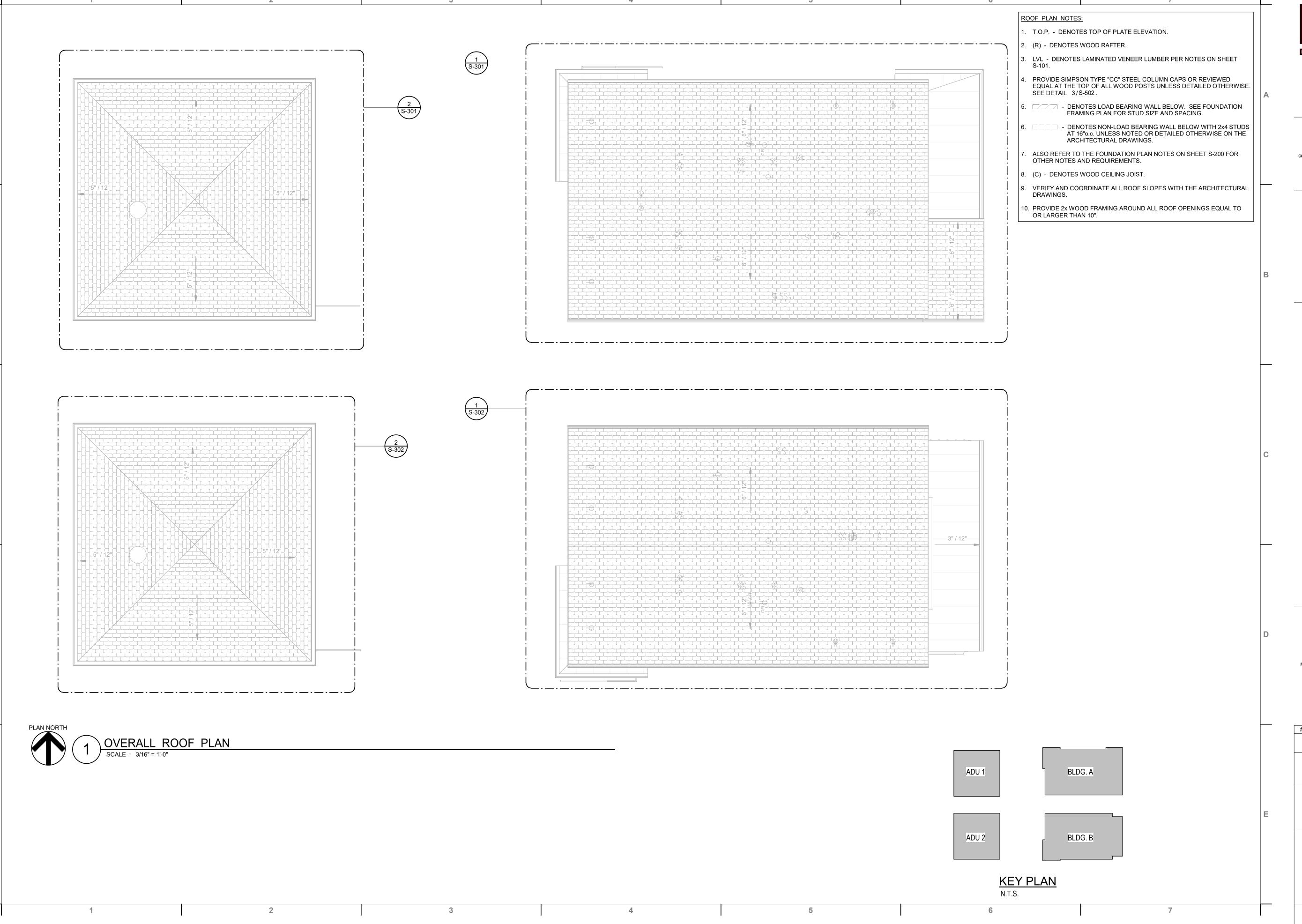
DATE

### INTERWIEDIATE REVI

DESCRIPTION

PROJECT NUMBER: 210117
PROJECT DATE: 2021.06.01
PROJECT MANAGER: SPB
PROJECT TEAM: SPB, DLB

SECOND FLOOR FRAMING PLANS -BUILDING A AND BUILDING B



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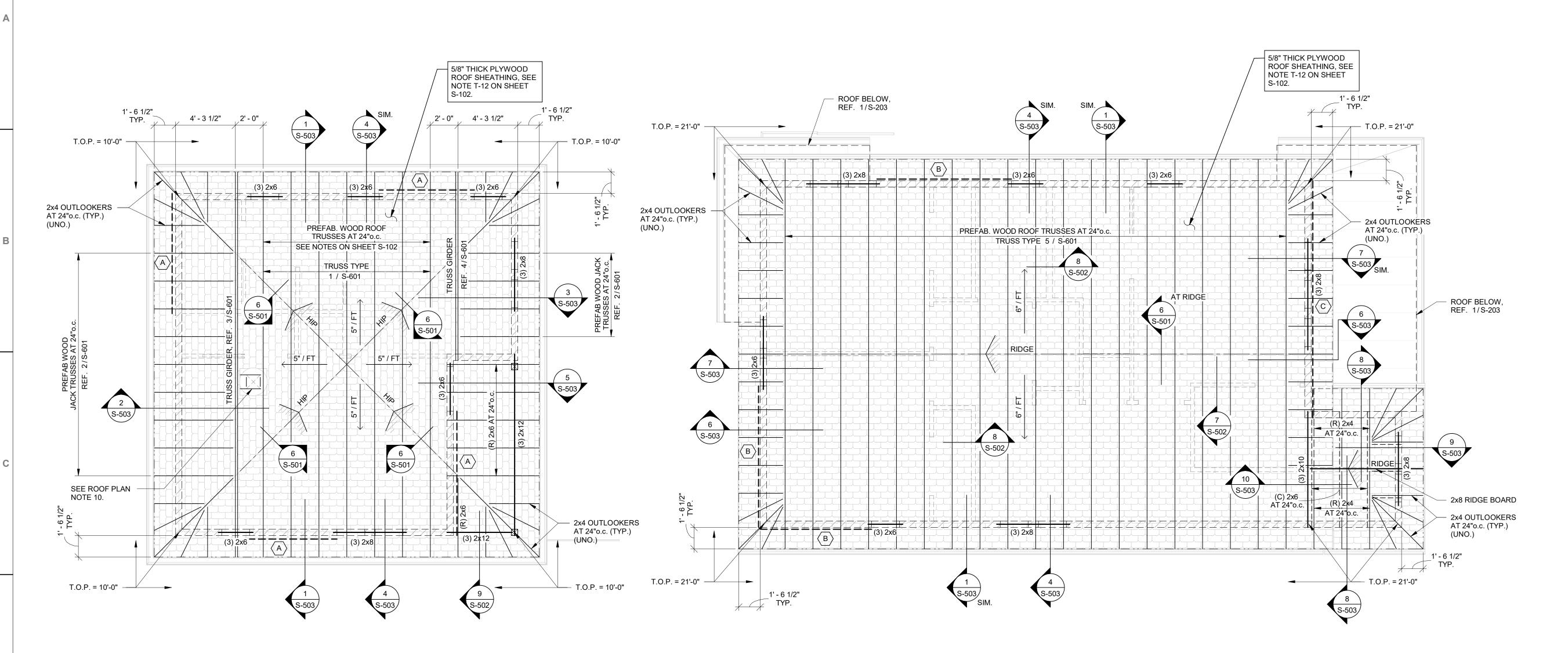
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## INTERMEDIATE REVIEW

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**OVERALL ROOF PLAN** 



PLAN NORTH

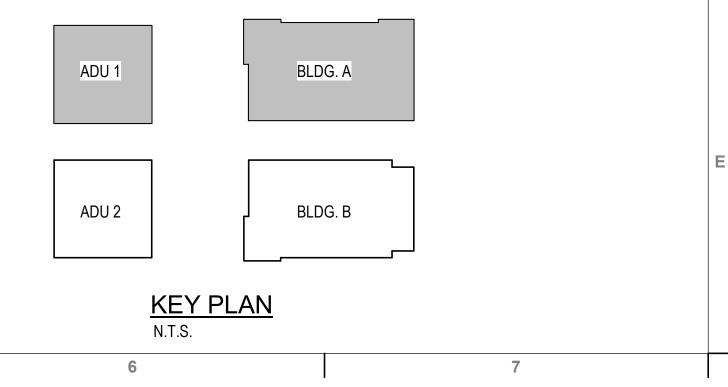
ROOF FRAMING PLAN - ADU 1

SEE SHEET S-300 FOR ROOF PLAN NOTES.



1 ROOF FRAMING PLAN - BUILDING A
SCALE: 1/4" = 1'-0"

SEE SHEET S-300 FOR ROOF PLAN NOTES.



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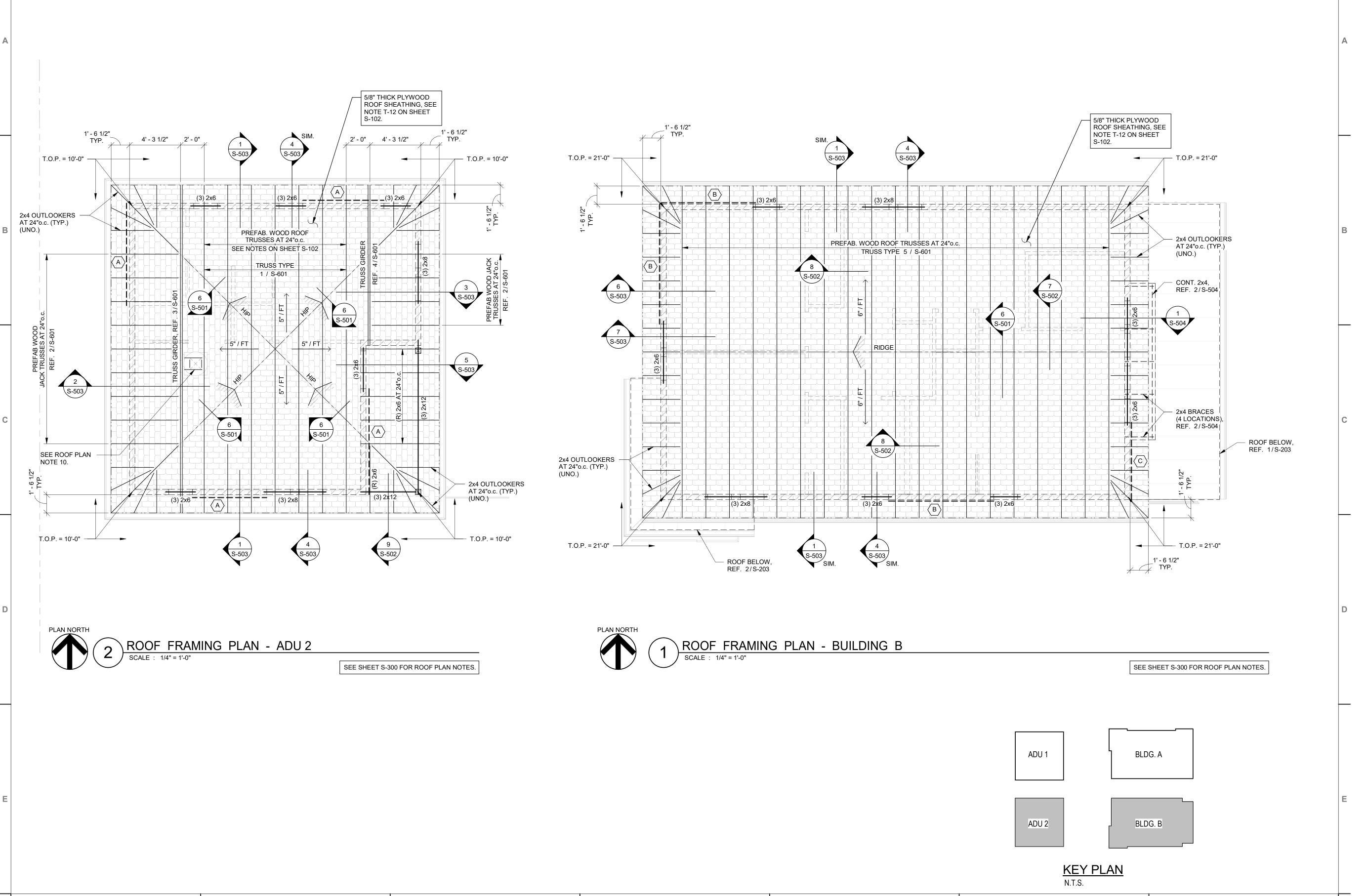
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ROOF FRAMING PLANS -BUILDING A AND ADU 1



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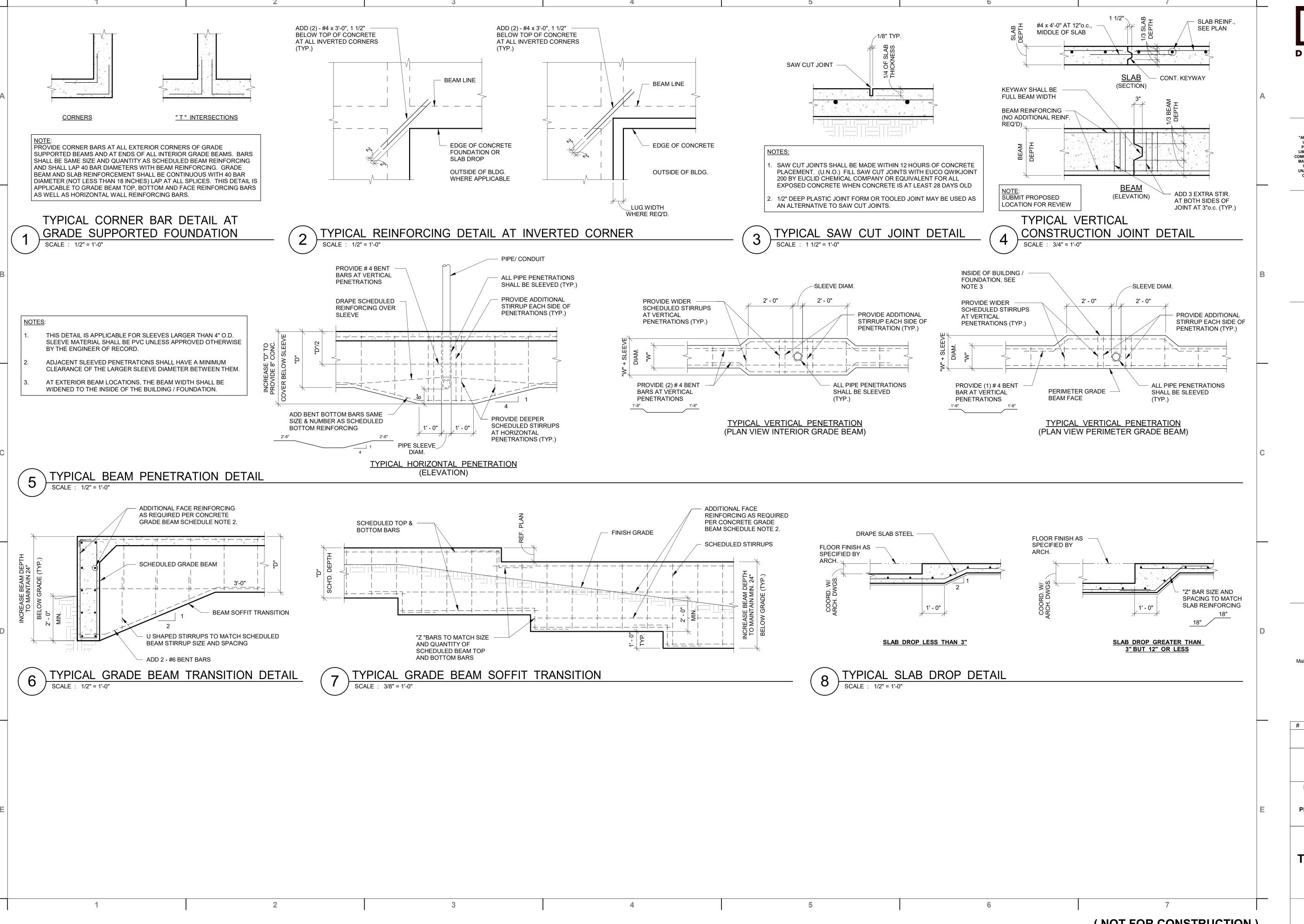
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PROJECT TEAM: SPB, DLB

ROOF FRAMING PLANS -BUILDING B AND ADU 2

**S-302** 

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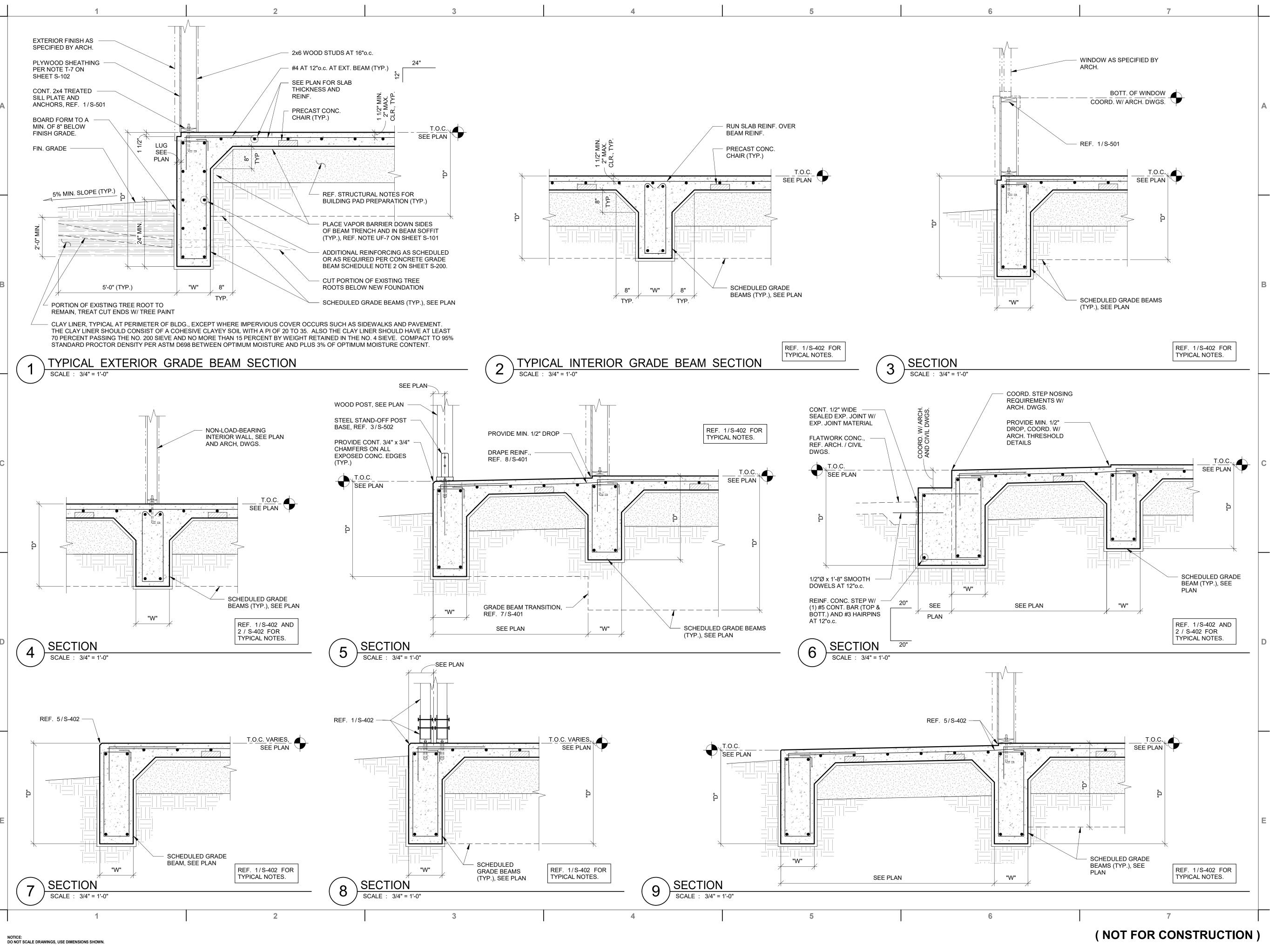
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SCHEDULE OF REVISIONS **INTERMEDIATE REVIEW** PROJECT NUMBER: 210117

DESCRIPTION

PROJECT DATE: 2021.06.01 PROJECT MANAGER: SPB PROJECT TEAM: SPB, DLB

**TYPICAL FOUNDATION DETAILS** 



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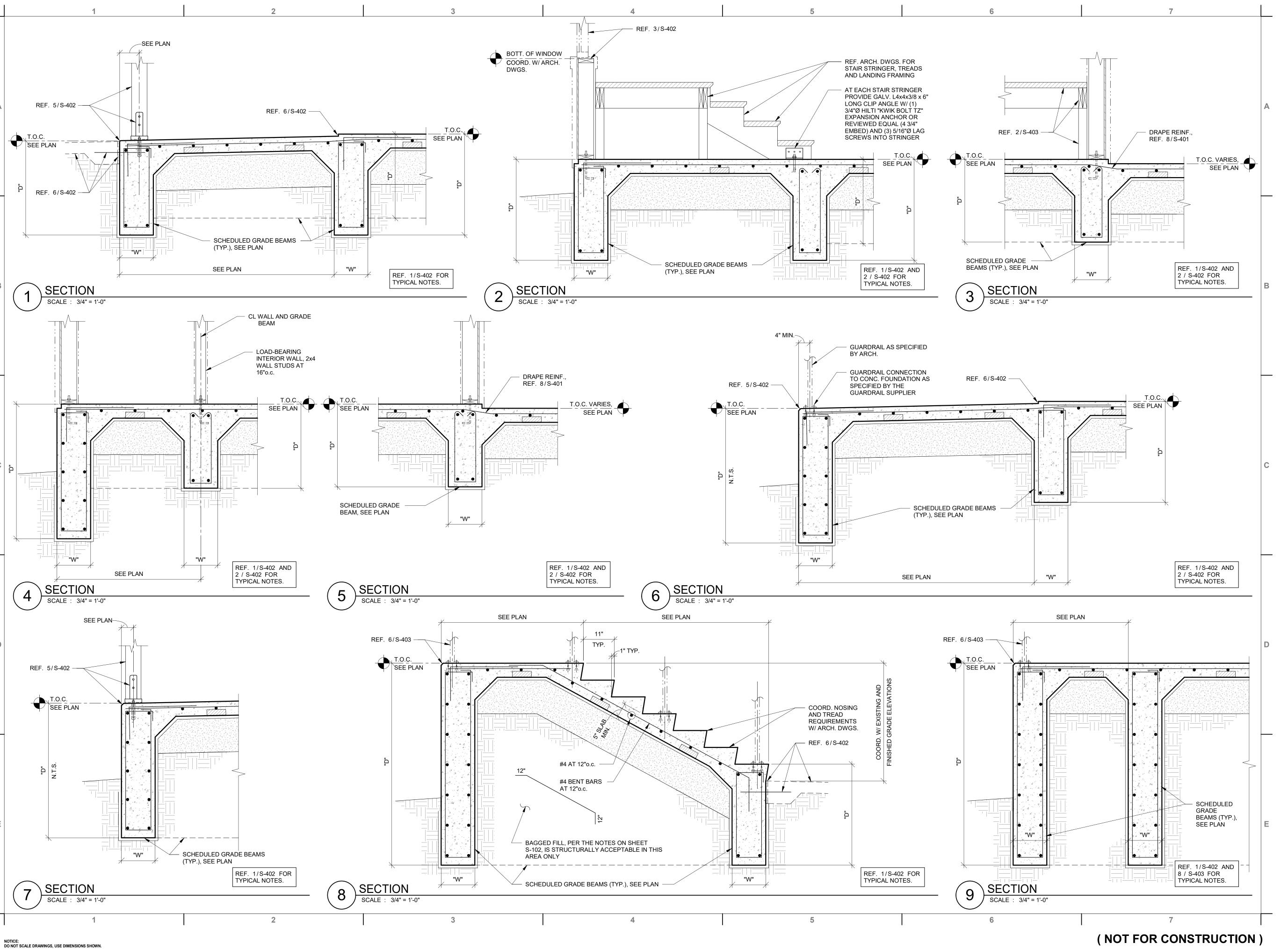
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DATE

DESCRIPTION

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**FOUNDATION SECTIONS AND DETAILS** 



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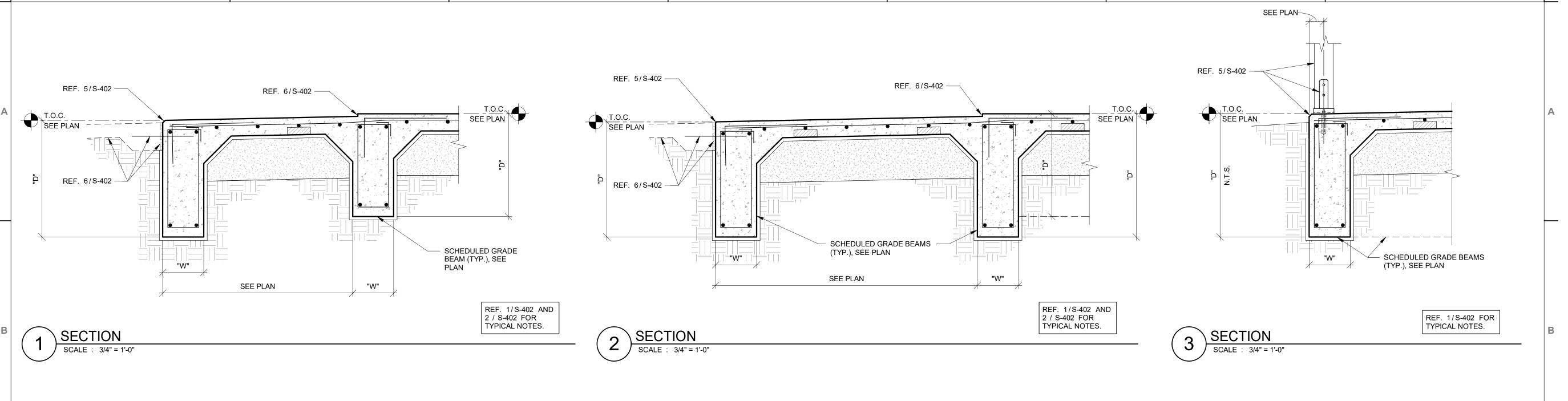
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PROJECT NUMBER: 210117
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PROJECT TEAM: SPB, DLB

DESCRIPTION

FOUNDATION SECTIONS
AND DETAILS





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DESCRIPTION

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PROJECT TEAM: SPB, DLB

FOUNDATION SECTIONS **AND DETAILS** 

MIN. 48" LAP -ATTACH TOGETHER DOUBLE 2X HEADER, SEE PLAN IF W/ 16d NAILS @ 6" o.c. TOP PLATE NOT NOTED. PROVIDE COORD. W/ (3) 2x8 W/ PLYWOOD FILLER. EXISTING \ CRIPPLE STUDS @ SAME SPACING NOTES: (3) 16d NAILS (TYP.) SEE TYP. SHEAR WALL DETAIL FOR REQUIREMENTS AT SHEAR WALLS. HEADER HEADER 2. SEE ARCH'L FOR ROUGH OPENING HEADER, SEE PLAN, IF NOT TRIMMER/ DIMENSIONS & LOCATION OF OPENING. NOTED PROVIDE (3) 2x8 W/ STUD PLYWOOD FILLER. ROUGH SILL, MAY BE DOUBLED WINDOW CRIPPLE STUDS REGULAR STUDS, SEE PLAN MIN. ONE CRIPPLE STUD UNDER HEADER. DØOR IF HEADER SPAN EXCEEDS FOUR FEET NAIL CRIPPLE STUD TO PROVIDE TWO CRIPPLE STUDS. FULL HEIGHT STUD W/ 16d NAILS @ 6" o.c. CRIPPLE STUDS UNDER SILL PLATE (TYP.) TREATED 2X 1/2"Ø J RODS AT 48" o.c. OR 1/2"Ø SILL PLATE HILTI "KWIK BOLT TZ" EXPANSION T.O.C., COORD. W/ ANCHORS (3 1/4" EMBED) OR REVIEWED EQUAL AT 48" o.c. (TYP.)

3 1/2" ASSEMBLY A **ASSEMBLY B** ASSEMBLY C ASSEMBLY D **ASSEMBLY E** "W" = 1 1/2" OR 1 3/4" (4 PCS) 3 1/2" (1 PCS) (3 PCS) (2 PCS) (4 PCS) AND 3 1/2" OR 2 11/16" (1 PCS) 2 ROWS OF 16d (2) - 2 ROWS OF 16d 2 ROWS OF 1/2" DIA. (2) - 2 ROWS OF 16d 2 ROWS OF 1/2" DIA. @ 12" o.c. @ 12" o.c. @ 12" o.c. @ 12" o.c. @ 12" o.c.

TYPICAL FRAMED OPENING DETAIL

ON THE WOOD SHEAR

SEE WOOD SHEAR WALL

SCHEDULE FOR SIMPSON

STRONG-TIE HOLDOWNS

ANCHORS LOCATED AT EACH END OF THE SHEAR

HOLE OR NOTCH THRU STUDS

AND FOUNDATION

WALL SCHEDULE

SEE PLANS FOR MINIMUM SHEAR WALL LENGTHS, SHEAR WALLS MAY BE PERFORATED SEE SCHEDULE AND 12 / S-505

SCHEDULED HOLDDOWN AND THREADED ROD AT SECOND FLOOR WHERE APPLICABLE SOLID BLOCKING **BETWEEN ALL** FLOOR JOIST OR TRUSS, FRAMING. REF. PLAN PROVIDE SOLID WOOD BLOCKING BOTH SIDES OF THREADED ROD ROOF RAFTER OR TRUSSES, REF. PLAN SEE WOOD SHEAR WALL DOUBLE TOP PLATE SCHEDULE FOR NUMBER OF STUDS REQ'D. ATTACH SIMPSON STRONG TIE LTP4: THE STUDS TOGETHER W/ 16"o.c. AT SHEAR WALL A AND B **∟=**=⊨=∃ 16d NAILS AT 6"o.c. 12"o.c. AT SHEAR WALL C STAGGERED 2x WALL BLOCKING AT 48"o.c. MIN. TO BE LOCATED AT SHEATHING EDGES (SEE SEE NOTES 1 AND NOTE 3 NOTE 2 ON THE WOOD

> SHEAR WALL SCHEDULE). 1/2" DIA. J-RODS, SEE SCHEDULE FOR SPACING

> > AND NOTE 7 FOR POST-INSTALLED ANCHOR OPTION.

SEE PLAN FOR SHEAR WALL LOCATIONS.

HOLE OR NOTCH THRU PLATES

SCALE : 3/4" = 1'-0"

TYPICAL MULTI-MEMBER BEAM ASSEMBLY DETAIL

WOOD SHEAR WALL SCHEDULE								
	SHEAR WALL		SILL PLATE RODS	HOLDDOWNS	NUMBER		055	
MARK	SHEATHING (SEE NOTE 1)	& SPACING (SEE NOTE 3)	AND SPACING (SEE NOTE 7)	EA. END (SEE NOTE 4 & NOTE 5)	OF STUDS EA. END	CAST-IN-PLACE (SEE NOTE 6)	OPTIONAL DRILL & EPOXY (SEE NOTE 6)	SEE DETAIL
(A)	15/32" PLYWD. ONE SIDE	10d COMMON AT 6"o.c.	1/2" DIA. x 7" J-RODS AT 24"o.c.	HTT4	2	5/8" DIA. ROD SSTB16	5/8" DIA. W/ 8" MIN. EMBED. AND SET USING SIMPSON SET-XP EPOXY	
⟨B⟩	15/32" PLYWD. ONE SIDE	10d COMMON AT 6"o.c.	1/2" DIA. x 7" J-RODS AT 16"o.c.	HTT5	2	5/8" DIA. ROD SSTB20	5/8" DIA. W/ 10" MIN. EMBED. AND SET USING SIMPSON SET-XP EPOXY	
<u>(C)</u>	15/32" PLYWD. ONE SIDE	10d COMMON AT 2"o.c.	1/2" DIA. x 7" J-RODS AT 16"o.c.	HTT5KT	3	5/8" DIA. ROD SSTB24	5/8" DIA. W/ 12" MIN. EMBED. AND SET USING SIMPSON SET-XP EPOXY	12 / S-505 FOR STRAPS

- 1. ALL SHEATHING SHALL BE APA RATED STRUCTURAL I SHEATHING. IF SHEATHING IS TO BE APPLIED TO THE OUTSIDE OF AN EXTERIOR WALL, THE SHEATHING SHALL BE STRUCTURAL SHEATHING, EXTERIOR EXPOSURE 1 GRADE PLYWOOD.
- 2. ALL SHEATHING EDGES SHALL BE SUPPORTED BY 2 INCH NOMINAL OR WIDER BLOCKING, SEE DETAIL FOR BLOCKING LOCATIONS.
- 3. SCHEDULED NAIL SIZE AND SPACING TO BE PROVIDED AT ALL EDGES OF PLYWOOD SHEATHING INCLUDING THE TOP AND SILL PLATES. ATTACH THE SHEATHING TO ALL OTHER MEMBERS (IN FIELD) WITH SCHEDULED NAILS AT 12" o.c.
- 4. HOLDDOWNS TO BE MANUFACTURED BY "SIMPSON STRONG TIE" OR A REVIEWED EQUIVALENT.
- 5. FASTEN HOLDDOWNS TO THE NUMBER OF STUDS SHOWN IN THE SCHEDULE AND PER MANUFACTURER'S RECOMMENDATIONS. WHERE SHEAR WALL ENDS ABUT WOOD POSTS, ATTACH SCHEDULED SIMPSON STRONG-TIE HOLDDOWN TO WOOD POST.
- 6. FOUNDATION ANCHORS TO BE MANUFACTURED BY "SIMPSON STRONG TIE" OR A REVIEWED EQUIVALENT (CAST-IN-PLACE AND DRILLED AND EPOXY ANCHORS SHALL BE INSTALLED PER THE MANUFACTURER'S RECOMMENDATIONS). AS AN ALTERNATIVE, ANCHORS CAN BE INSTALLED AFTER THE CONCRETE HAS BEEN POURED. SEE SCHEDULE FOR THE DRILL AND EPOXY ALTERNATIVE.
- 7. SILL PLATE POST-INSTALLED ANCHOR OPTION: 1/2" DIA. WITH 6" MIN. EMBEDMENT AND SET USING SIMPSON SET-XP EPOXY. SPACING AS INDICATED IN SCHEDULE ABOVE.

TYPICAL WOOD SHEAR WALL DETAIL, SCHEDULE AND NOTES SCALE : 3/8" = 1'-0"

SIMPSON RPS22 **BOLTS AS** ∠ 25% W FOR LOAD BEARING STUDS STRAP EACH SIDE REQUIRED, SEE 40%W FOR NON-LOAD BEARING STUDS OF TOP PLATE NOTE BELOW CENTERED ON SIMPSON RPS22 HOLE. STRAP CENTERED ON NOTCH. NOTES: HOLE SHALL BE DRILLED SO AS TO NOT TO SPLINTER THE STUD. SIMPSON RPS22 W - DENOTES WIDTH OF MEMBER. STRAP TIE ONE SIDE \_\_\_W/3 MAX. 3/8" MIN. \_\_\_\_\_\_3/8" MIN. 3/8" MIN. \_\_\_\_3/8" MIN. PLAN-SILL PLATE PLAN-DBL. TOP PLATE NOTCH ON PLATE - 40% W MAX. FOR LOAD-BEARING STUDS. FOR "D" GREATER THAN 1/3W FOR "D" GREATER THAN 1/3W PROVIDE ANCHOR BOLTS AT PROVIDE STRIP AT EACH SIDE <u>\_\_"W"</u> 60% W MAX. FOR NON-LOAD BEARING. 12" MAXIMUM AT EACH SIDE AS SHOWN - 2D/3 MAX. OF THE HOLE.

2

ROOF SYSTEM, 8d NAILS AT 6"o.c. REF. ARCH. FROM SHEATHING TO BLOCKING ROOF SHEATHING, SEE TIMBER FRAMING NOTE T-12 SEE PLAN ON SHEET S-102 WOOD RAFTER OR CONT. RIDGE BOARD AT PREFAB WOOD RAFTERS OR BLOCKING TRUSS, REF. PLAN BETWEEN PREFAB WOOD TRUSSES, REF. PLAN NOTE: SIMILAR AT HIP CONDITION.

TYPICAL SECTION AT RIDGE SCALE : 1 1/2" = 1'-0"

ROOF SYSTEM, 8d NAILS AT 6"o.c. REF. ARCH. FROM SHEATHING TO BLOCKING ROOF SHEATHING, -SEE TIMBER FRAMING NOTE T-12 ON SHEET SEE PLAN S-102 PREFABRICATED WOOD TRUSS TOP CHORD \ OR RAFTER, SEE PLAN 2x WOOD BLOCKING WOOD 2x OVERBUILD NAILED BETWEEN OR RAFTER BEYOND TRUSSES AND RAFTERS

TYPICAL SECTION AT VALLEY SCALE : 1 1/2" = 1'-0"

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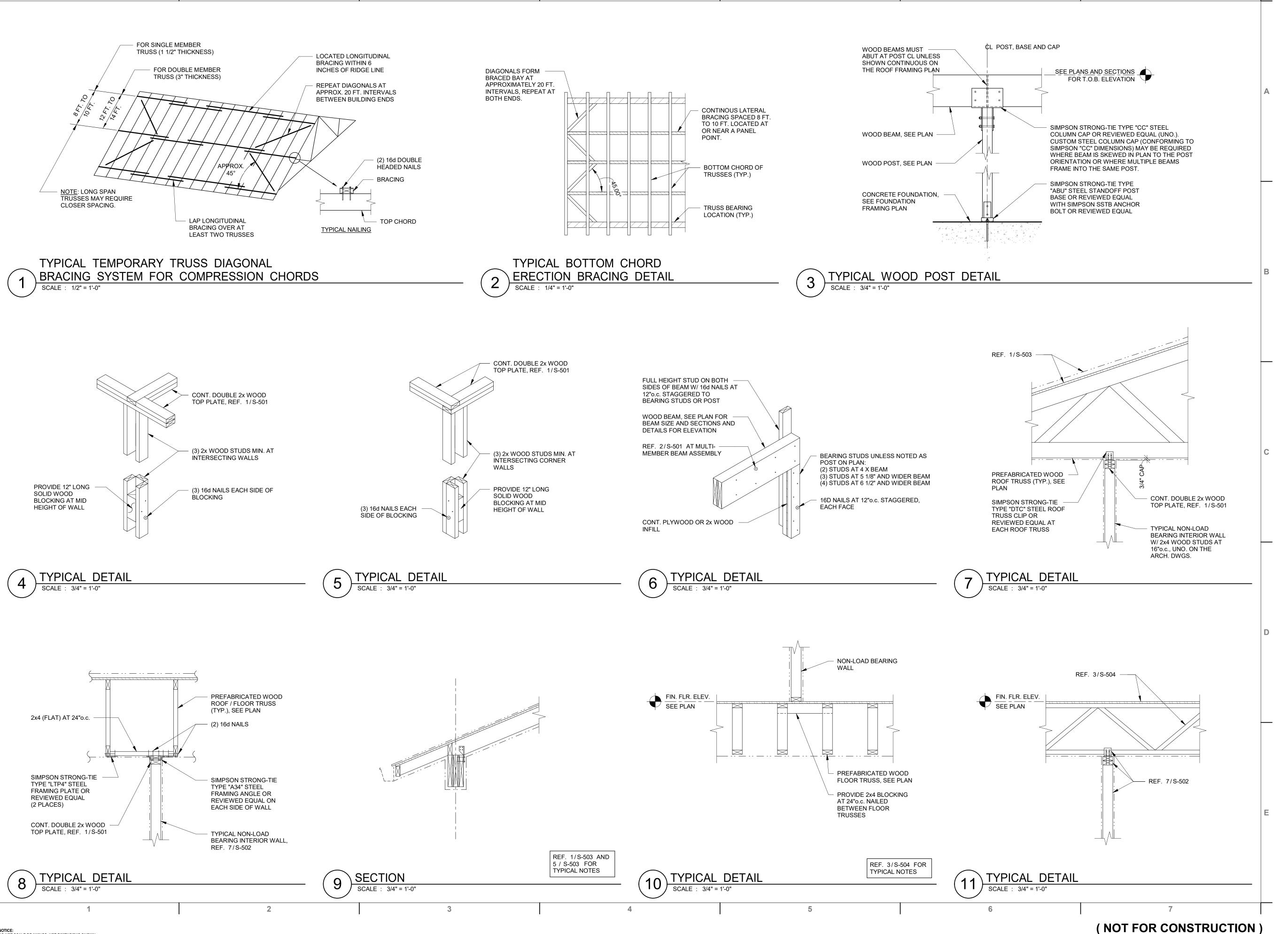
PROJECT TEAM: SPB, DLB

**TYPICAL WOOD** FRAMING DETAILS

S-501

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2021.06.01

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Texas Firm Registration Number: F-8020 Mailing Address: 14439 N.W. Military Hwy., Suite 108 - 417 San Antonio, Texas 78231 Tel: (210) 273-5293 or (210) 355-0559

www.spb-engineering.com

INTERMEDIATE REVIEW

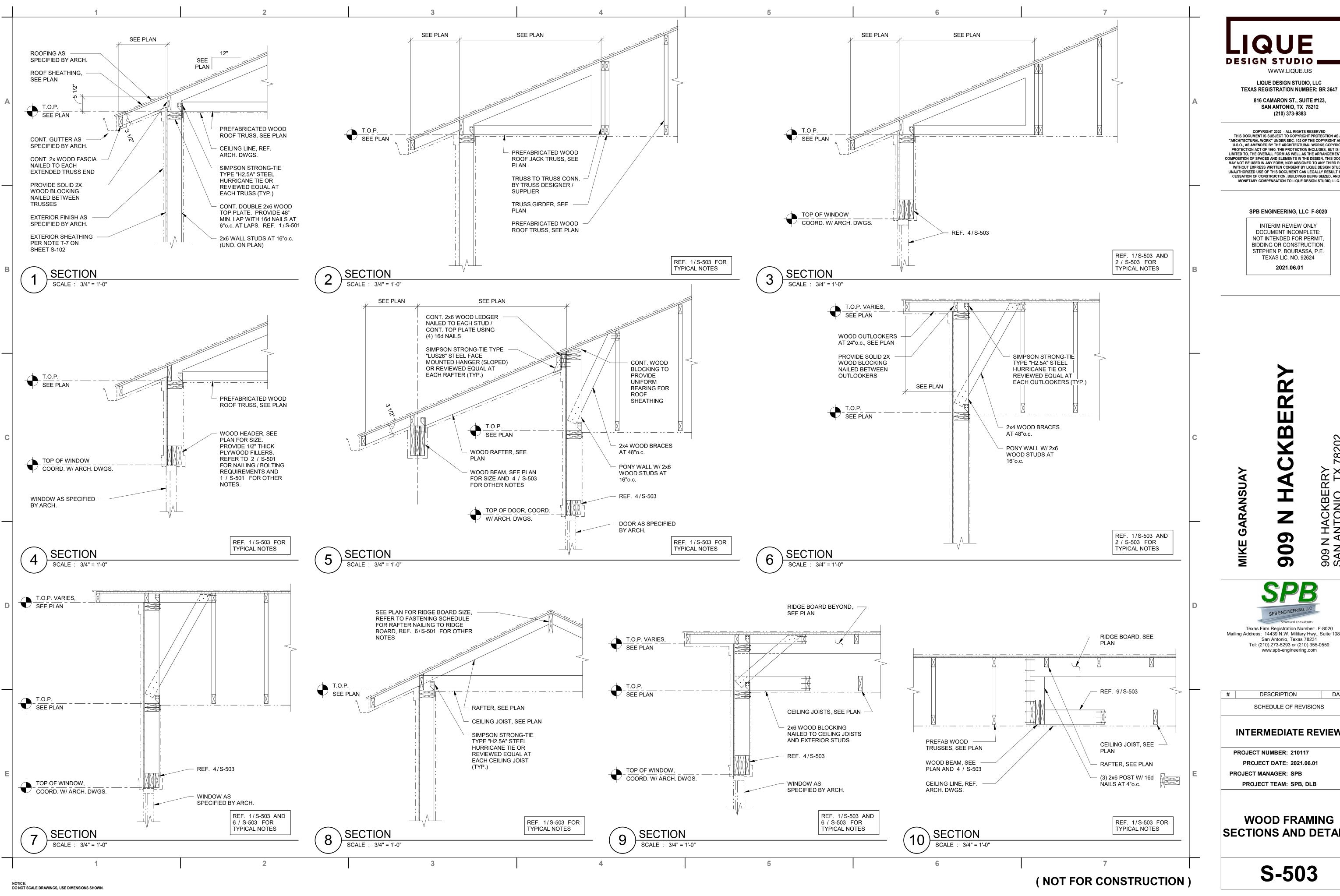
DESCRIPTION

SCHEDULE OF REVISIONS

DATE

PROJECT NUMBER: 210117 PROJECT DATE: 2021.06.01 PROJECT MANAGER: SPB PROJECT TEAM: SPB, DLB

**TYPICAL WOOD** FRAMING DETAILS



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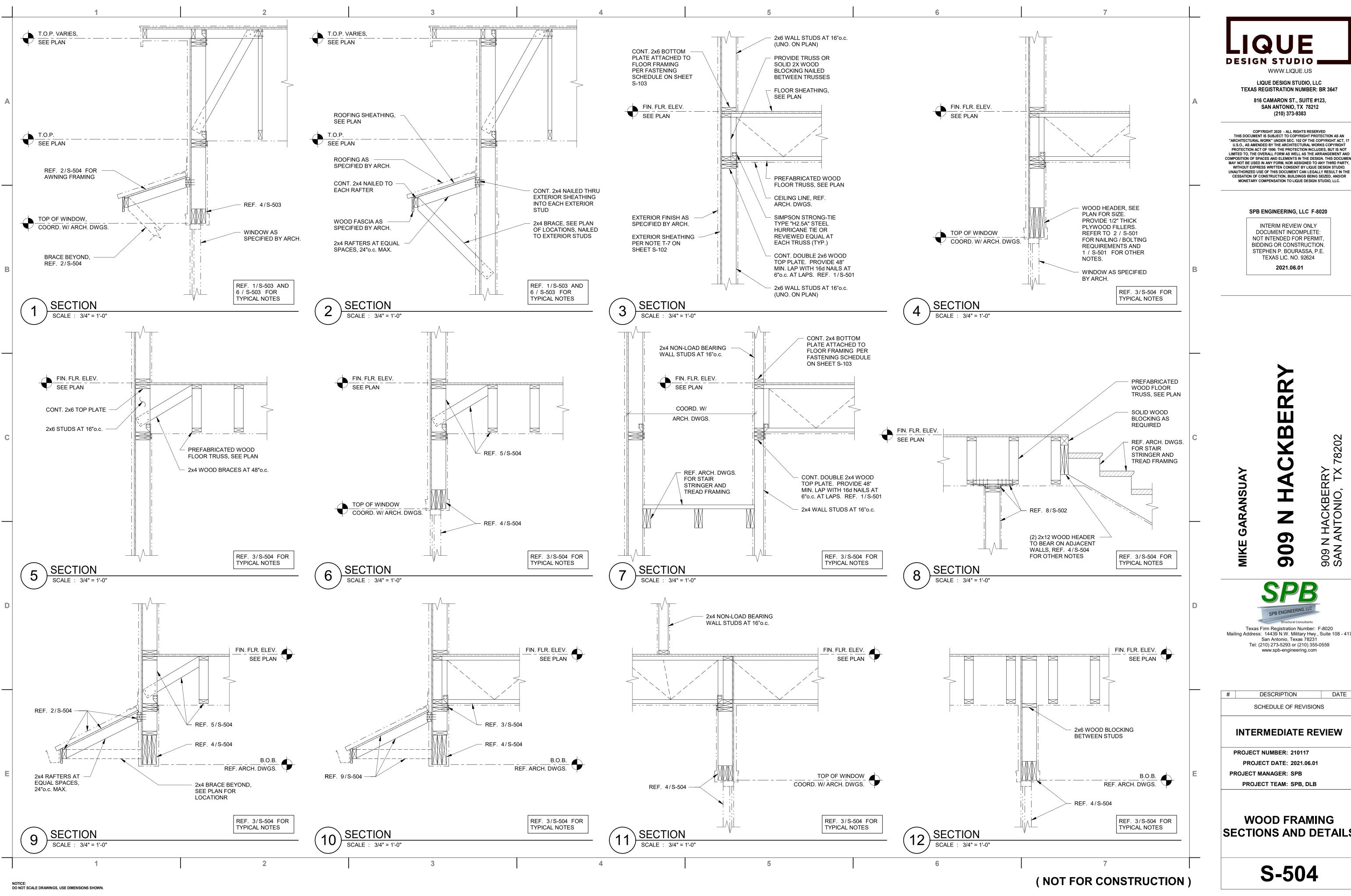
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INTERMEDIATE REVIEW

DATE

PROJECT DATE: 2021.06.01 PROJECT MANAGER: SPB PROJECT TEAM: SPB, DLB

**WOOD FRAMING SECTIONS AND DETAILS** 



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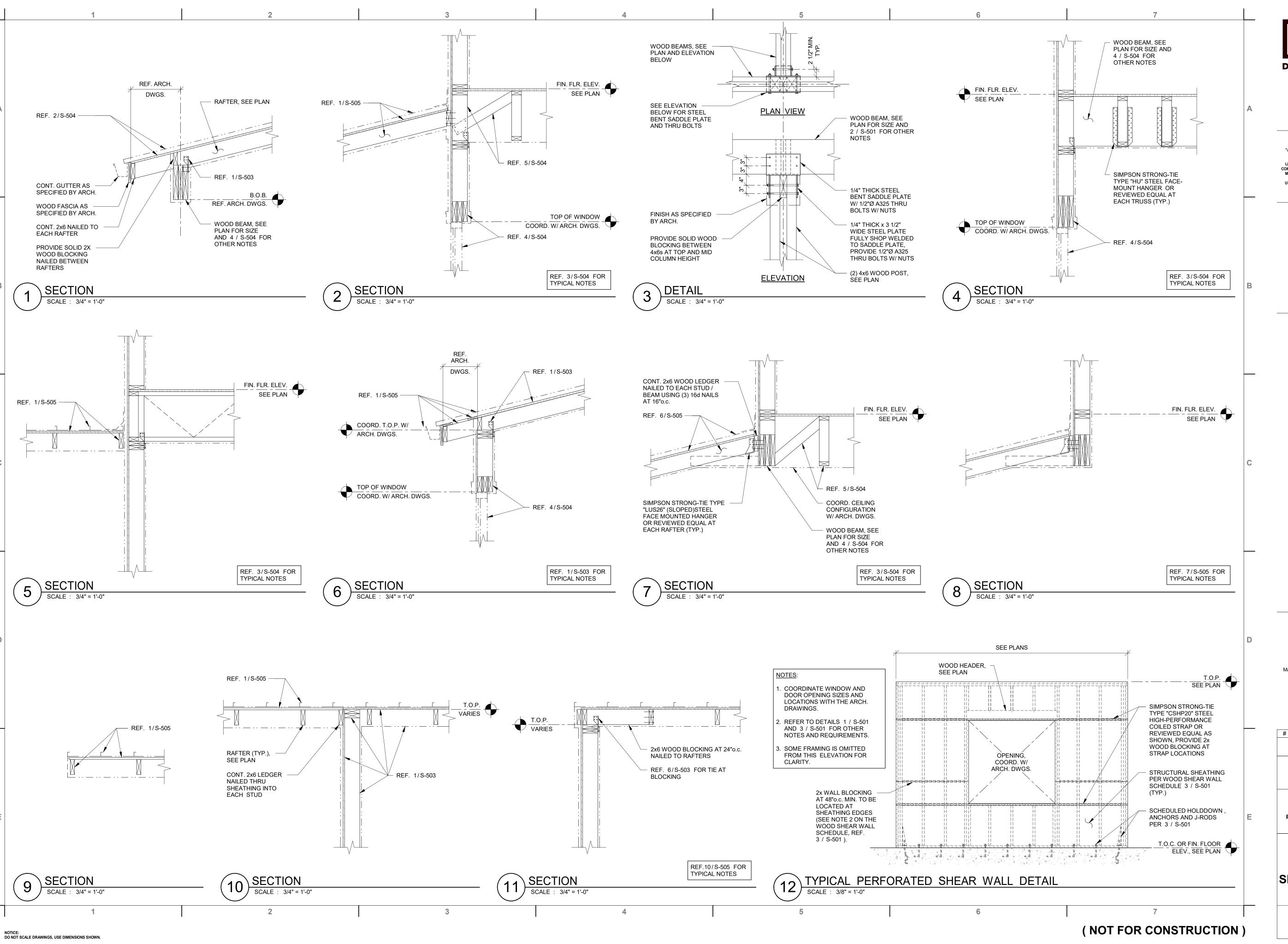
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**INTERMEDIATE REVIEW** 

DATE

PROJECT DATE: 2021.06.01 PROJECT MANAGER: SPB PROJECT TEAM: SPB, DLB

**WOOD FRAMING SECTIONS AND DETAILS** 



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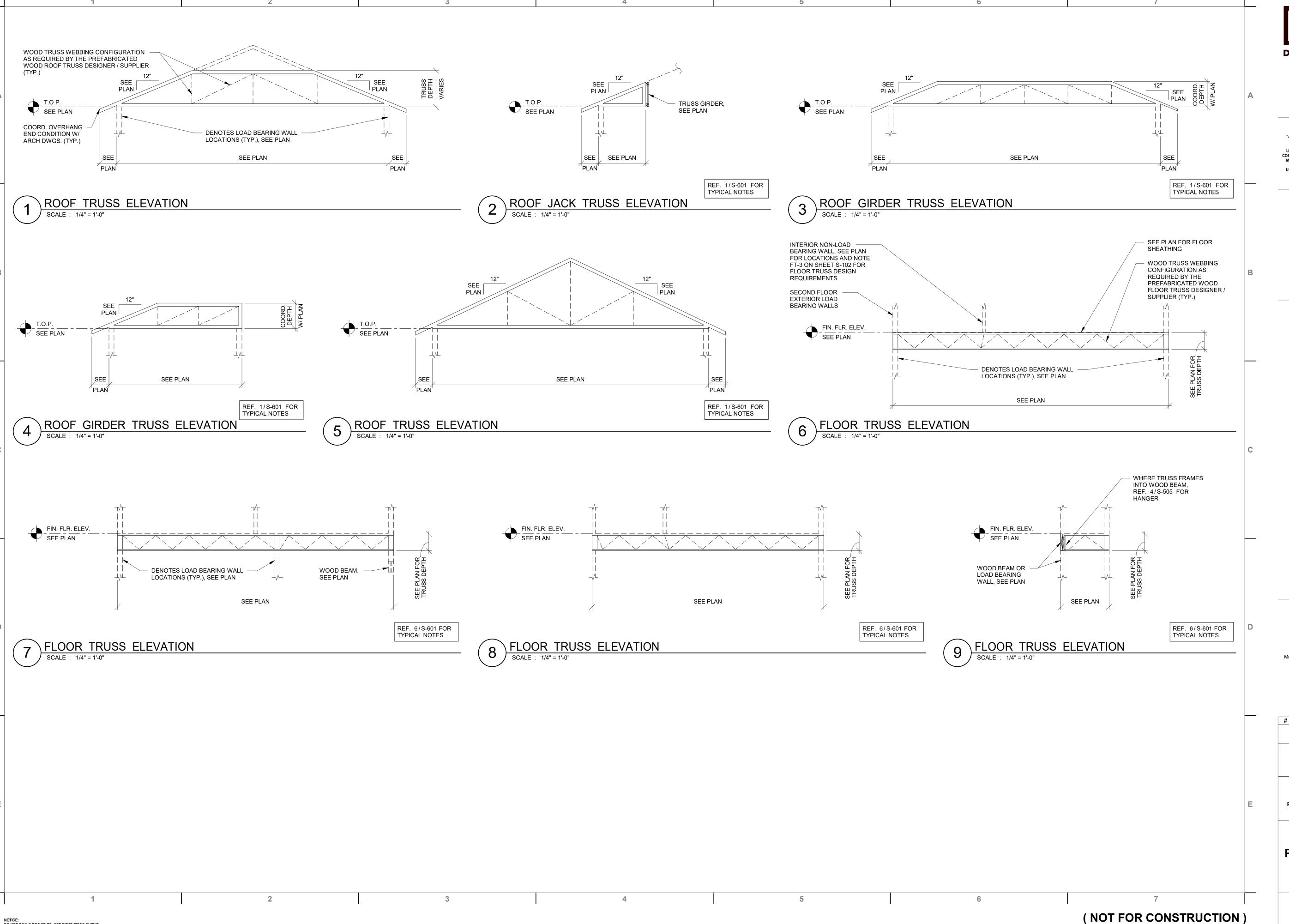
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> DESCRIPTION DATE SCHEDULE OF REVISIONS

### INTERMEDIATE REVIEW

PROJECT NUMBER: 210117 PROJECT DATE: 2021.06.01 PROJECT MANAGER: SPB PROJECT TEAM: SPB, DLB

**WOOD FRAMING SECTIONS AND DETAILS** 



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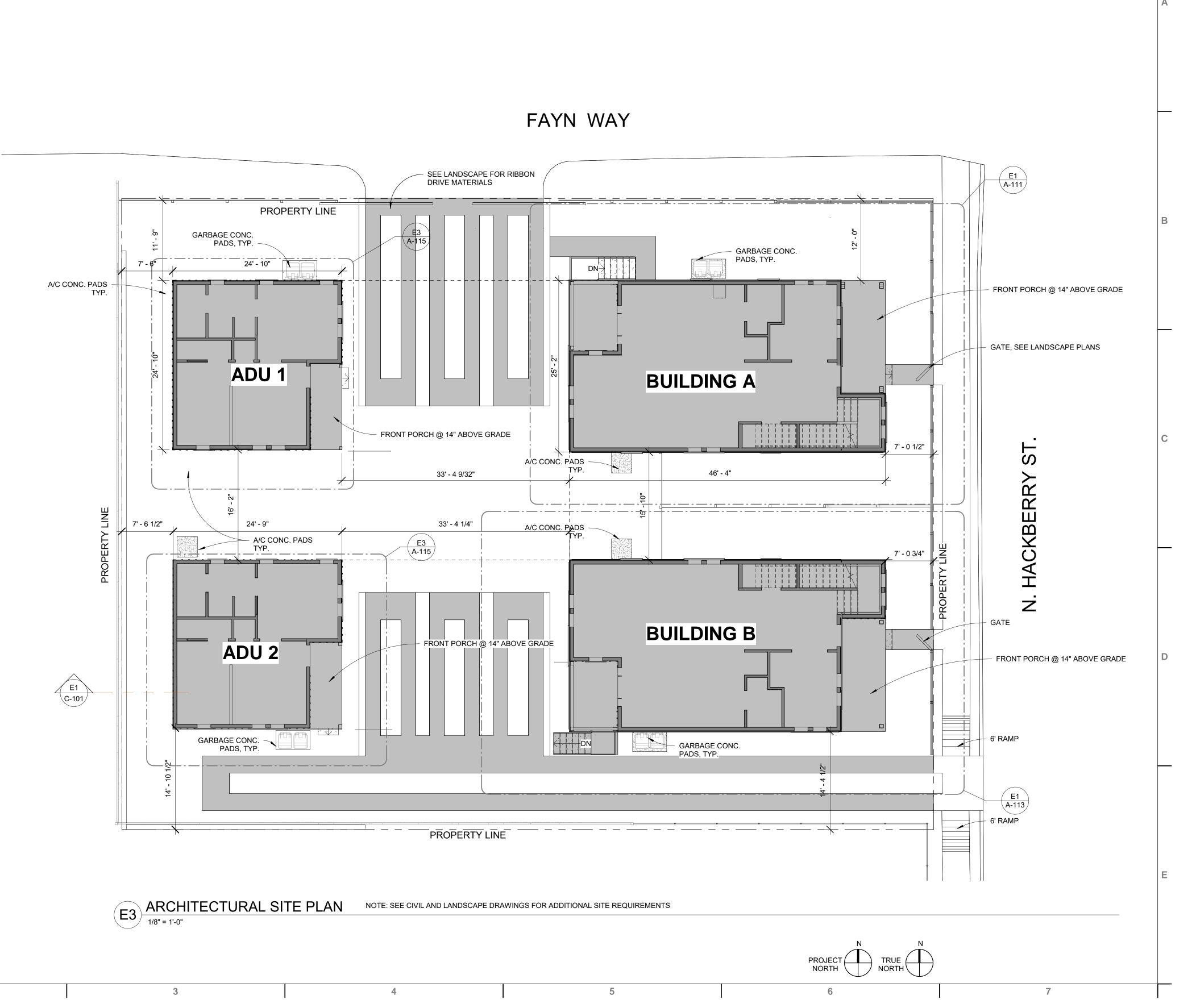
DATE DESCRIPTION SCHEDULE OF REVISIONS

**INTERMEDIATE REVIEW** 

PROJECT NUMBER: 210117 PROJECT DATE: 2021.06.01 PROJECT MANAGER: SPB PROJECT TEAM: SPB, DLB

PREFAB WOOD TRUSS **ELEVATIONS** 

**S-601** 



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1 ISSUED FOR HISTORICAL BOARD 2021.06.29 REVIEW
A INTERMEDIATE CONSTRUCTION DOCUMENTS
# DESCRIPTION DATE

## CONSTRUCTION DOCUMENTS HISTORICAL BOARD REVIEW

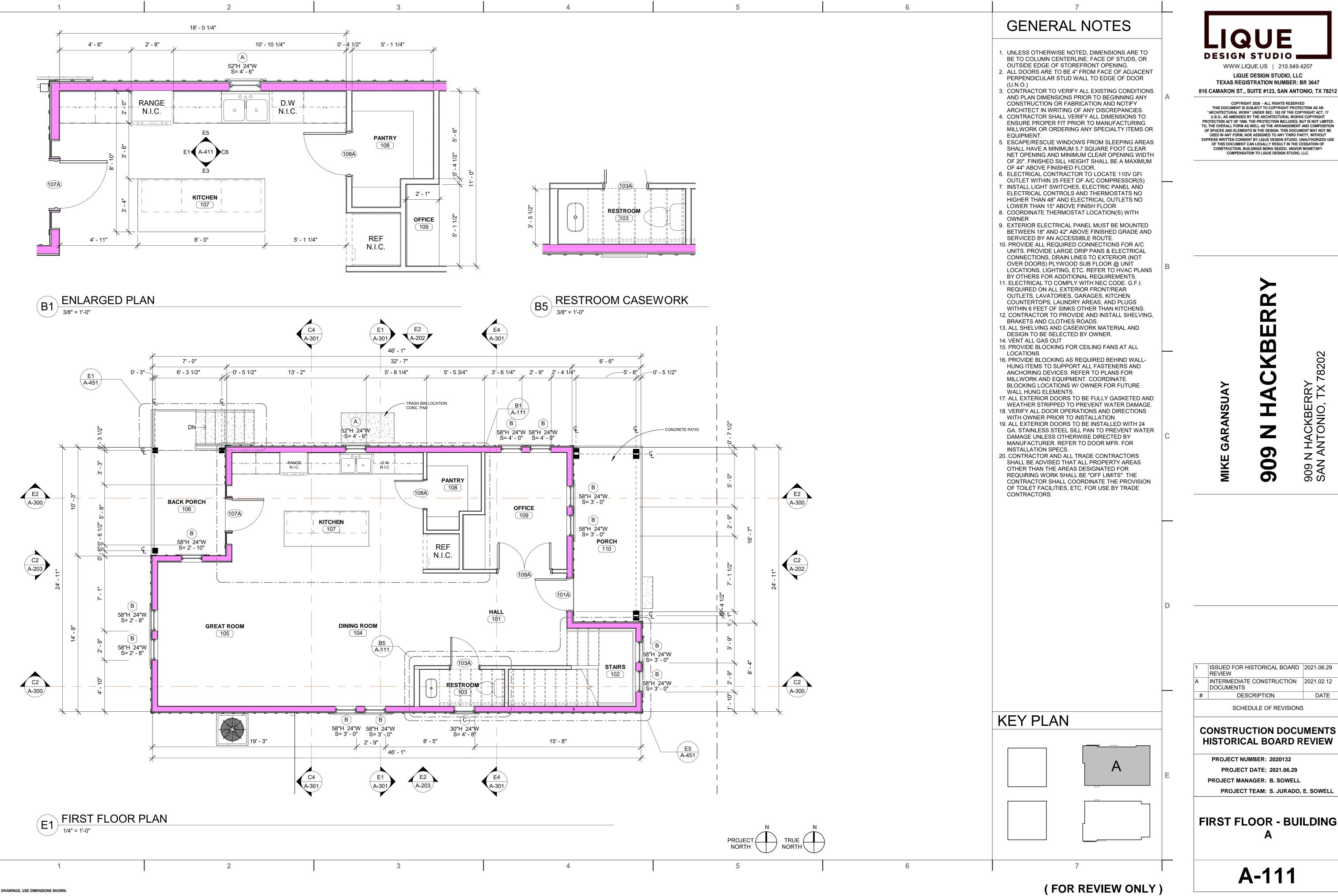
SCHEDULE OF REVISIONS

PROJECT NUMBER: 2020132
PROJECT DATE: 2021.06.29
PROJECT MANAGER: B. SOWELL

PROJECT TEAM: S. JURADO, E. SOWELL

ARCHITECTURAL SITE PLAN

A-051



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ISSUED FOR HISTORICAL BOARD | 2021.06.29 INTERMEDIATE CONSTRUCTION 2021.02.12

SCHEDULE OF REVISIONS

DESCRIPTION

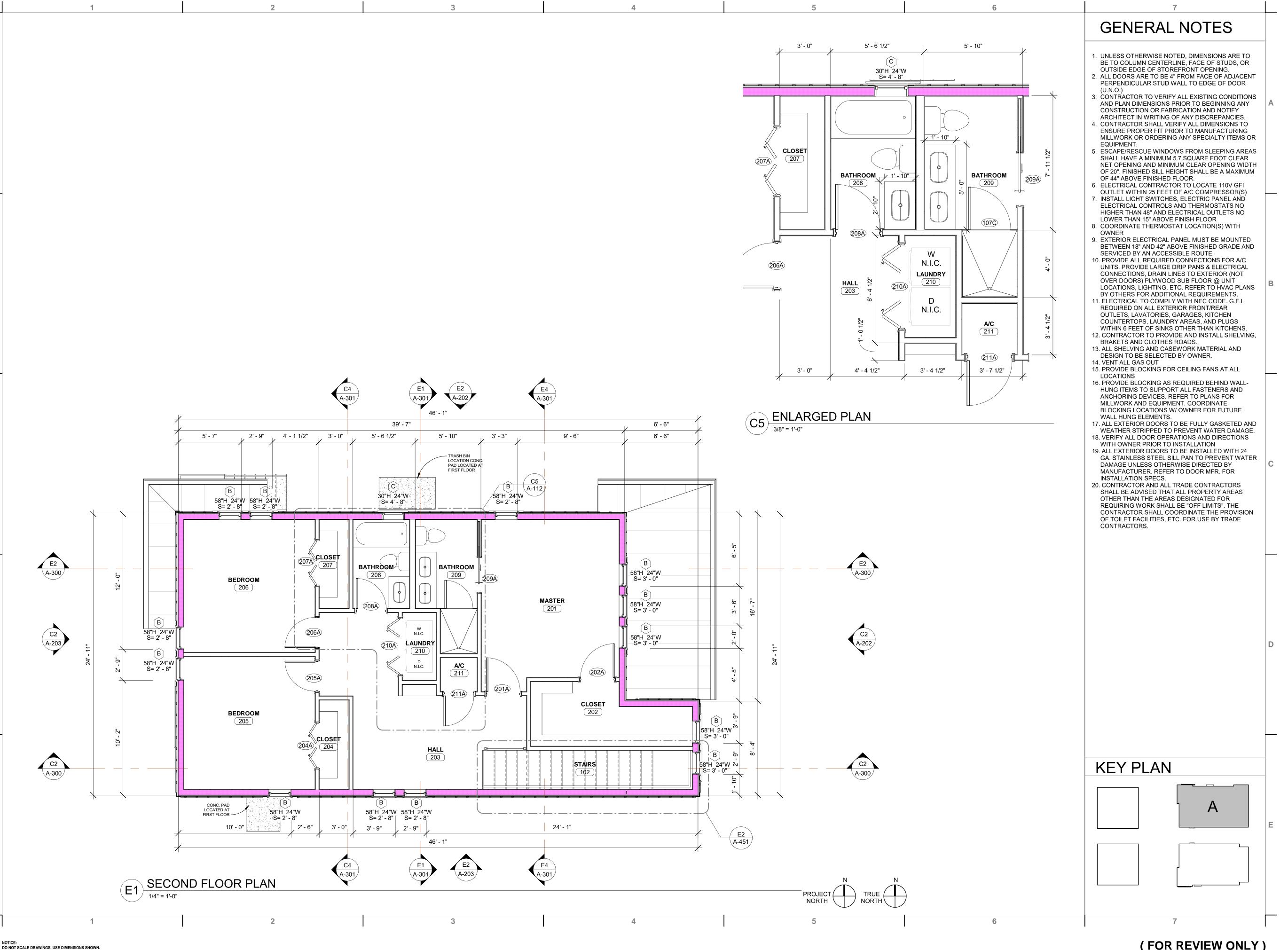
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### **CONSTRUCTION DOCUMENTS** HISTORICAL BOARD REVIEW

PROJECT NUMBER: 2020132 PROJECT MANAGER: B. SOWELL

PROJECT TEAM: S. JURADO, E. SOWELL

**FIRST FLOOR - BUILDING** 



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INTERMEDIATE CONSTRUCTION 2021.02.12 DATE DESCRIPTION

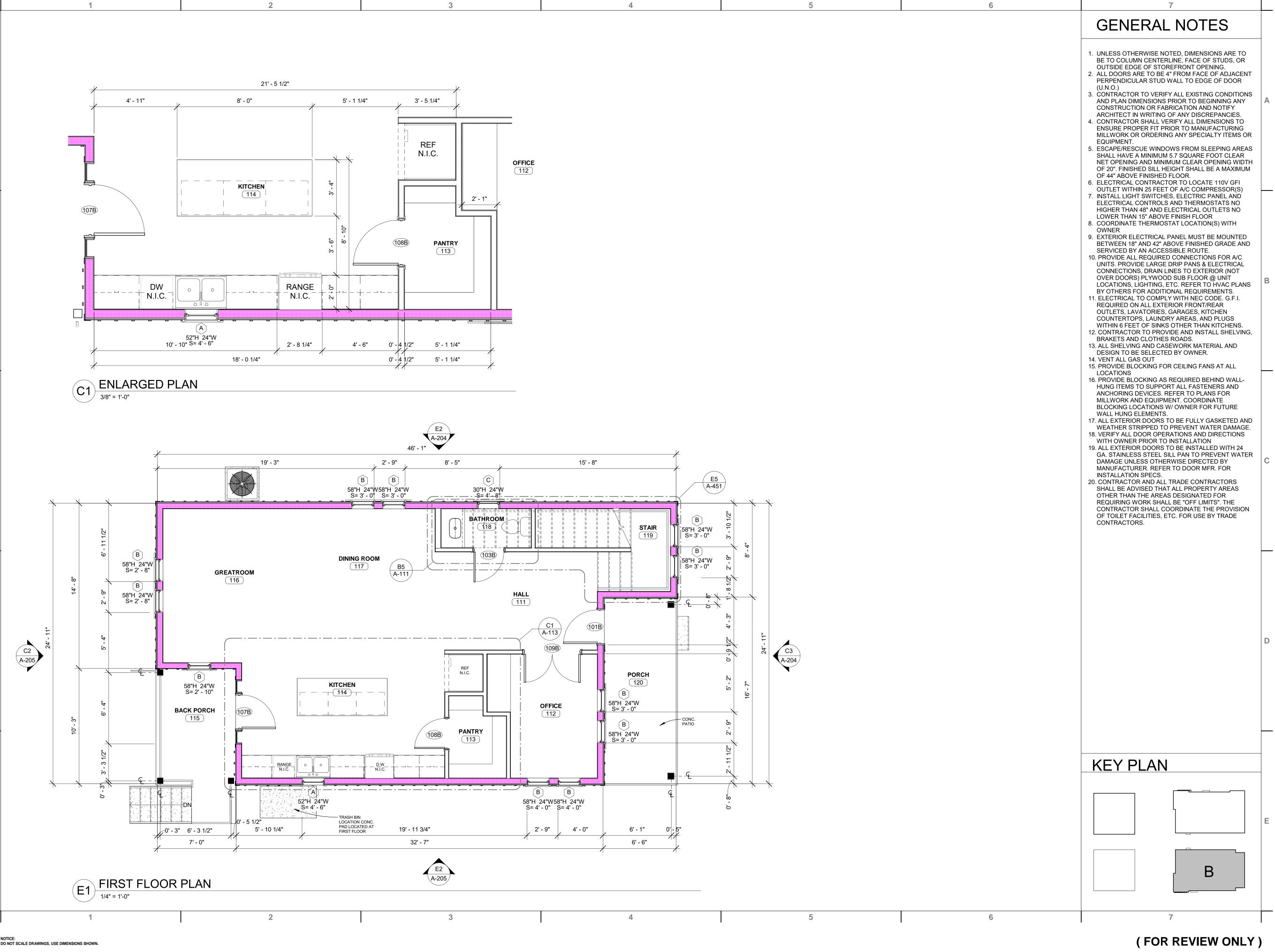
SCHEDULE OF REVISIONS

### **CONSTRUCTION DOCUMENTS** HISTORICAL BOARD REVIEW

PROJECT NUMBER: 2020132 PROJECT DATE: 2021.06.29

PROJECT MANAGER: B. SOWELL PROJECT TEAM: S. JURADO, E. SOWELL

**SECOND FLOOR -BUILDING A** 



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INTERMEDIATE CONSTRUCTION 2021.02.12 **DOCUMENTS** DATE DESCRIPTION

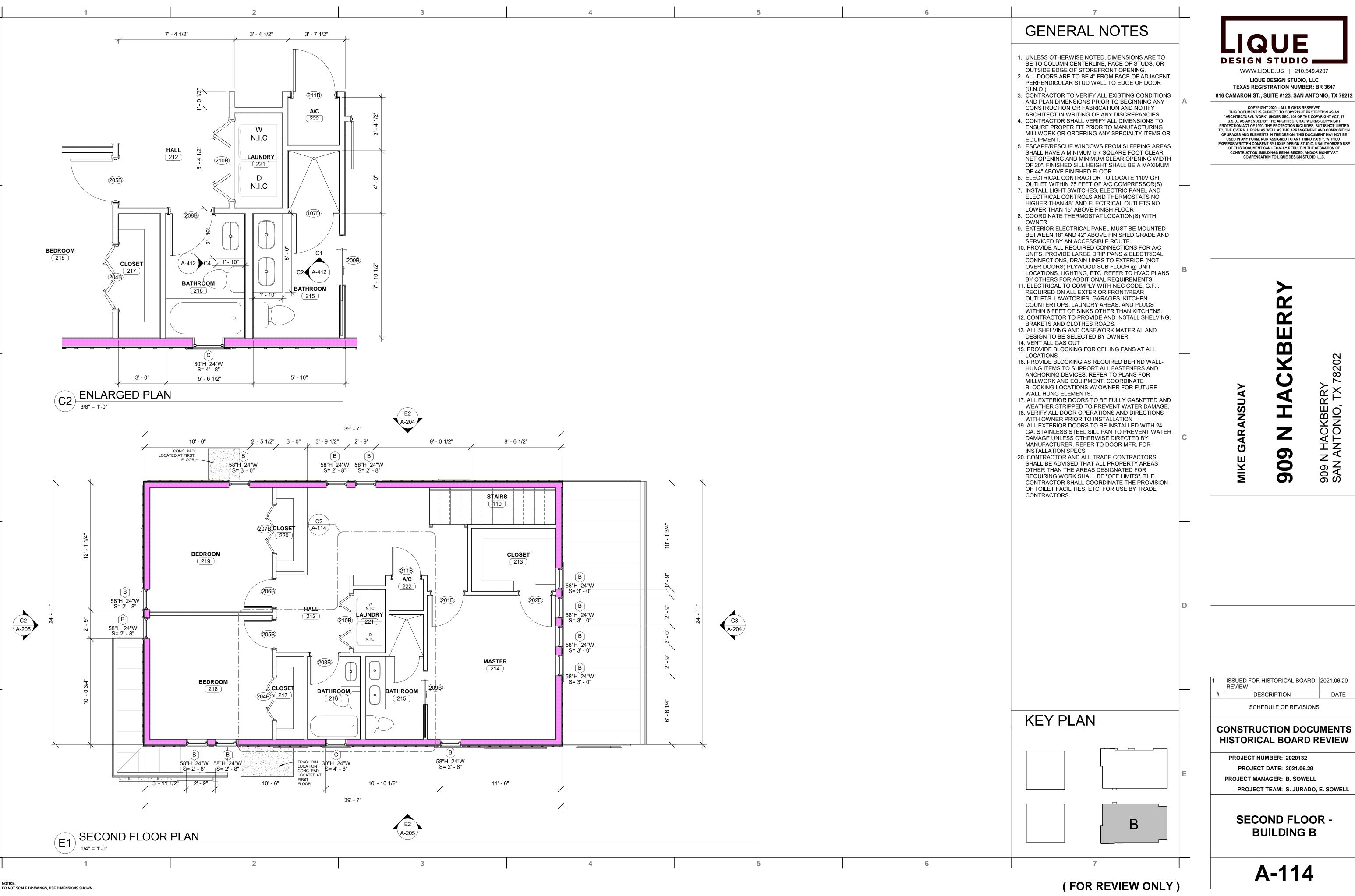
SCHEDULE OF REVISIONS

## **CONSTRUCTION DOCUMENTS** HISTORICAL BOARD REVIEW

PROJECT NUMBER: 2020132 PROJECT DATE: 2021.06.29

PROJECT MANAGER: B. SOWELL PROJECT TEAM: S. JURADO, E. SOWELL

**FIRST FLOOR - BUILDING** 



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## **CONSTRUCTION DOCUMENTS**

PROJECT NUMBER: 2020132 PROJECT DATE: 2021.06.29

PROJECT TEAM: S. JURADO, E. SOWELL

**SECOND FLOOR -BUILDING B** 

GENERAL NOTES

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REVIEW 2021.06.29

INTERMEDIATE CONSTRUCTION 2021.02.12 DOCUMENTS DESCRIPTION DATE

SCHEDULE OF REVISIONS

## CONSTRUCTION DOCUMENTS HISTORICAL BOARD REVIEW

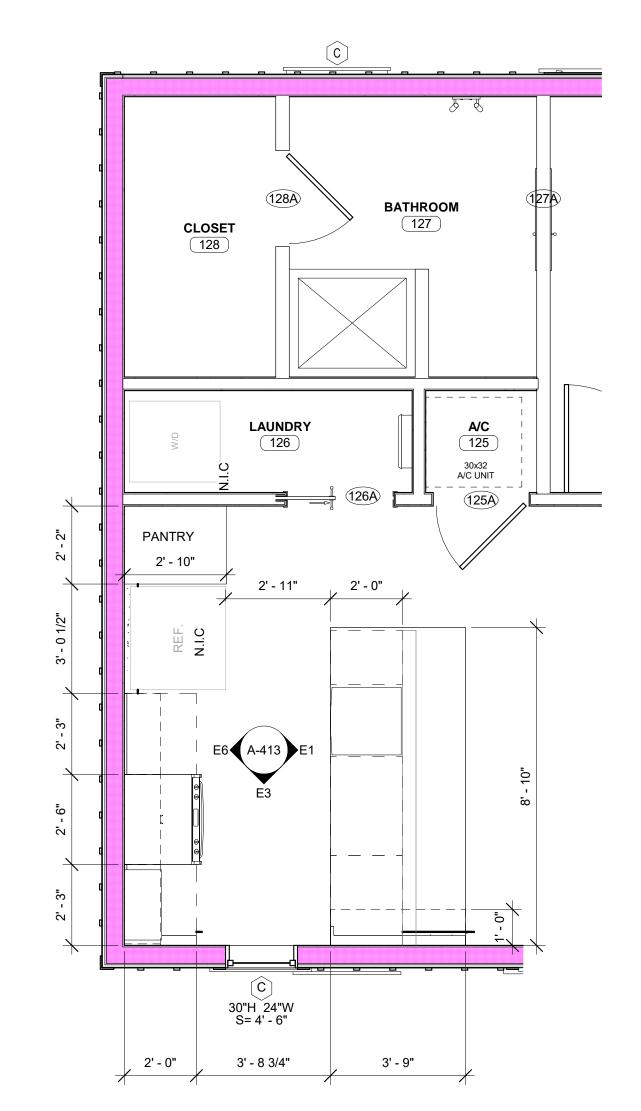
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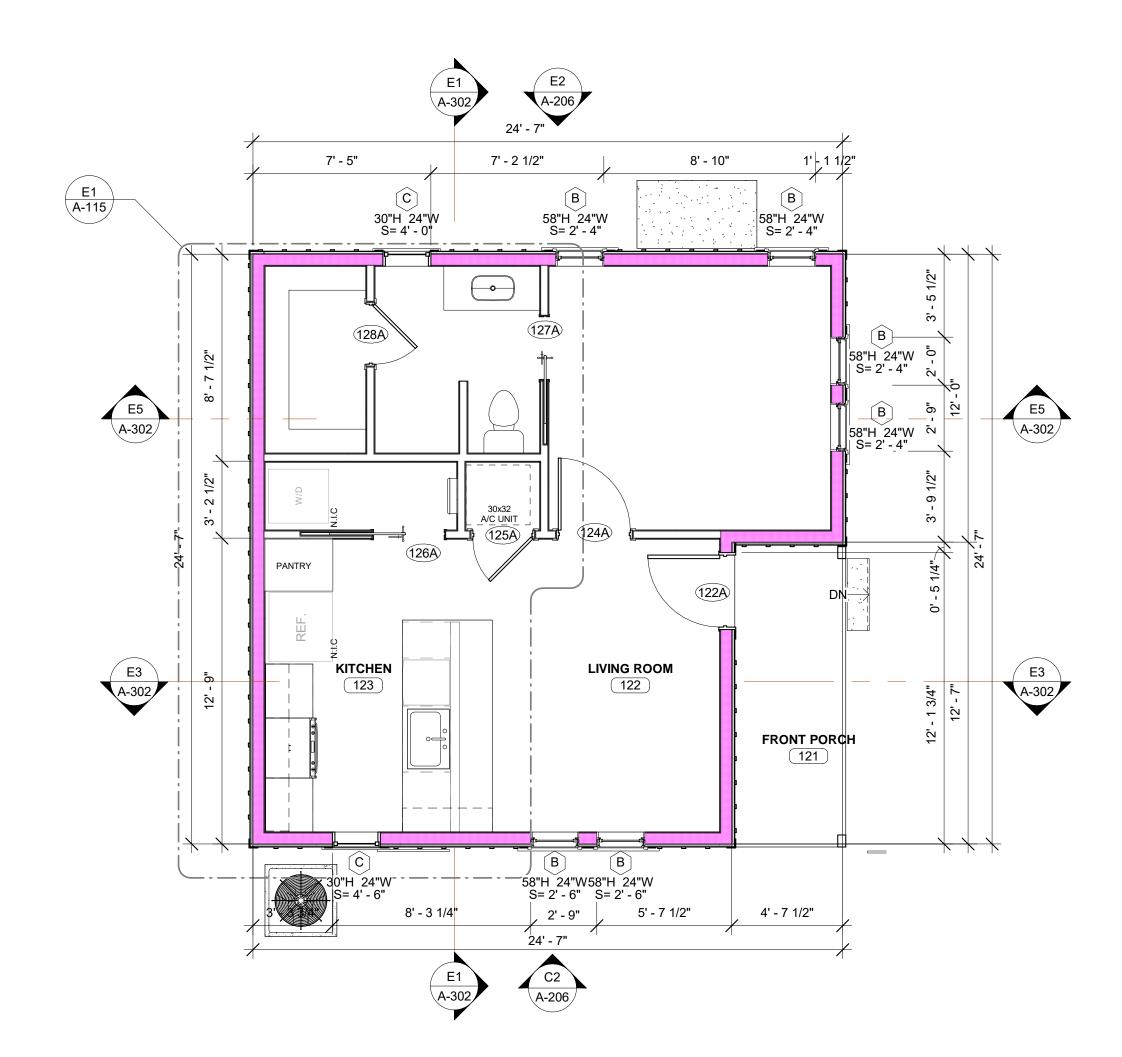
PROJECT DATE: 2021.06.29

PROJECT MANAGER: B. SOWELL
PROJECT TEAM: S. JURADO, E. SOWELL

FLOOR PLAN - ADU

A-115

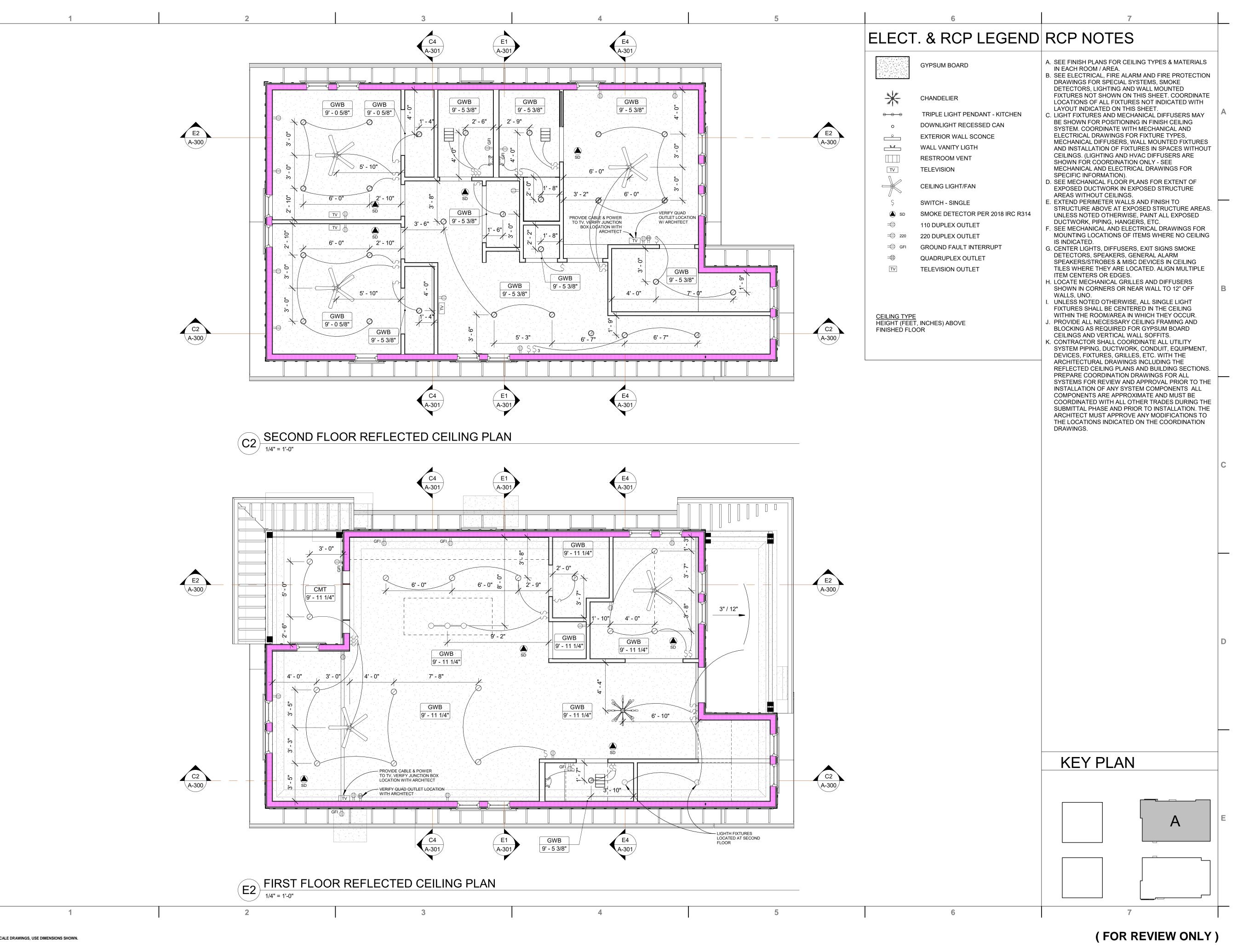




E3 ENLARGED FIRST FLOOR ADU PLAN

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**ENLARGED PLAN** 



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DESCRIPTION

SCHEDULE OF REVISIONS

DATE

### **CONSTRUCTION DOCUMENTS** HISTORICAL BOARD REVIEW

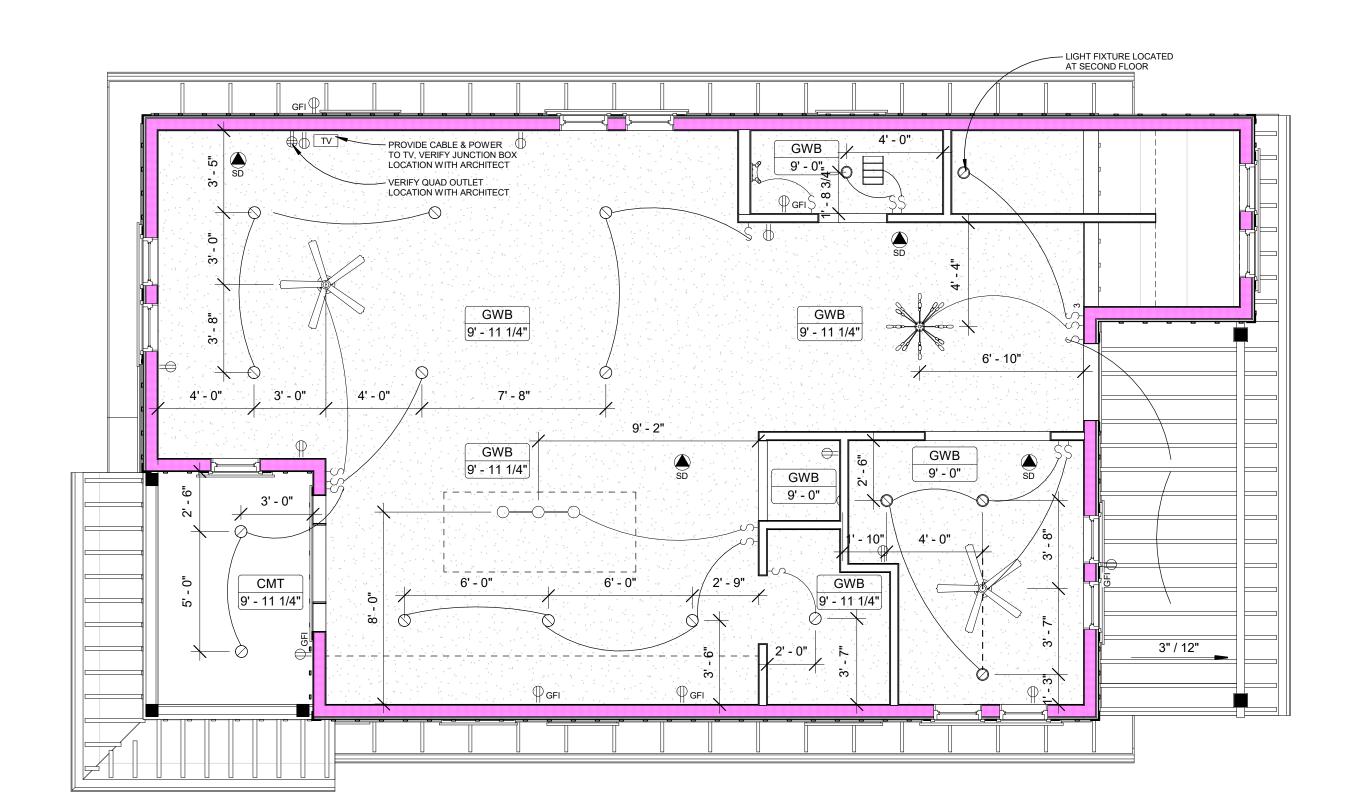
PROJECT NUMBER: 2020132 PROJECT DATE: 2021.06.29 PROJECT MANAGER: B. SOWELL

PROJECT TEAM: S. JURADO, E. SOWELL

**REFLECTED CEILING PLAN BUILDING A** 

C2 SECOND FLOOR REFLECTED CEILING PLAN

1/4" = 1'-0"



FIRST FLOOR REFLECTED CEILING PLAN

1/4" = 1'-0"

ELECT. & RCP LEGEND RCP NOTES

GYPSUM BOARD

CHANDELIER

TRIPLE LIGHT PENDANT - KITCHEN

DOWNLIGHT RECESSED CAN EXTERIOR WALL SCONCE WALL VANITY LIGTH

RESTROOM VENT TV **TELEVISION** 

CEILING LIGHT/FAN

SWITCH - SINGLE SMOKE DETECTOR PER 2018 IRC R314

110 DUPLEX OUTLET 220 DUPLEX OUTLET GROUND FAULT INTERRUPT

QUADRUPLEX OUTLET TELEVISION OUTLET

<u>CEILING TYPE</u> HEIGHT (FEET, INCHES) ABOVE FINISHED FLOOR

A. SEE FINISH PLANS FOR CEILING TYPES & MATERIALS IN EACH ROOM / AREA.

B. SEE ELECTRICAL, FIRE ALARM AND FIRE PROTECTION DRAWINGS FOR SPECIAL SYSTEMS, SMOKE DETECTORS, LIGHTING AND WALL MOUNTED FIXTURES NOT SHOWN ON THIS SHEET. COORDINATE LOCATIONS OF ALL FIXTURES NOT INDICATED WITH

LAYOUT INDICATED ON THIS SHEET. C. LIGHT FIXTURES AND MECHANICAL DIFFUSERS MAY BE SHOWN FOR POSITIONING IN FINISH CEILING SYSTEM. COORDINATE WITH MECHANICAL AND ELECTRICAL DRAWINGS FOR FIXTURE TYPES, MECHANICAL DIFFUSERS, WALL MOUNTED FIXTURES AND INSTALLATION OF FIXTURES IN SPACES WITHOUT CEILINGS. (LIGHTING AND HVAC DIFFUSERS ARE SHOWN FOR COORDINATION ONLY - SEE MECHANICAL AND ELECTRICAL DRAWINGS FOR SPECIFIC INFORMATION).

D. SEE MECHANICAL FLOOR PLANS FOR EXTENT OF EXPOSED DUCTWORK IN EXPOSED STRUCTURE AREAS WITHOUT CEILINGS.

E. EXTEND PERIMETER WALLS AND FINISH TO STRUCTURE ABOVE AT EXPOSED STRUCTURE AREAS. UNLESS NOTED OTHERWISE, PAINT ALL EXPOSED DUCTWORK, PIPING, HANGERS, ETC. SEE MECHANICAL AND ELECTRICAL DRAWINGS FOR

MOUNTING LOCATIONS OF ITEMS WHERE NO CEILING IS INDICATED. G. CENTER LIGHTS, DIFFUSERS, EXIT SIGNS SMOKE DETECTORS, SPEAKERS, GENERAL ALARM SPEAKERS/STROBES & MISC DEVICES IN CEILING TILES WHERE THEY ARE LOCATED. ALIGN MULTIPLE

ITEM CENTERS OR EDGES. H. LOCATE MECHANICAL GRILLES AND DIFFUSERS SHOWN IN CORNERS OR NEAR WALL TO 12" OFF WALLS, UNO. UNLESS NOTED OTHERWISE, ALL SINGLE LIGHT

FIXTURES SHALL BE CENTERED IN THE CEILING WITHIN THE ROOM/AREA IN WHICH THEY OCCUR. PROVIDE ALL NECESSARY CEILING FRAMING AND BLOCKING AS REQUIRED FOR GYPSUM BOARD

CEILINGS AND VERTICAL WALL SOFFITS. K. CONTRACTOR SHALL COORDINATE ALL UTILITY SYSTEM PIPING, DUCTWORK, CONDUIT, EQUIPMENT, DEVICES, FIXTURES, GRILLES, ETC. WITH THE ARCHITECTURAL DRAWINGS INCLUDING THE REFLECTED CEILING PLANS AND BUILDING SECTIONS. PREPARE COORDINATION DRAWINGS FOR ALL SYSTEMS FOR REVIEW AND APPROVAL PRIOR TO THE INSTALLATION OF ANY SYSTEM COMPONENTS ALL COMPONENTS ARE APPROXIMATE AND MUST BE COORDINATED WITH ALL OTHER TRADES DURING THE SUBMITTAL PHASE AND PRIOR TO INSTALLATION. THE ARCHITECT MUST APPROVE ANY MODIFICATIONS TO THE LOCATIONS INDICATED ON THE COORDINATION

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ISSUED FOR HISTORICAL BOARD 2021.06.29 INTERMEDIATE CONSTRUCTION 2021.02.12

SCHEDULE OF REVISIONS

DESCRIPTION

DATE

### **CONSTRUCTION DOCUMENTS** HISTORICAL BOARD REVIEW

PROJECT NUMBER: 2020132 PROJECT DATE: 2021.06.29 PROJECT MANAGER: B. SOWELL

PROJECT TEAM: S. JURADO, E. SOWELL

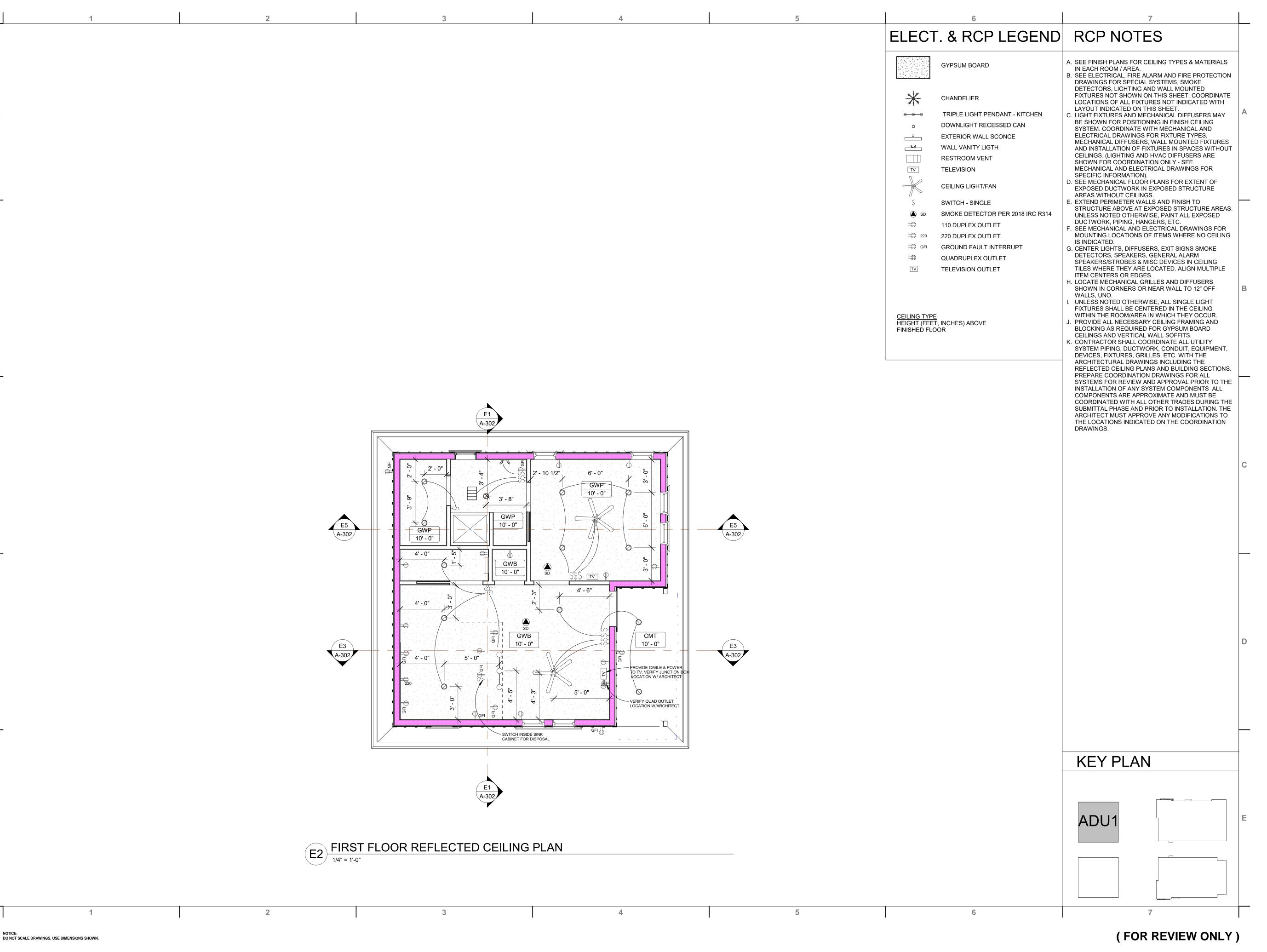
REFLECTED CEILING **PLAN BUILDING B** 

A-122

(FOR REVIEW ONLY)

**KEY PLAN** 

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ISSUED FOR HISTORICAL BOARD | 2021.06.29 INTERMEDIATE CONSTRUCTION 2021.02.12

SCHEDULE OF REVISIONS

DESCRIPTION

DATE

## **CONSTRUCTION DOCUMENTS** HISTORICAL BOARD REVIEW

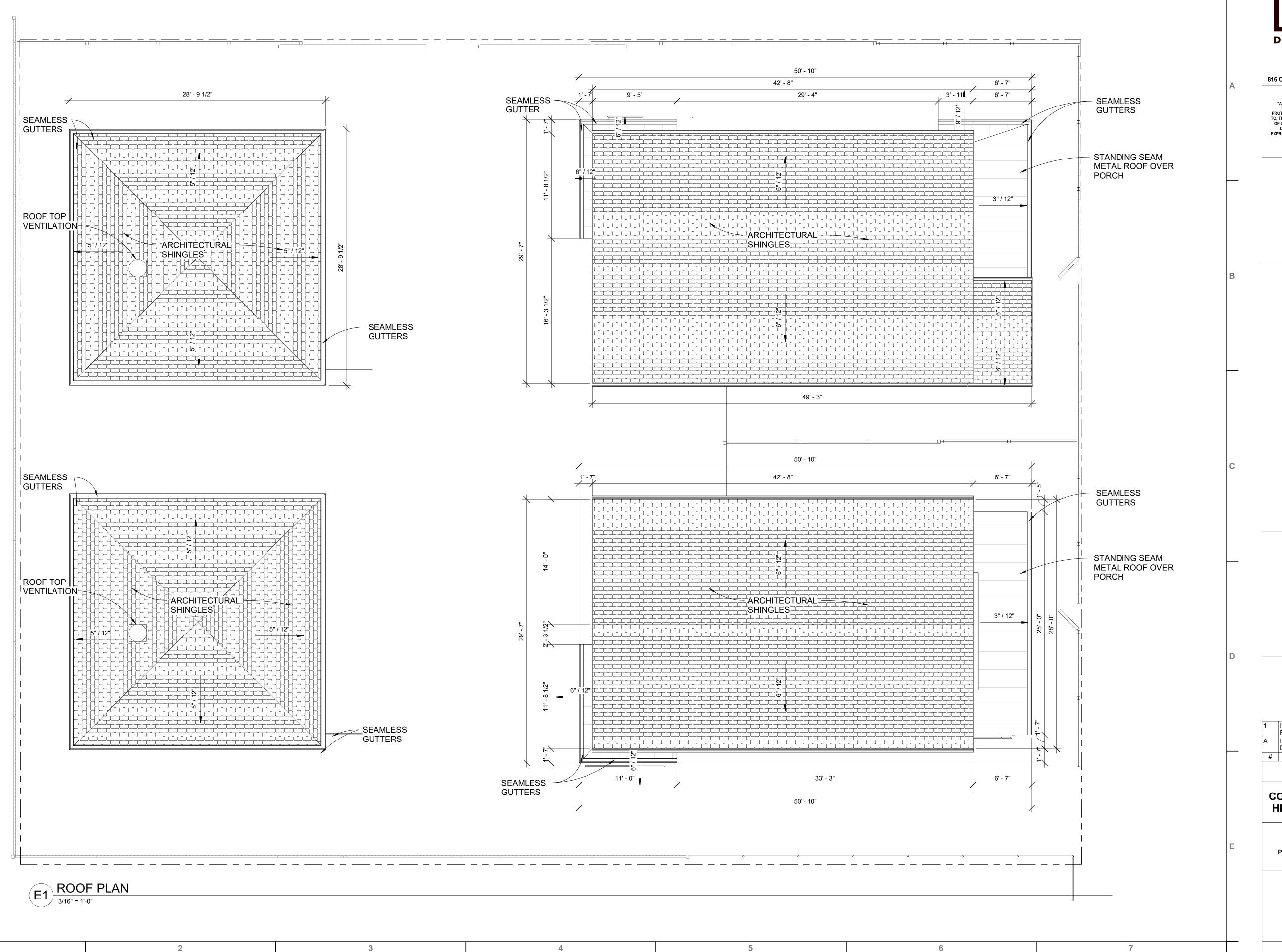
PROJECT NUMBER: 2020132 PROJECT DATE: 2021.06.29 PROJECT MANAGER: B. SOWELL

DOCUMENTS

PROJECT TEAM: S. JURADO, E. SOWELL

**REFLECTED CEILING** 

PLAN - ADU



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INTERMEDIATE CONSTRUCTION DOCUMENTS
DESCRIPTION
DATE

SCHEDULE OF REVISIONS

## CONSTRUCTION DOCUMENTS HISTORICAL BOARD REVIEW

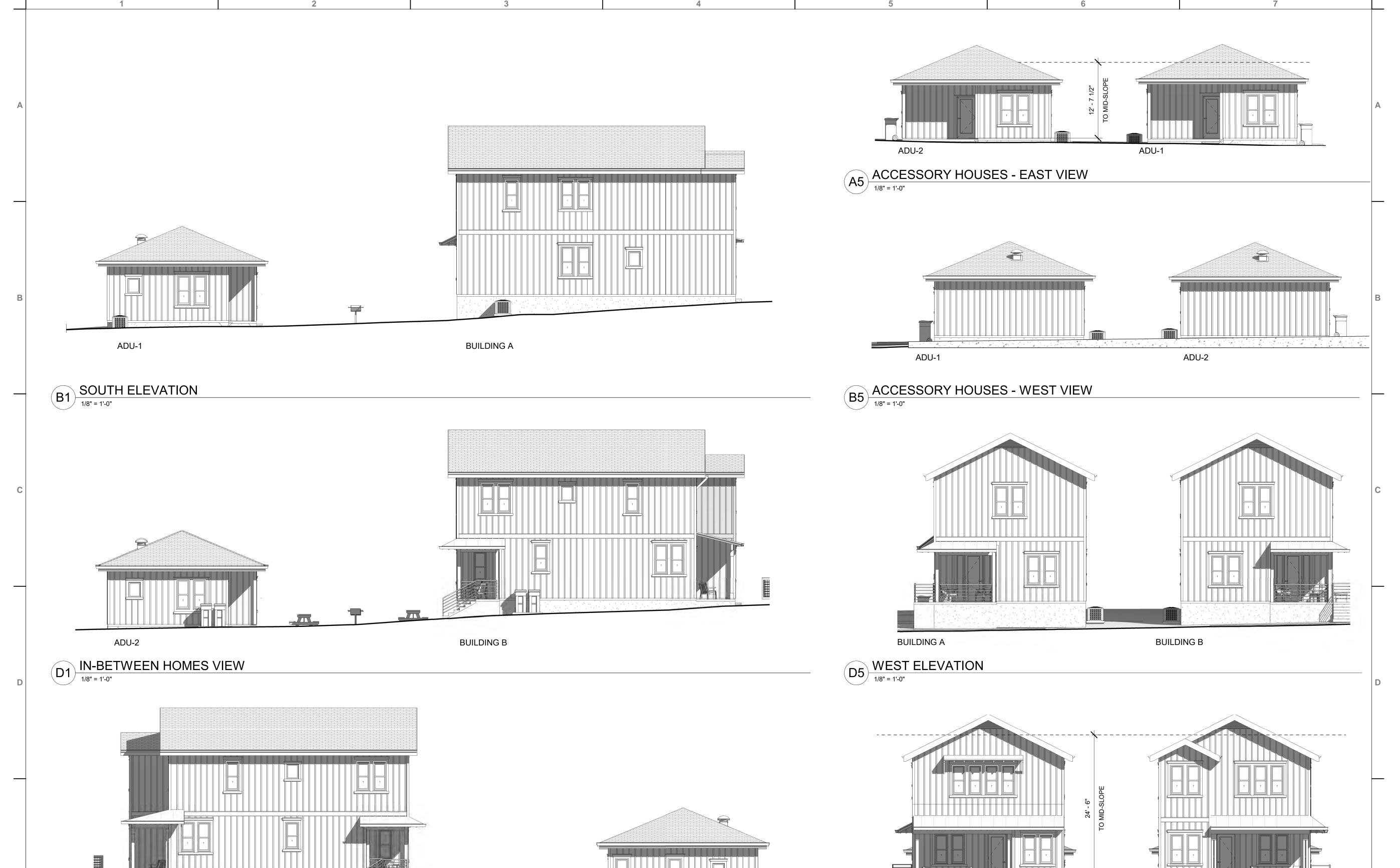
PROJECT NUMBER: 2020132

PROJECT DATE: 2021.06.29

PROJECT MANAGER: B. SOWELL

PROJECT TEAM: S. JURADO, E. SOWELL

**ROOF PLANS** 



**BUILDING B** 

E5 EAST ELEVATION

1/8" = 1'-0"

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INTERMEDIATE CONSTRUCTION DOCUMENTS

2021.06.29
2021.02.12

DESCRIPTION

SCHEDULE OF REVISIONS

DATE

## CONSTRUCTION DOCUMENTS HISTORICAL BOARD REVIEW

PROJECT NUMBER: 2020132
PROJECT DATE: 2021.06.29
PROJECT MANAGER: B. SOWELL

PROJECT TEAM: S. JURADO, E. SOWELL

**OVERALL ELEVATIONS** 

**A-201** 

BUILDING A

BUILDING A

NORTH ELEVATION

1/8" = 1'-0"



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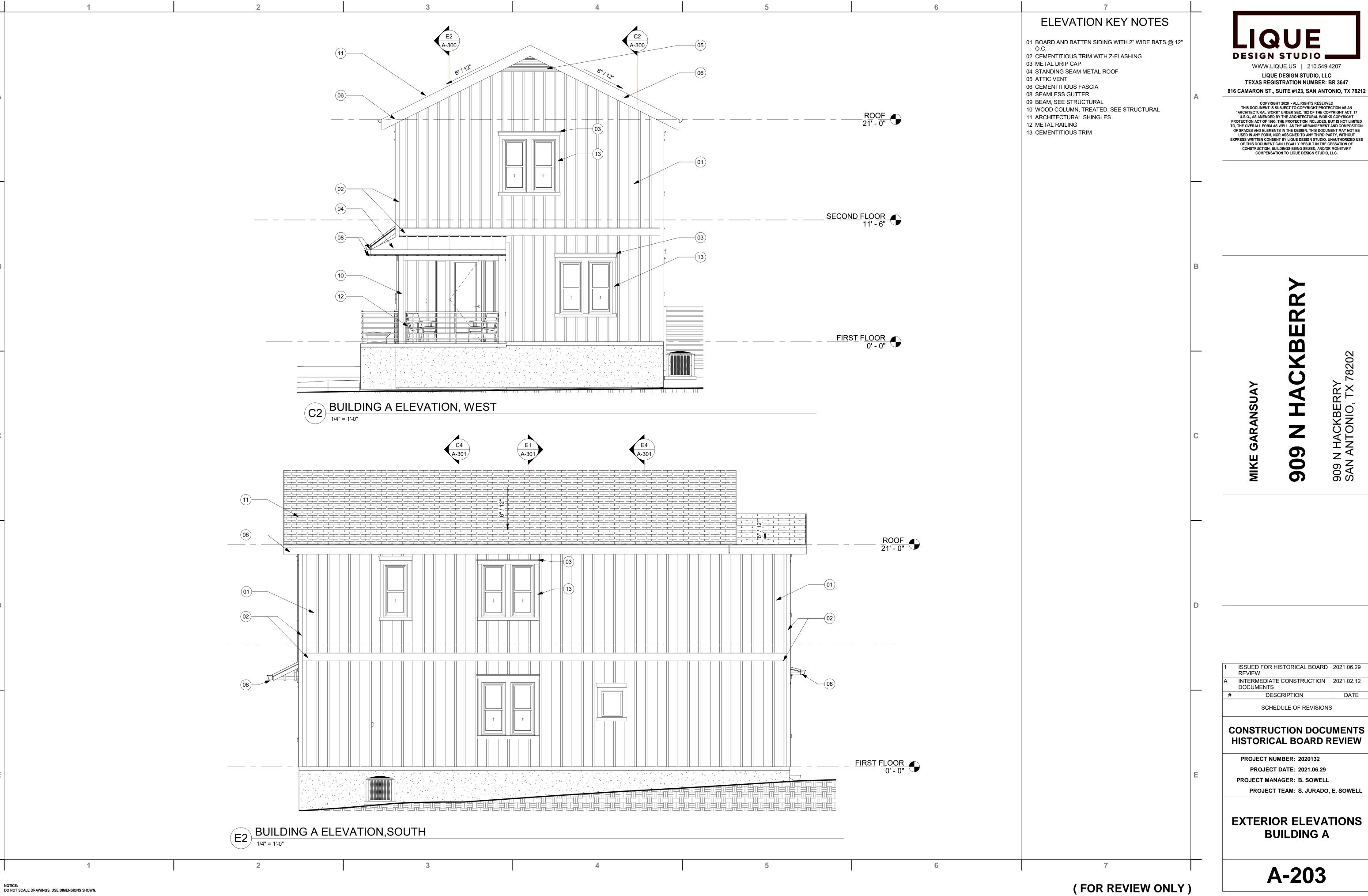
SCHEDULE OF REVISIONS

## **CONSTRUCTION DOCUMENTS** HISTORICAL BOARD REVIEW

PROJECT NUMBER: 2020132 PROJECT DATE: 2021.06.29 PROJECT MANAGER: B. SOWELL

PROJECT TEAM: S. JURADO, E. SOWELL

**EXTERIOR ELEVATIONS BUILDING A** 



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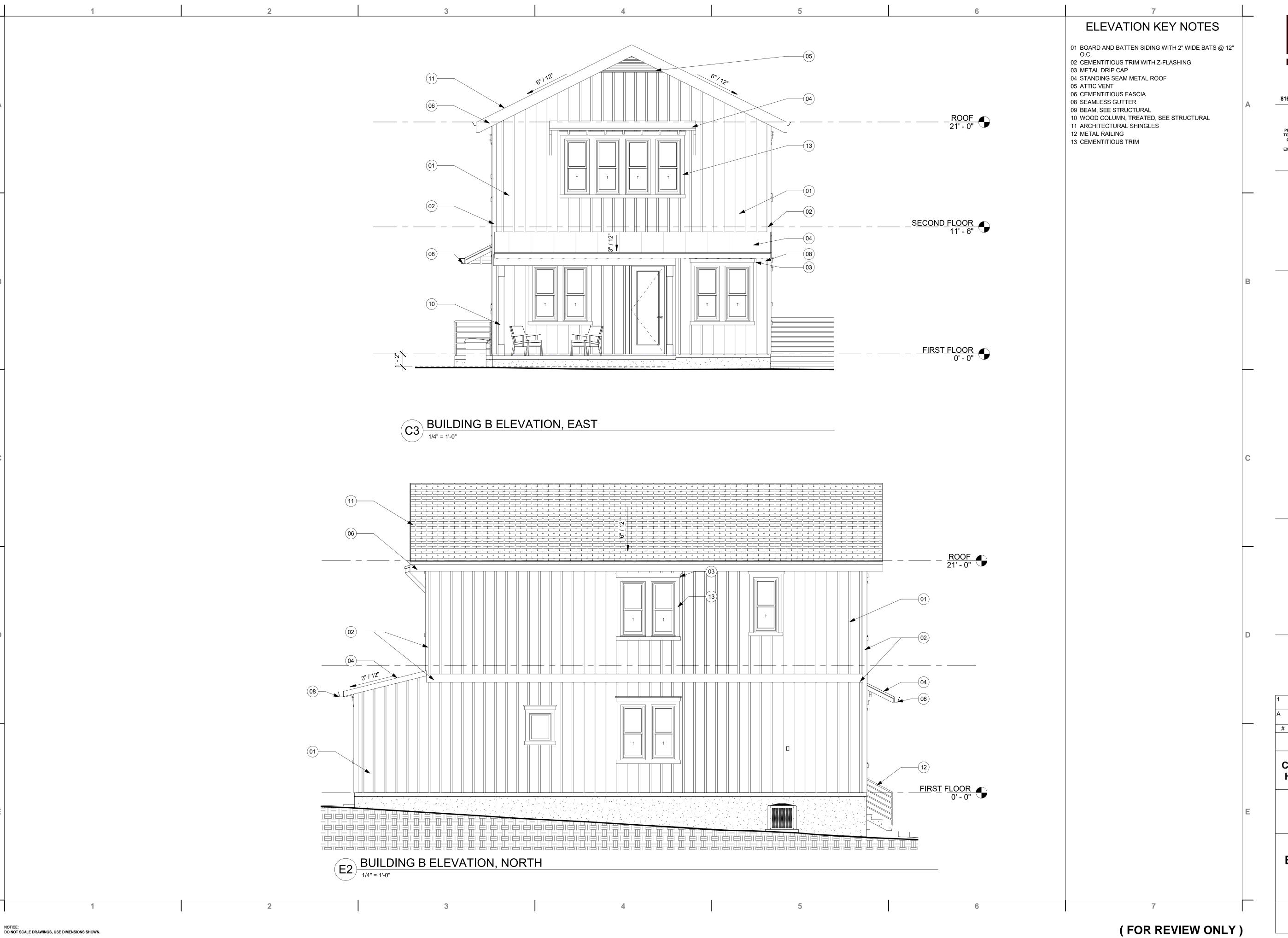
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## **CONSTRUCTION DOCUMENTS** HISTORICAL BOARD REVIEW

PROJECT NUMBER: 2020132 PROJECT DATE: 2021.06.29 PROJECT MANAGER: B. SOWELL

PROJECT TEAM: S. JURADO, E. SOWELL

**EXTERIOR ELEVATIONS BUILDING A** 



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## **CONSTRUCTION DOCUMENTS** HISTORICAL BOARD REVIEW

PROJECT NUMBER: 2020132 PROJECT DATE: 2021.06.29 PROJECT MANAGER: B. SOWELL

PROJECT TEAM: S. JURADO, E. SOWELL

**EXTERIOR ELEVATIONS BUILDING B** 



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2021.06.29
2021.02.12

SCHEDULE OF REVISIONS

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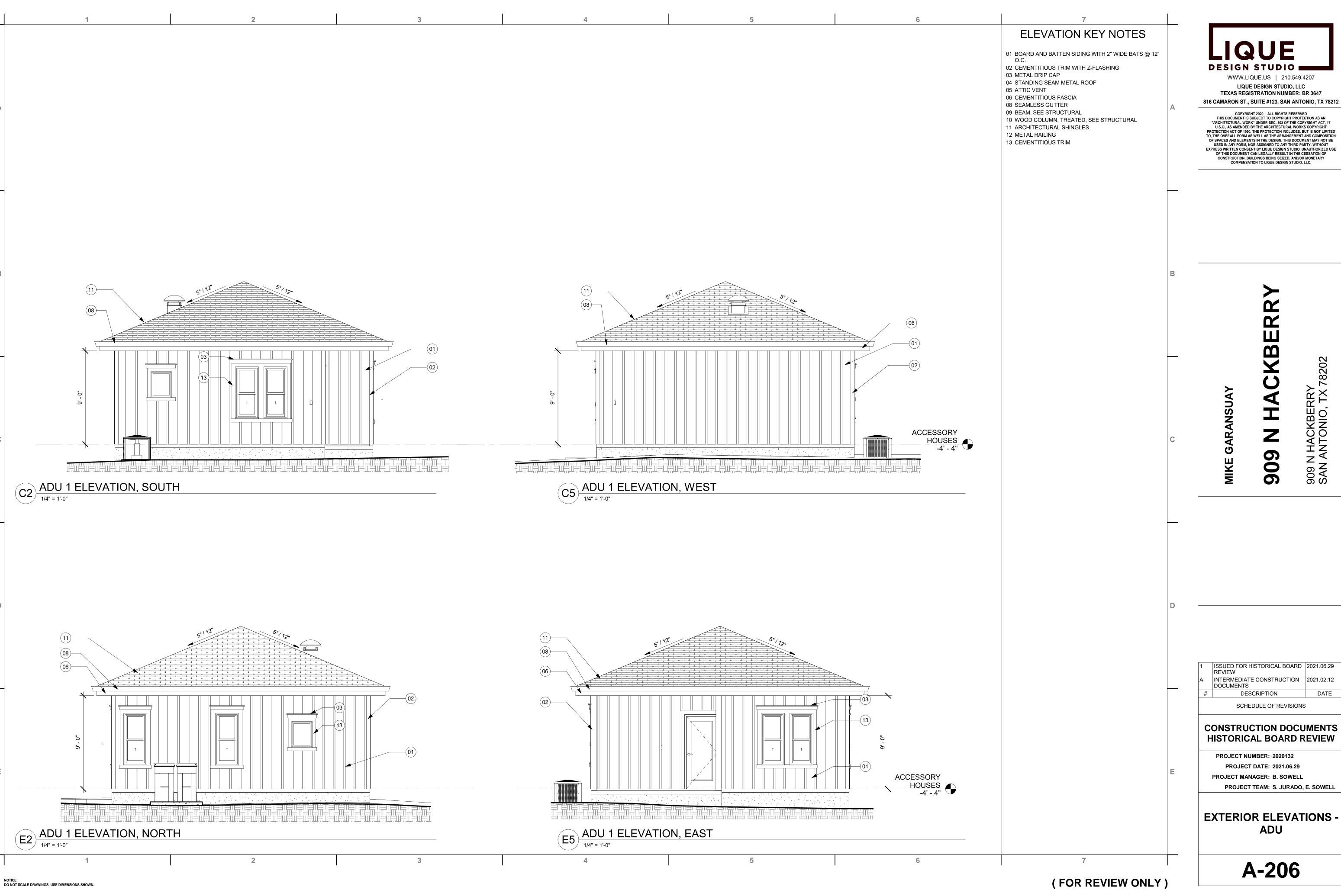
DESCRIPTION

## CONSTRUCTION DOCUMENTS HISTORICAL BOARD REVIEW

PROJECT NUMBER: 2020132
PROJECT DATE: 2021.06.29
PROJECT MANAGER: B. SOWELL

PROJECT TEAM: S. JURADO, E. SOWELL

EXTERIOR ELEVATIONS
BUILDING B



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SCHEDULE OF REVISIONS

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## **CONSTRUCTION DOCUMENTS** HISTORICAL BOARD REVIEW

PROJECT NUMBER: 2020132 PROJECT DATE: 2021.06.29 PROJECT MANAGER: B. SOWELL

PROJECT TEAM: S. JURADO, E. SOWELL

**EXTERIOR ELEVATIONS -**ADU



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### **CONSTRUCTION DOCUMENTS** HISTORICAL BOARD REVIEW

PROJECT NUMBER: 2020132 PROJECT DATE: 2021.06.29 PROJECT MANAGER: B. SOWELL

**BUILDING SECTIONS** 

**A-300** 



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## **CONSTRUCTION DOCUMENTS** HISTORICAL BOARD REVIEW

PROJECT NUMBER: 2020132 PROJECT DATE: 2021.06.29 PROJECT MANAGER: B. SOWELL

PROJECT TEAM: S. JURADO, E. SOWELL

**BUILDING SECTION BUILDING A** 

A-301



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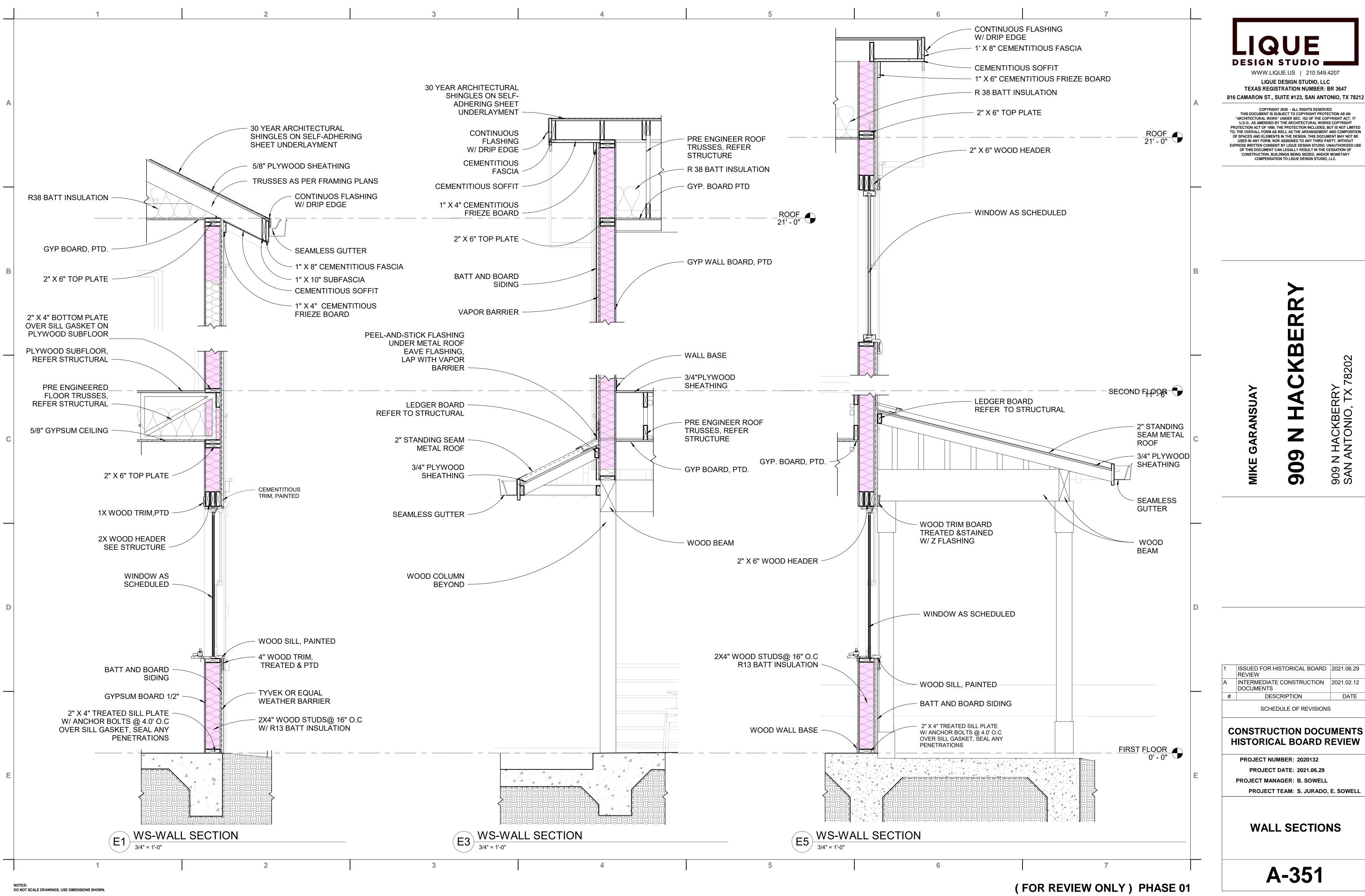
## CONSTRUCTION DOCUMENTS HISTORICAL BOARD REVIEW

PROJECT NUMBER: 2020132
PROJECT DATE: 2021.06.29
PROJECT MANAGER: B. SOWELL

PROJECT TEAM: S. JURADO, E. SOWELL

BUILDING SECTIONS -ADU

**A-302** 



## **CONSTRUCTION DOCUMENTS**



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## CONSTRUCTION DOCUMENTS HISTORICAL BOARD REVIEW

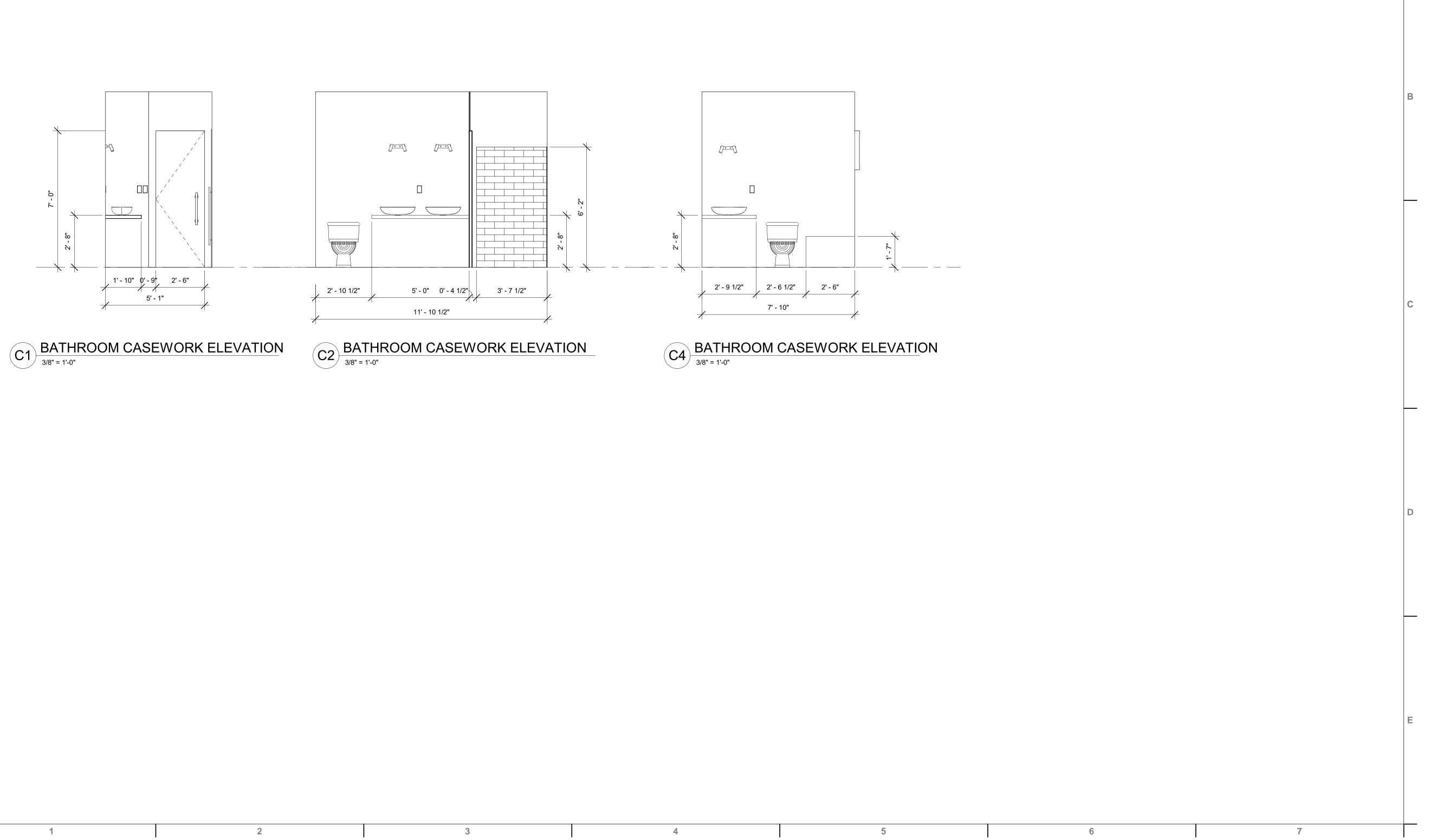
PROJECT NUMBER: 2020132

PROJECT DATE: 2021.06.29

PROJECT MANAGER: B. SOWELL

PROJECT TEAM: S. JURADO, E. SOWELL

## CASEWORK ELEVATIONS - BUILDING A



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## CONSTRUCTION DOCUMENTS HISTORICAL BOARD REVIEW

PROJECT NUMBER: 2020132
PROJECT DATE: 2021.06.29
PROJECT MANAGER: B. SOWELL

PROJECT TEAM: S. JURADO, E. SOWELL

CASEWORK ELEVATIONS
- BUILDING B

A-412



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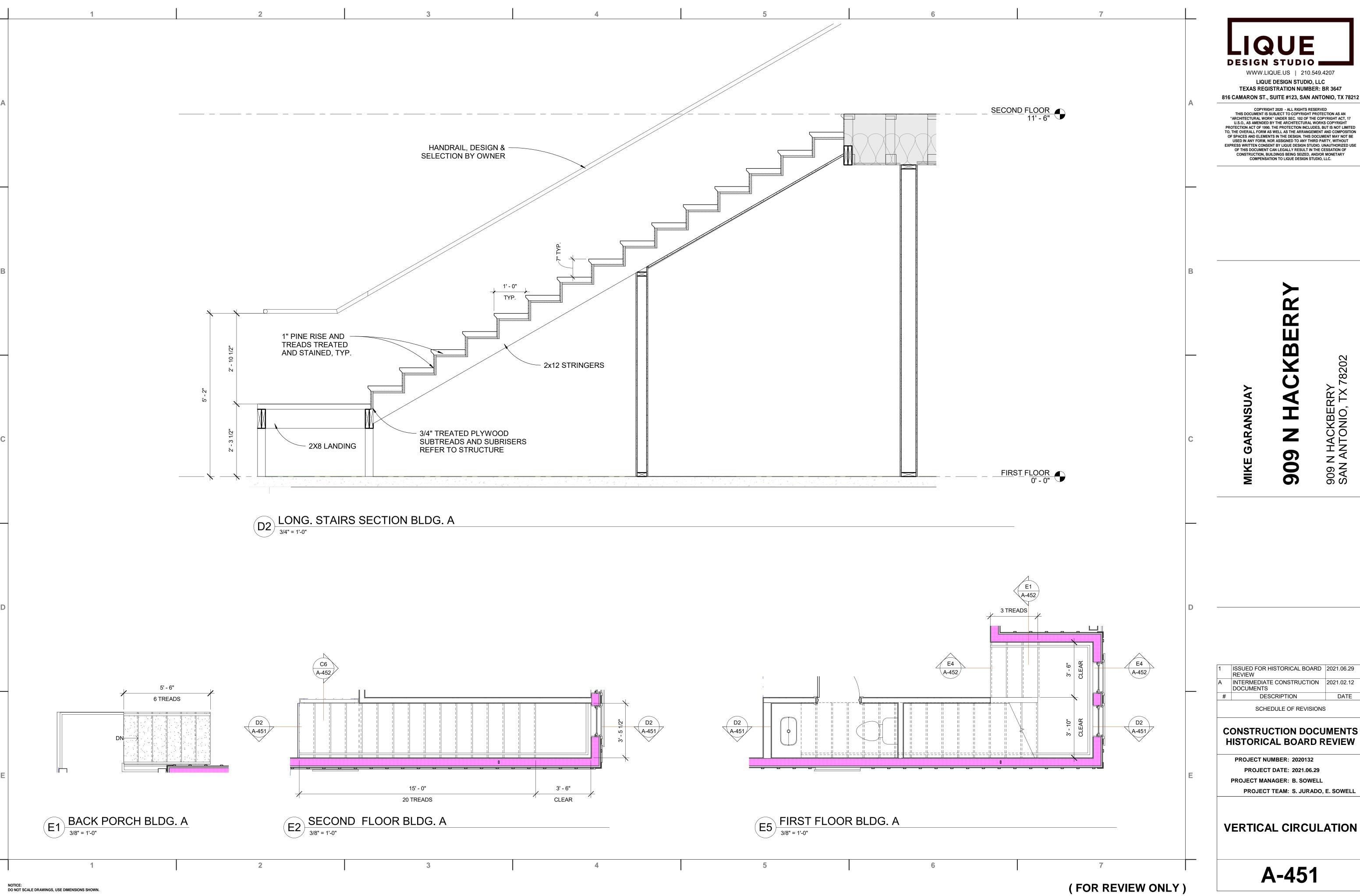
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PROJECT NUMBER: 2020132 PROJECT DATE: 2021.06.29 PROJECT MANAGER: B. SOWELL

PROJECT TEAM: S. JURADO, E. SOWELL

**CASEWORK ELEVATIONS** - ADU

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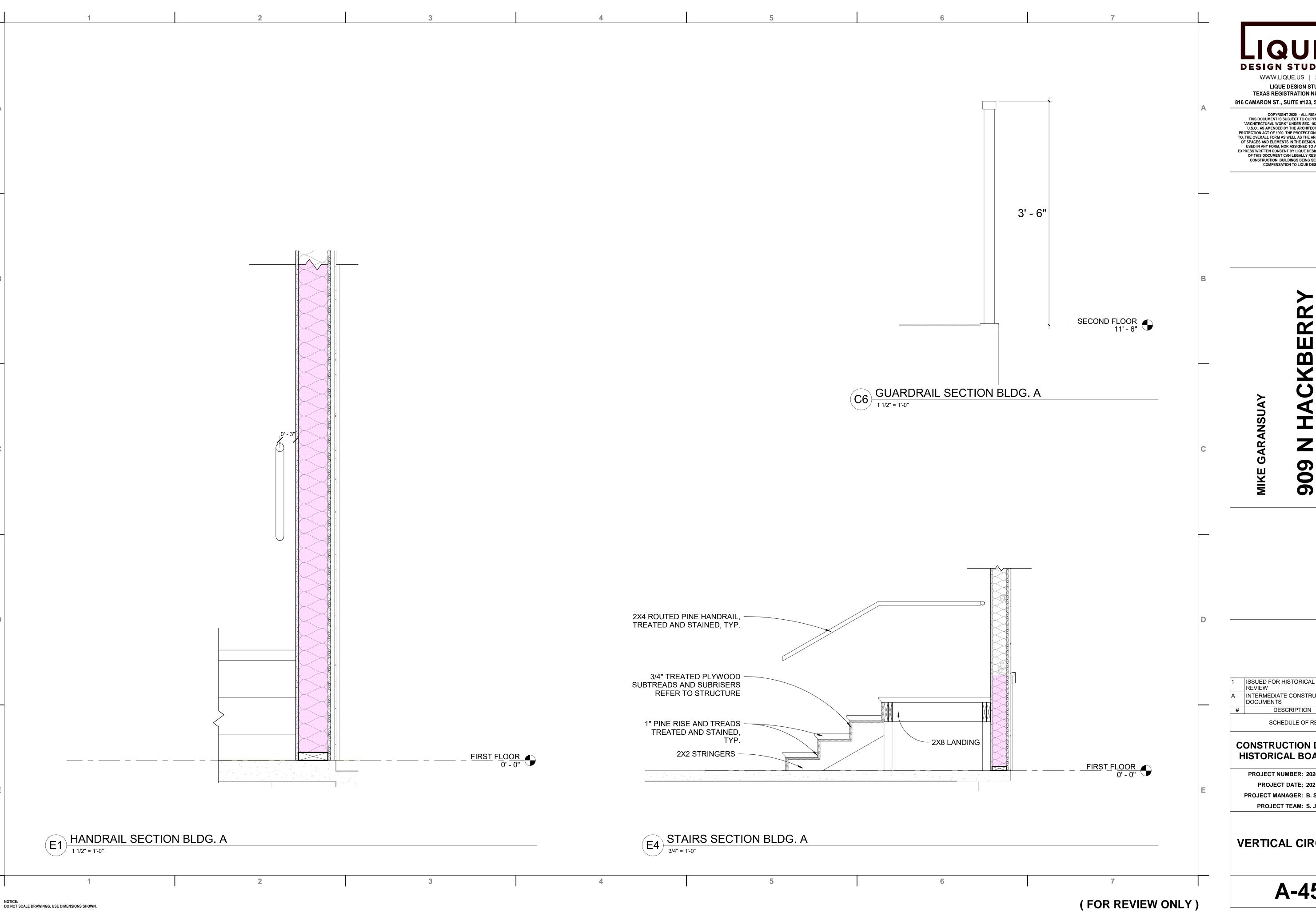
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### **CONSTRUCTION DOCUMENTS** HISTORICAL BOARD REVIEW

PROJECT NUMBER: 2020132 PROJECT DATE: 2021.06.29

PROJECT TEAM: S. JURADO, E. SOWELL

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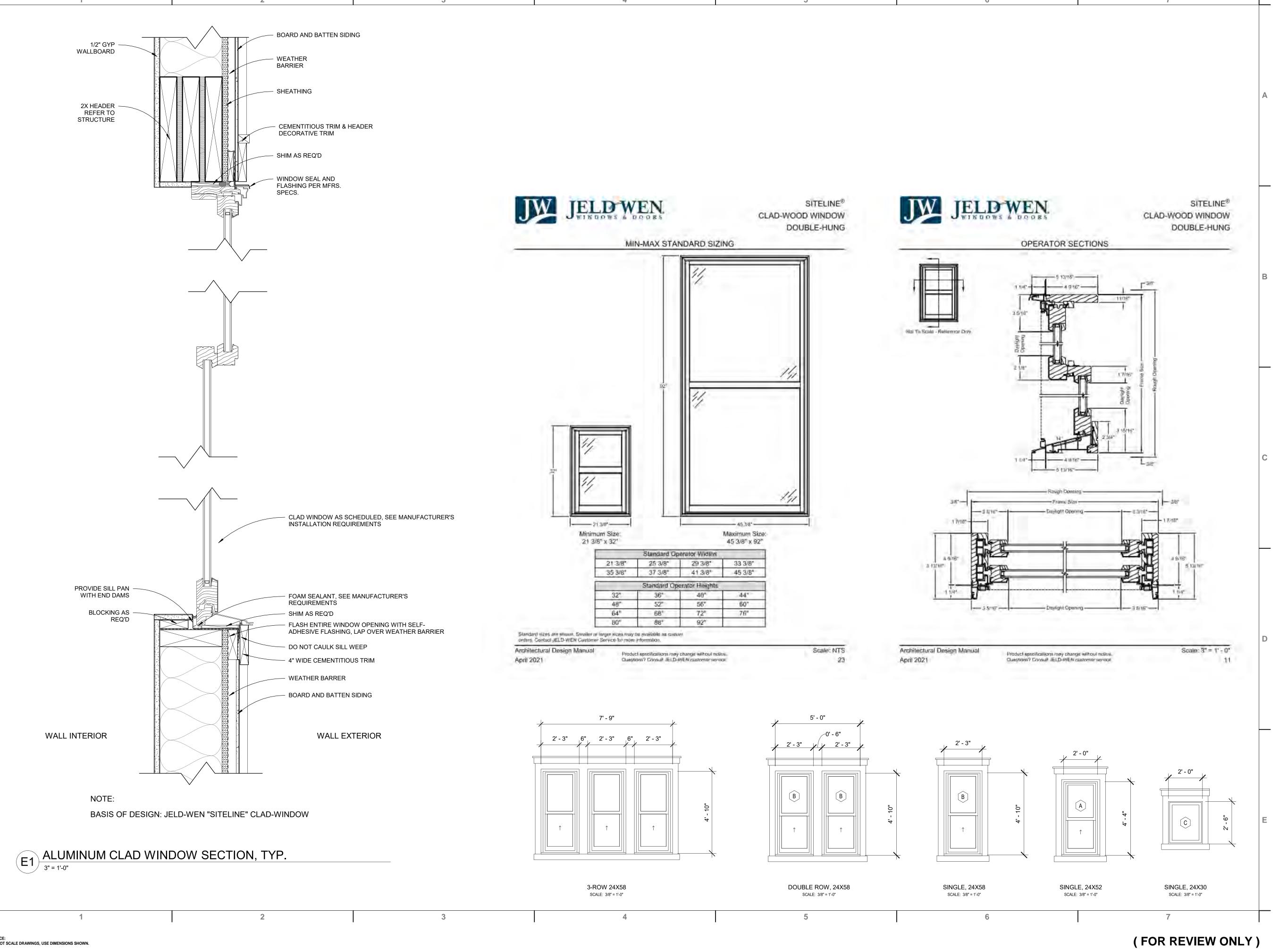
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## **CONSTRUCTION DOCUMENTS** HISTORICAL BOARD REVIEW

PROJECT NUMBER: 2020132 PROJECT DATE: 2021.06.29 PROJECT MANAGER: B. SOWELL

PROJECT TEAM: S. JURADO, E. SOWELL

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PROJECT NUMBER: 2020132 PROJECT DATE: 2021.06.29 PROJECT MANAGER: B. SOWELL

PROJECT TEAM: S. JURADO, E. SOWELL

**GLAZING ELEVATIONS & DETAILS**